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FINAL REPORT

TO

NATIONAL SONIC BOOM EVALUATION OFFICE

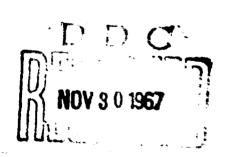
# RESPONSE OF STRUCTURES TO SONIC BOOMS

PRODUCED BY XB-70, B-58 AND F-104 AIRCRAFT

BASED ON SONIC BOOM EXPERIMENTS
AT
EDWARDS AIR FORCE BASE

CONTRACT NO. AF 49(638) - 1739

OCTOBER 1967



# JOHN A. BLUME & ASSOCIATES RESEARCH DIVISION

SAN FRANCISCO





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RESPONSE OF STRUCTURES TO SONIC BOOMS
PRODUCED BY XB-70, B-58 AND F-104 AIRCRAFT

Ву

J. A. Blume, R. L. Sharpe G. Kost, J. Proulx

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#### **ABSTRACT**

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The response of test structures and structure elements to sonic booms produced by XB-70, B-58 and F-104 aircraft was studied. These aircraft produced sonic booms of different signature durations. They were flown at several flight track offsets, altitudes and Mach numbers so as to generate different overpressure levels and signature characteristics. Free field signature data and the effects of free field signature parameters on structural response were analysed. Studies were made of the plate response (lateral deformation) and racking response (in-plane deformation) of the test structures. Damage complaints resulting from the test missions were investigated and the results analysed. The implications of the magnitudes of the responses of the test structures and the investigation of the damage claims resulting from the test missions on possible damage caused by supersonic flights were discussed.

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# RESPONSE OF STRUCTURES TO SONIC BOOMS PRODUCED BY X8-70, B-58 AND F-104 AIRCRAFT

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#### 1. SUMMARY OF MAJOR FINDINGS AND RECOMMENDATIONS

This report was prepared as partial fulfillment of the requirements of Contract AF 49(638)-1739. The report summarizes the work performed by John A. Blume & Associates Research Division during the sonic boom experiments at Edwards Air Force Base. A detailed discussion of findings derived from analyses of the data measured and recorded is presented.

The general objective of the structure response portion of the Edwards Air Force Base Program, "determine the response of typical structures to sonic booms having different signature characteristics and evaluate damage resulting from the program overflights", was accomplished. The response of test structures and structure elements to sonic booms produced by XB-70. B-58 and F-104 aircraft was studied. These aircraft produced sonic booms of different signature durations. They were flown at several flight track offsets, altitudes and Mach numbers so as to generate different overpressure levels and signature characteristics. Free field signature data and the effects of free field signature parameters on structural response were analysed. Studies were made of the plate response (lateral deformation) and racking response (in-plate deformation) of the test structures. Damage complaints resulting from the test missions were investigated and the results analysed. The implications of the magnitudes of the responses of the test structures and the results of the investigations of damage claims resulting from the test missions on possible damage caused by supersonic flights were evaluated.

This chapter presents a summary of major findings and recommendations, and Chapter II presents a detailed summary of this report. Detailed discussions of the analyses performed are covered in Chapters III through XI.

The findings presented in this report are based on detailed analyses of structure response and free field overpressure data for seventeen comparable XB-70, B-58, and F-104 missions flown within minutes of each other.

The measured plate response of three gypsum board/wood stud/wood siding walls and one large plate glass window, and the measured racking response of two typical wood frame houses, one one-story and one two-story house, were analysed in detail and compared with response predicted using boom signatures. In addition, the plate and racking response of a long-span steel frame-metal siding building were analysed.

#### MAJOR FINDINGS

Free field signature data and the effects of free field signature parameters on structural response were analysed and the following are major findings:

- I. Sonic booms from large aircraft such as the XB-70 and the future Supersonic Transport will affect a greater range of structure elements (those elements with frequencies below approximately 5 cps) than will sonic booms from smaller aircraft such as the B-58 and F-104. These results are predictable if the boom and structure element characteristics are known. The natural frequency at which the maximum DAF occurred was primarily a function of the time from start of boom to negative peak  $T_2$ . As  $T_2$  increased, the maximum DAF occurred at a lower natural frequency.  $T_2$  increased as size of aircraft increased.
- 2. The Dynamic Amp'ification Factors (DAF) computed from free field signatures and peak positive free field overpressures were independent of the channel on which the signatures were recorded. Therefore, a single free field microphone would have supplied sufficient data to predict structure element response.

- 3. The ratio  $P_2/P_1$  (absolute value of peak negative overpressure to peak positive overpressure) decreased as the offset of the aircraft increased for XB-70 missions. The magnitude of the maximum DAF decreased as the ratio  $P_2/P_1$  decreased.
- 4. The DAF spectra obtained using a wave model described by free field signature parameters  $P_1$ ,  $P_2$ ,  $T_1$ , and  $T_2$  (peak positive overpressure, peak negative overpressure, rise time, and time from start of boom to negative peak, respectively) were equal at the 95 percent confidence level to the DAF spectra obtained from digitized free field signatures. The wave model can be used to

predict structure response if these parameters and the characteristics of the structure element are known.

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5. In the analysis of the effects of lateral offset of aircraft, the ratio  $P_2/P_1$  in the recorded free field signatures caused the predominant effect on DAF. The recorded signatures showed little change in rise times ( $T_1$ ) or in durations ( $\tau$ ) for overhead and offset missions for each type of aircraft. Therefore the influence of lateral offset on DAF spectra was limited to the effect of the ratio  $P_2/P_1$ .

The plate and racking response of the one-story and two-story test houses (E-I and E-2, respectively) and of the long span steel frame structure (E-3) to sonic booms generated by the Edwards AFB test flights were analysed. The major findings were as follows:

- 6. Peak plate displacements of three typical walls in the two test houses were less than 0.034 inches for sonic boom overpressures of approximately 2 psf. Racking displacements at the roof line of the northeast corners of Test Houses E-I and E-2 were extremely small (less than 0.0018" for E-I and less than 0.005" for E-2) for sonic booms on the order of 2 psf overpressure.
- 7. Measured displacements of the three typical walls were nearly equal to predicted displacements based on either free field or net pressure signature data. Racking displacements predicted from free field peak overpressures and DAF spectra calculated from free field pressure signatures were in good agreement with measured displacements. The response of the large glass window in E-I was predictable using free field signature data.
- 8. Structure response could be adequately predicted by using peak overpressures and DAF spectra calculated from free field signatures.
- 9. The future SST for peak overpressures of about 2 psf should produce racking displacements of typical houses that will be of similar magnitude, or possibly smaller, than those caused by the XB-70 missions. These racking displacements should be negligible and far less than those required to cause damage.
- 10. No sonic boom damage was observed in the test structures prior to or after the test flights. There were minor shrinkage cracks in the test structures prior to start of test flights. However, no discernible extension or widening of these cracks was observed although observations were made and recorded daily.

11. Damage to properly designed and constructed houses from low magnitude sonic booms is extremely unlikely. Damage should not occur to structure elements such as glass windows from racking motions caused by low magnitude sonic booms.

The supersonic test missions subjected a large number of buildings and structures at Edwards AFB and in communities near Edwards to sonic booms. A survey was made of all glass windows and doors in buildings and structures at Edwards to provide a basis for determining the extent of glass damage caused by the test program. An engineering investigator inspected each complaint received from Edwards and the adjacent communities. The major findings were as follows:

- 12. As the condition of the glass panes at Edwards AFB was determined prior to the test program, the number of damaged panes caused by booms from test missions should be an indicator of glass damage to be expected from future level supersonic flights gene and sonic boom peak overpressures of 2 to 3 psf. The rate was one damage and per 7.9 million boom-pane exposures. This rate was 27 percent of the rate for buildings in communities adjacent to Edwards which were not condition surveyed prior to test missions.
- 13. During Phase I, the IIO,390 glass panes in structures at Edwards were subjected to more booms from test missions than were the 605,000 glass panes in the adjacent communities; however, the aircraft while over Edwards were flying straight courses and then made turns at supersonic speeds over adjacent communities. Some focusing of the boom overpressure (or super booms) may therefore have been produced with peak overpressures greatly exceeding those produced on the Base. As a result, the valid glass damage rate per mission during Phase I was 8.8 times the rate during Phase II when aircraft generally flew straight courses while at supersonic speeds.
- 14. Fifty-eight percent of all incidents of damage for which complaints were received during Phases I and II were listed as possibly caused by sonic booms generated by test program flights. Of these valid incidents, 80 percent were for glass, 5.5 percent for plaster or stucco, 0.0 percent for structural, and 14.5 percent for bric-a-brac or other fallen object damage.

#### RECOMMENDATIONS

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As stated previously, the general objective of the structure response portion of the Edwards Program was accomplished. Measured response of structures and structure elements agreed quite closely with response predicted using free field signature data and computed structure characteristics. The magnitudes of the measured response to sonic booms with peak overpressures in the order of 2 psf were very small. Results of these tests and of others have indicated that damage to properly designed and constructed houses from low magnitude sonic booms is unlikely. However, judging from the large number of damage complaints which have been filed since 1955, some damage must be caused by low overpressure sonic booms.

Factors that could explain the apparent discrepancy between results of tests and actual damage claims received are the possible range of material properties and environmental conditions and also the variation in boom characteristics. Investigations of damage complaints and claims indicate that failure or damage may occur as a result of a combination of factors. Therefore, the problem of damage evaluation does not appear to have an absolute solution. The evaluation of damage claims could be based, however, on a determination of the most probable cause of failure when the factors affecting failure such as the environmental conditions and material properties of the element and the sonic boom loading on the element are known. The application of statistical techniques and engineering procedures to data developed from detailed examinations and engineering evaluations of damage complaints and claims plus selective laboratory testing of damaged elements could be used to establish the most probable cause and amount of damage.

The ultimate objective of the studies of structure response to sonic booms is to understand the mechanism of failure under sonic boom loading so that damage claims can be evaluated and a prediction of future damage can be made with a good degree of reliability. The Edwards Air Force Base tests and others have furnished sufficient data to establish the description of the loading mechanism of sonic booms on structure elements and to predict the dynamic response of structure elements. In order to understand the mechanism of element failure and to determine the most likely cause of damage due to sonic boom load-

ing, data are needed on in situ strengths and modes of failure of structure elements.

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It is, therefore, recommended that the following studies be implemented to obtain the data and knowledge necessary to establish most probable or most likely cause of damage for sonic boom damage claims:

- I. Several cities in the United States should be selected that are or will be overflown by numerous supersonic flights. All complaints of glass damage and major plaster and stucco damage that occur in these selected cities should be investigated in depth by trained and experienced research-oriented engineering personnel to determine the most likely cause of damage. Environmental conditions that might affect the strength of the structure element and thus cause or contribute to the damage should be evaluated. Samples of damaged glass, plaster or plasterboard should be taken, as appropriate, and tested for failure strength.
- 2. Overpressure levels in each of the selected cities should be measured by instruments capable of measuring the N-wave in detail. A number of simpler peak overpressure measuring gages should also be installed. The overpressure measurements are necessary to provide a basis for evaluating damage.
- 3. Structure element populations, at least glass panes, should be determined in the selected cities.
- 4. Statistical methods should be applied in analysing the data to determine probable amounts and types of damage from future supersonic flights.
- 5. A program of laboratory testing to determine average strengths and variations therefrom of in situ glass, plaster, plasterboard and stucco should be developed and implemented. The data and knowledge gained would strengthen the criteria for determining most likely cause of damage.
- 6. A "Guide for Sonic Boom Damage Investigation and Evaluation" should be prepared.

As discussed in Chapters VI and IX, additional analyses of the data recorded by NASA during the Edwards tests for XB-70 and B-58 missions at varying distances from the measuring-recording systems should be performed to de-

termine if trends in the ratios  $P_2/P_1$  (peak positive overpressure to numerical value of peak negative overpressure) and  $T_1/\tau$  (rise time to boom duration) can be established and hence determine the effect of these ratios on structure response. These data could help establish the effective width of the "boom swath" under an aircraft flying at supersonic speed and hence the area subjected to damaging conditions.

#### II. INTRODUCTION

Sonic boom experiments were conducted at Edwards Air Force Base from 4 June to 23 June 1966 (Phase I) and from 31 October 1966 to 17 January 1967 (Phase II). The general objectives of the program were to:

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- I. Evaluate the judgments by human observers of the relative acceptability of sonic booms and noise of different intensities from various types of aircraft.
- 2. Determine the response of typical structures to sonic booms having different signature characteristics and evaluate damage resulting from the program overflights.
- 3. Obtain detailed measurements of sonic boom signatures as functions of the type of aircraft and mode of operation and the atmosphere and ground through which the waves were propagated.
  - 4. Observe the response of animals to sonic booms.

Completion of objectives I, 3 and 4 was the responsibility of other participants in the program and are covered in their reports. The work preformed by John A. Blume & Associates Research Division (JABARD) in fulfilling general objective 2 including the results of analyses of structure response data measured and recorded during the test program and the evaluation of damage reported during the test program are presented in this report.

The test program was designed to subject instrumented structures to sonic booms of different signature characteristics. The aircraft utilized were the XB-70, B-58, and F-104, each of which produces sonic booms of diferent signature du ations. These aircraft were flown at several flight track offsets, altitudes and Mach numbers so as to generate different overpressure levels and signature characteristics.

The JABARD effort involved four major phases of work:

- a. Construction of three test structures and installation of instrumentation;
- b. Recording and reduction of data from test program aircraft missions;

- c. Analyses of structural response data; and
- d. Investigation and evaluation of complaints of damage resulting from the test program supersonic overflights.

A brief summary is presented in this chapter of the construction of the test structures; procedures for instrumentation, data recording and data reduction; analyses of structure response data; investigation of complaints of damage from test program missions; and findings resulting from the analyses of data concerning free field signatures, structure response and damage to structure elements.

#### TEST STRUCTURES

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Two wood frame test house structures were built at Edwards AFB, a two-story house and a one-story house. They were complete homes with all services and standard built-in items. Both were built in accordance with plans obtained from a large housing developer and home builder and are representative of typical contemporary midwestern construction. The houses were furnished and equipped with appliances, drapes, rugs and dishes. Both of the test houses were instrumented to measure and record the loading on and the response of the house and certain elements.

in addition to the two test houses, the Powling Alley on the Base was selected as a representative structure with a long-span roof. Instruments were installed to measure and record the response of the roof structure and the building frame to sonic booms.

During Phase I of the program, a two-story house identical to the two-story test structure at Edwards was built and leased in Lancaster, California. Furnishings and instrumentation were similar to those installed in the two-story test structure at Edwards AFB. A detailed discussion of the test structure is presented in Chapter III.

#### DATA RECORDING AND DATA REDUCTION

Instrumentation was installed in and on the test structures to measure accederation and displacements of the structures and various structure elements, overpressure levels on the exterior and interior of the structures, strain of certain structure elements such as window panes and flanges of the long-span roof girders, acoustical levels at different locations in the test house structures and free field overpressures near the houses; and to provide data on the acoustical and vibrational signals transmitted to the human subjects (in test houses only). In order to make these measurements, three basic types of instruments (transducers) were installed: microphones, accelerometers, and strain gages. Each instrument was selected to be compatible with the characteristics (frequency response and size) of the structural element on which it was mounted.

The instrumentation locations were selected to measure as many critical parameters as possible with the number of channels and types of transducers available. The structural response data recorded during Phase II at test structures E-I, E-2 and E-3 totalled 2160 boom channels for comparable XB-70, B-58 and F-104 flights, 307 boom channels for SR-71 flights and over 300 boom channels of free field data. In addition, 6832 boom channels of acoustic microphone and high frequency accelerometer data were recorded for use by others. In order to acquire this large amount of data in reliable and useable form, detailed coordination, high standards of quality control and careful maintenance of equipment were instituted and followed. The quality of the recordings and recoverability of data recorded during Phase II were extremely high. Practically all data were recoverable with over 90 percent of the records of excellent quality.

The signals generated by the transducers when subjected to sonic booms were recorded on analog magnetic tape by precision recorders. In order to evaluate and analyse the data, the recorded data on the analog tapes were reproduced on oscillographic photo-sensitive paper. The recordings on paper provided a visual record of the pressures, accelerations, etc., produced by the booms and were used to make comparative judgments of the different instrument measurements. The analog data was also converted to digital form so that it could be processed by digital computers. Several different computer programs were developed and used in the analyses of the data.

All pertinent data such as aircraft mission characteristics, instrument locations and calibrations, and summaries of free field signature data were logged daily and punched on data cards to facilitate summarization, printing and distribution of the data. Chapter IV presents a detailed discussion of instrumentation and procedures followed in recording and reducing data.

#### ANALYSES OF STRUCTURE RESPONSE DATA .

The primary purposes of the analyses of the structure response data were to determine and compare the response of test and other structures to sonic booms produced by XB-70, B-58, and F-104 aircraft; to compare predicted response with actual or measured response as determined from the instrumented test structures; and to develop a means of predicting structure response due to sonic booms generated by the future SST based on data from presently available aircraft.

A comprehensive analysis of the response of a structure or structure element to sonic boom overpressure loading involves consideration of the free field pressure wave as it envelopes the structure so as to determine the actual loading on the structure. This procedure is quite complex and time consuming. It was recognized that a method of approximating structure response that was based on free field signature data, thereby eliminating the fabrication of the loading waveform on the structure, would be extremely valuable.

Examination of the pressure signature data showed that the free field pressure signatures were quite similar to the exterior and interior loading pressure signatures. Preliminary analyses of the data indicated that a direct relation between predicted response based on free field data and predicted response based on exterior loading data should exist. As the actual or net loading pressure on a wall or window is the exterior pressure minus the interior pressure it seemed reasonable that a direct relationship between free field and net pressure loading existed. Further analyses showed that the response spectra or Dynamic Amplification Factors (DAF) computed from free field, exterior and net pressure signatures were similar. DAF represents the influence on structure response of the change in the magnitude of the load with respect to time. A DAF spectrum covers !) the natural frequencies for which the response is impulse sensitive (low frequencies), 2) the natural frequencies for

which the response is pressure and duration sensitive (middle frequencies) and 3) the natural frequencies for which the response is only pressure sensitive (high frequencies). Therefore the DAF concept was used in the analyses of the data.

Since the advantages of being able to predict response using free field data were readily apparent, an analysis plan was developed to determine if structural response calculated using free field data was a good approximation of the actual or measured response of elements of the test structures. The comparison of predicted response based on net loading with measured response was also included.

Concurrently with the analyses of structure response, the free field data was studied to determine if the data from the five microphone channels were consistent and to determine the effects on structural response of variations in free field signature parameters. The study included a determination of the relative effects of the different parameters on structural response. If one or two parameters were predominant, a simple gage measuring these parameters could supply adequate information to predict response. It was apparent that a wave model described by free field signature parameters (peak positive overpressure,  $P_1$ , peak negative overpressure,  $P_2$ , rise time,  $T_1$ , and time from start of boom to peak negative overpressure,  $T_2$ ) would be necessary to evaluate the effects of variations in the parameters on DAF spectra. However, DAF spectra calculated from the wave model would have to be equal to spectra obtained from actual digitized free field signatures. The model would make it possible to predict the response of a structure element knowing the free field siganture parameters  $P_1$ ,  $P_2$ ,  $T_1$ ,  $T_2$ , and the structural properties of the element. The wave model was also used to study the effect on structure response of variations in free field siganture parameters beyond those in the recorded data and, therefore, provide a basis for prediction of structure response to sonic booms produced by larger aircraft such as the SST.

A generalized DAF spectrum was also developed, with which it is possible to approximate response knowing only peak positive overpressure and signature duration and the structural properties of the element. This spectrum is useful when preliminary results are required. For more devailed answers the wave model with values of the required parameters may be used.

in summary, a direct relationship between structural response and free field signatures is extremely valuable because free field signatures are easier and less costly to obtain than structural response data; free field signature parameters such as peak overpressures, rise time and duration can be predicted using available procedures; and the effect of the variations of the free field signature parameters have been investigated and the results known.

In the analysis of the response data, Phase II data were used for the major portion of the structural analysis, with Phase I data used where appropriate. Phase I missions were analysed only where the data could be used to validate findings from Phase II missions. This procedure was followed because the Phase II data contained measurements of comparable overhead and offset missions of XB-70, B-58, and F-104 aircraft (lown within minutes of each other, while Phase I data contained measurements of only three X3-70 missions, two of which were test program missions.

#### INVESTIGATION OF COMPLAINTS OF DAMAGE

During the planning phases of the Edwards Test Program it became evident that many of the supersonic missions would subject a large number of buildings and structures at Edwards AFB and in communities near Edwards to sonic booms. Based on past experience, some damage was expected to occur. Therefore, to provide a fairly reliable basis for determining the extent of glass damage caused by the test program, a survey was made of all glass windows and doors in buildings and structures at Edwards. Provisions were also made to have an engineering investigator inspect each complaint received from Edwards and the adjacent communities. The findings of the engineering investigator were evaluated and then compared with the missions flown.

# SUMMARY OF FINDINGS RESULTING FROM THE ANALYSES OF EDWARDS AFB DATA

The findings resulting from the analyses of free field signature data, effects of free field signature parameters on dynamic amplification factors, test structure plate response, test structure racking response,

structure damage, generalized DAF spectrum, and damage complaint investigations are presented below. Detailed discussions can be found in the respective chapters. Analyses were based on data that was recorded at Edwards AFB and obtained from seventeen comparable XB-70, B-58, and F-104 missions that were flown within minutes of each other.

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#### Analyses of Free Field Signature Parameters (Chapter V)

The free field signature parameters were analysed to determine if the channels were statistically equal or if the measured values of a parameter were independent of the channel on which it was recorded; to determine which parameters most influenced the Dynamic Amplification Factor (DAF); and to determine the number of samples necessary for studies of structure response data. The analytical techniques were such that the findings are stated with a 95 percent confidence level; that is, there is a 95 percent probability that the findings are correct. Following are the findings resulting from these analyses:

- I. For the XB-70 missions the peak negative overpressure,  $P_2$ , and the ratio of the absolute value of peak negative overpressure to peak positive overpressure,  $P_2/P_1$ , were not independent of the channel on which they were recorded. All other parameters studied (positive overpressure,  $P_1$ , rise time,  $T_1$ , and time from start of boom to negative peak,  $T_2$ ) were independent of the channels.
- 2. For the B-58 missions  $P_2$  was not independent of the channel on which it was recorded. All other parameters studied  $(P_1, T_1, T_2, \text{ and } P_2/P_1)$  were independent of the channels.
- 3. For the F-104 missions all parameters  $(P_1, P_2, T_1, T_2, \text{ and } P_2/P_1)$  were independent of the channels on which they were recorded. A single channel would have been adequate to measure free field signatures.
- 4. The magnitude of  $P_1$  and of the absolute value of  $P_2$  decreased as the lateral offset and/or altitude of the aircraft increased.
- 5. The ratio  $P_2/P_1$  decreased as the offset of the aircraft increased for XB-70 missions.

- 6. There was little difference between rise times  $(T_i)$  of overhead and offset missions for each type of aircraft.
- 7. There was little d'insience between times from start of boom to negative peak  $(T_2)$  of overhead and offset missions for each type of aircraft.
- 8. The Dynamic Amplification Factors (DAF) computed from free field signatures were independent of the channel the signatures were recorded on. Therefore, a single microphone would have been sufficient to evaluate DAFs.
- 9. The magnitude of the maximum DAF decreased as the ratio  $P_2/P_1$  decreased.
- 10. The natural frequency at which the maximum DAF occurred was a function of the time from start of boom to negative peak  $T_2$ . As  $T_2$  increased, the maximum DAF occurred at a lower natural frequency.
- II. Sonic booms from large aircraft such as the XB-70 and the future SST will affect a greater range of structure elements than will sonic booms from smaller aircraft such as the B-58 and the F-104.
- 12. The number of missions needed in the study of the response of structure elements varied depending on the degree of precision in the results and on the confidence level.

# Effects of Free Field Signature Parameters on Dynamic Amplification Factors (Chapter VI)

The effects of free field signature parameters on DAF were studied in order to develor a wave model from free field signature parameters (overpressures, rise time, duration) that could be used to make an accurate evaluation of DAF, and to analyse the effects on DAF of values of free field signature parameters beyond the range of the recorded data. The findings of this study are presented below:

- I. The DAF spectra obtained using a wave model described by free field signature parameters  $P_1$ ,  $P_2$ ,  $T_1$ , and  $T_2$  were equal at the 95 percent confidence level to the DAF spectra obtained from digitized free field signatures.
  - 2. Using the derived wave model it was found that:

a. the magnitude of the maximum DAF decreased as the ratio  $P_2/P_1$  decreased,

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b. the magnitude of the maximum DAF increased as the ratio  $T_{1}/\tau$  increased, and

- c. the magnitude of the maximum DAF decreased as the damping coefficient increased.
- 3. In the analysis of the effects of lateral offset of aircraft, the  $P_2/P_1$  in the recorded free field signatures caused the predominant of the change in rise times or in durations ( $\tau$ ) for overhead and offset missions for each type of aircraft. Therefore, the influence of lateral offset on DAF spectra was limited to the effect of the ratio  $P_2/P_1$ .

#### Analysis of Structural Response Data - Plate Response (Chapter VII)

The analysis of structural response data was divided into two sections: plate response and racking response. Plate response was defined as the normal deformation of individual structure elements and was primarily of a bending mode. Racking response was defined as the deformation of the structure as a whole and was primarily of a shearing mode. The findings from the analyses of plate response are presented under three headings: A. Wall Plate Response in Test Houses E-1 and E-2; B. Window Plate Response in Test House E-1; and C. Response of Roof Frame of the Bowling Alley, E-3.

#### A. Wall Plate Response in Test Houses E-1 and E-2:

Predicted displacements were computed and compared with measured displacements for three walls. The effects of flight track offset, Mach number and aircraft vector on plate response were investigated. In addition, the results of Phase II tests were compared with those from previous tests. The findings of these ar Jyses were as follows:

I. Peak plate displacements of three typical walls in the two test houses were less than 0.034 inches for sonic boom overpressures of approximately 2 psf. Results from Phase I were of similar magnituds.

- 2. The DAF spectra curves determined from the free field, exterior and net pressure loading signatures were in significant agreement for structure element natural frequencies from 10 to 40 cps.
- 3. There was an indication that plate response may decrease with an increase in offset for flights of the same altitude and Mach number.
- 4. Plate response decreased slightly with an increase in Mach number for flights of the same altitude and offset. An increase in Mach number from 1.8 to 2.5 for overhead flights of the XB-70 caused a decrease in plate response of approximately 10 percent.
- 5. Peak displacements of a wall subjected to nearly side-on vectors (flight track nearly parallel to the wall surface) were 50 percent of the displacements of an equivalent wall subjected to nearly head-on vectors (flight track nearly perpendicular to the wall surface). Similar results were also found at White Sands.<sup>3</sup>
- 6. Measured displacements of three typical walls compared very well with predicted displacements based on either free field or net pressure signature data. For predicted displacements computed using free field data, the average ratios of predicted to measured displacements were equal to 1.03, 1.05, and 1.00 at the 95 percent confidence level for the BRI-1, DR-2, and BRI-2 walls respectively. For predicted displacements computed using net pressure data, the average ratios of predicted to measured displacements were equal to 1.00 at the 95 percent confidence level for both the BRI-1 and DR-2 walls. These findings applied to comparable overhead missions of XB-70, B-58, and F-104 aircraft (flights flown within a few minutes of each other), and to XB-70 missions with different offset and Mach number.
- 7. Plate response could be adequately predicted using peak overpressures and DAF spectra calculated from free field signatures.

#### B. Window Plate Response in Test House E-1:

Predicted displacements based on peak overpressures and DAF spectra calculated from free field signatures were corputed and compared with the measured displacements. The findings are as follows:

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- 2. Large glass windows such as the one in E-I garage respond to a sonic boom loading primarily in the fundamental mode of vibration. A minor excitation of the second symmetrical mode also occurs.
- 3. For the E-I garage window, the maximum stress determined from the strain data for the missions investigated was 790 psi. The corresponding theoretical predicted stress was 980 psi.
- 4. Greater response of the E-I window was measured for B-58 missions than for XB-70 and F-104 missions. This was expected, since the DAF spectra curves obtained from B-58 signature data peaked at about 5 cps and the frequency of the fundamental mode of vibration of this window was approximately 5.7 cps.
- 5. Window plate response could be adequately predicted using heak overpressure and DAF spectra calculated from free field signatures. For windows located on the trailing vector side of the structure, the free field data was reduced by an appropriate factor to account for orientation and geometry of the structure.

#### C. Response of the Roof Frame of the Bowling Alley, E-3:

One of the main steel roof girders (118 foot span) was instrumented to record strain in the bottom flange, vertical acceleration of the girder and roof structure and overpressures on the roof. These records were analysed and the following findings resulted:

- 1. The maximum stress due to sonic boom loading in the bottom flange of the building frame at midspan was approximately 450 psi.
- 2. Peak vertical displacements of the center of the building frame for XB-70 mission 12-2 and B-58 mission 12-1 were 0.19" and 0.'1" respectively. Free field peak overpressures near E-2 for these missions were 2.19 psf and 2.39 psf respectively.

- 3. The shape of net overpressure signatures on the root of the Bowling Alley measurably differed from those for typical free field N-waves.
- 4. DAF spectra determined from net overpressure signatures differed from spectra determined from typical free field N-waves.

# Analysis of Structural Response Data - Racking Response (Chapter VIII)

Racking response was defined as the deformation of the structure as a whole and was primarily of a shearing mode. The findings resulting from the analyses of racking response data are presented under A. Test Houses E-1 and E-2, and B. Bowling Alley E-3.

#### A. Test Houses E-1 and E-2:

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Predicted response using free field peak overpressures a DAF spectra computed from free field signatures was compared with measured racking response. The results of the Phase II tests were also compared with those from Phase I 19,20 and White Sands<sup>2</sup> tests. The following findings resulted from these analyses:

- i. Racking displacements at the roof line of the northeast corners of Test Houses E-1 and E-2 were extremely small (less than 0.0018" for E-1 and less than 0.005" for E-2) for sonic booms on the order of 2 psf overpressure.
- 2. Racking displacements of E-I and E-2 recorded during Phases I and II were of similar magnitudes for similar overpressures.
- 3. The racking displacements of E-1 and E-2 recorded during Phase II were of magnitude similar to displacements obtained at White Sands  $^2$  for structures of similar construction and for similar overpressures.
- 4. Racking displacements predicted from free field peak overpressures and DAF spectra calculated from free field pressure signatures were in good agreement with measured displacements. For both the east-west and north-south racking of House E-I the average ratio of the predicted to measured displacement was equal to 1.0 at the 95 percent confidence level for comparable XB-70, B-58, and F-104 missions. These findings applied to both head-on and side-on vectors.

5. Racking response could be adequately predicted by using peak overpressures and DAF spectra calculated from free field signatures.

6. The future SST, for peak overpressures of about 2 psf, should produce racking displacements of typical houses that will be of similar magnitude, or possibly smaller, than those caused by the XB-70 missions. These racking displacements should be negligible and far below those required to cause damage.

#### B. Bowling Alley E-3:

As free field signatures were not measured near the Bowling Alley, a comparison could not be made of displacements predicted from free field data versus measured displacements. The magnitude of the racking displacements were less than the maximum measured for E-2.

#### Structure Damage (Chapter IX)

A review of the results of the tests at Edwards and tests by others as applied to damage from sonic boom of low magnitudes (in order of 2 psf) was made. The findings resulting therefrom are presented below:

- 1. Damage to properly designed and constructed houses from low magnitude sonic booms is extremely unlikely.
- 2. Damage should not occur to structure elements such as glass windows from racking motions caused by low magnitude sonic booms.
- 3. Further data is needed on in situ strengths and modes of failure of structure elements in order to determine the most likely cause of damage.

#### Generalized DAF Spectrum (Chapter X)

A generalized DAF spectrum was derived for use in predicting the response of a structure element when only the nominal overpressure and the type of aircraft are known. The findings are as follows:

- i. A generalized DAF spectrum was obtained by studying the asymptotic behavior of DAF spectra computed from digitized free field signature data.
- 2. When the nominal pressure signature of a sonic boom is known, the generalized DAF spectrum can be used to predict the nominal response of a known structure element.
- 3. The magnitudes of the generalized DAF spectrum for different durations (Figure 10-1) were:

Duration of Boom in Seconds	Range of Natural Frequencies in cps	Generalized DAF Magnitude
0.5	0.8 to 50	2.0
0.4	1.1 to 50	2.0
0.3	1.5 to 50	2.0
0.2	2.1 to 50	2.0
0.1	4.4 to 50	2.15

4. If a DAF spectrum is desired that is more detailed than the generalized DAF spectrum, and if the free field signature parameters  $(P_1, P_2, T_1, T_2)$  have been measured, a DAF spectrum can be computed from the wave model as described in Chapter VI.

#### Damage Complaint Investigations (Chapter XI)

To provide a fairly reliable basis for determining the extent of glass damage caused by the test program, a survey was made of all glass windows and doors in buildings and structures at Edwards. Provisions were also made to have an engineering investigator inspect each complaint received from Edwards and the adjacent communities. The findings resulting from analyses made of the survey data, complaint investigations and test flight data are presented below:

1. The rate of vaild glass damage in Edwards AFB buildings, all of which had been condition surveyed prior to the test program, was 0.127 panes damaged per million boom-pane exposures or 27 percent of the rate for build-

ings in communities adjacent to Edwards which were not condition surveyed prior to test missions.

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- 2. During Phase I, the IIO,390 glass panes in structures at Edwards were subjected to more booms from test missions than were the 605,000 glass panes in the adjacent communities; however, the aircraft while over Edwards were flying straight courses and then made turns at supersonic speeds over adjacent communities. Some focusing of the boom overpressure (or super booms) may therefore have been produced with peak overpressures greatly exceeding those produced on the Base.
- 3. During Phase I, 90 percent of the incidents of valid glass damage (engineering investigator determined damage could have been caused by sonic toom) were attributable to B-58 missions. The ramaining IO percent were apparently due to F-104 missions.
- 4. The valid glass damage rate per mission during Phase I was 8.8 times the rate during Phase II when aircraft generally flew straight courses while at supersonic speeds.
- 5. The number of complaints received decreased from 61 during Phase I to eleven during Phase II. This large decrease in number of complaints can be attributed to two factors: a) the B-58 aircraft made turns and other maneuvers at supersonic speeds over several communities adjacent to Edwards AFB during Phase I, and b) during Phase II the XB-70 flew supersonically on a relatively straight course over a few of the cities adjacent to Edwards.
- 6. For all incidents of damage recorded during Phases I and II, 60.5 percent were for glass damage.
- 7. Fifty-eight percent of all incidents of damage received during Phases I and II were listed as valid. Of these valid incidents, 80 percent were for glass, 5.5 percent for plaster or stucco, 0.0 percent for structural, and 14.5 percent for bric-a-brac or other fallen object damage.
- 8. Glass damage was repaired or the broken glass removed for 55 percent of the glass damage incidents before the engineering investigator could investigate the alleged damage and hence, the validity of all glass damage could not be definitely established.

- 9. Damaged glass panes ranged in size from 1.3 square feet to 82.5 square feet. 2.7, 43.2, 43.2, and 10.9 percent of the incidents of damage occurred in the 0-2, 2-9, 9-40 and over 40 square feet size groups respectively.
- 10. No senic boom damage was observed in the test structures prior to or after the test flights. There were minor shrinkage cracks in the test structures prior to start of test flights. However, no discernible extension or widening of these cracks was observed although observations were made and recorded daily.
- II. As the condition of the glass panes at Edwards AFB was determined prior to the Test Program, the number of damaged panes caused by booms from test missions should be a reliable indicator of valid glass damage to be expected from future level supersonic flights generating sonic boom peak overpressures of 2 to 3 psf. The rate was one damaged pane per 7.9 million boom-pane exposures.
- 12. A large percentage (from 51 to 84 percent) of future valid incidents of damage from sonic boom should be for glass.

#### PROJECT ORGANIZATION

The work described in this report was performed under the general direction of the National Sonic Boom Evaluation Office, United States Air Force. The JABARD project group was composed of R. L. Sharpe, Project Manager; Dr. J. A. Blume, Senior Technical Consultant to the group; L. A. Lee, Project Field Manager; G. Kost, Senior Research Engineer; J. Prouix, Statistical-Research Engineer; K. F. Schopp, Head, Computer Support Services; R. E. Monroe, Research Engineer; R. F. Runge, Research Engineer - Damage Investigations; and W. W. Powers, Liaison and Photography.

Major subcontractors supplied the following services:

AETRON Division of Aerojet General Corporation - instrumentation engineers and technicians for operating instrumentation during Phase II.

The Boeing Company, Airplane Division - furnished pressure microphone systems on exteriors of E-I and E-2, and personnel to install and operate instrumentation - Phase II.

Lockheed California Company - furnished pressure microphone systems for measuring and recording boom signatures under NASA supervision, and personnel to install and operate equipment - Phase II.

Datucraft, Inc. - furnished instrumentation for House L-2, and personnel to install and operate systems - Phase I. Furnished high-frequency accelerometer systems for Houses E-1 and E-2 - Phases I and II. Dr. J. H. Wiggins was a technical consultant during Phase I.

#### PRESENTATION OF REPORT

A summary of the major findings was presented in Chapter I. This chapter presented a brief description of the test program and test structures, the methods of analysis used, and a summary of all findings resulting from the work. The following chapters will present, in order, construction of the test structures, description of instrumentation and procedures followed in recording and reducing data, analysis of free field signatures, effects of fine field signature parameters on response spectra or Dynamic Amplification Factors (DAF), test structure plate apponse, test structure racking response, structure damage, generalized DAF spectrum, and damage complaint investigations. A bibliography and glossary of terms then follow. The report terminates with appendices containing details of the structural and statistical principles utilized, instrumentation details, calibration procedures, aircraft mission logs for Phases I and II, typical instrumentation log, summary of free field signature data, a typical analog tape log and derivations of certain formulae.

#### III. TEST STRUCTURES

The test facilities at Edwards Air Force Base consisted of two test house structures located south of the main runway and a bowling alley located about two miles northwest of the test house area. In addition, a test house in Lancaster was leased for Phase I testing. The types of test house structures to be constructed and instrumented were selected after review of many different house plans. Two houses were selected to be built on the Base, a two-story house (National Homes Model 8603), and a one-story house (Model 9855). These two models have been mass produced and constructed in the midwest, and a survey of the midwest area indicated that these homes were typical of contemporary midwestern construction.

Model 8603 is a two-story home with four bedrooms, two and one-half baths, living room, dining room, kitchen and family room with a total living area of 1,905 square feet. Model 9855 is a one-story home with three bedrooms, two baths, living room and kitchen-dining-family room with a total living area of 1,205 square feet.

The structures were built on an expedited schedule. Authorization to proceed with procurement was received on 18 April 1966. Contract documents were then prepared, competitive bids taken, and notice to proceed with construction of two houses, one Model 8603 and one Model 9855 at Edwards AFB was issued on 24 April 1966. Figure 3-la shows the Plot Plan for the two structures at Edwards.

A two-story house, identical to the two-story house at Edwards, was built and leased in Lancaster, California. Authorization to proceed was issued on 18 April 1966, a lease was signed on 30 April, and construction started I May 1966. Figure 3-Ib shows the location of the test structure in Lancaster.

Blume representatives monitored and inspected the construction of test structures at Edwards Air Force Base and Lancaster. The basic construction materials are listed in Table 3-1. The construction of the houses at Edwards AFB included the required extensions of sewer, water and butane gas services, construction of concrete driveways and sidewalks, and other minor work necessary for installation and operation of test equipment. All test house construction was completed on I June 1965. Construction schedules for the three structures are listed in Table 3-2. Photographs of the one-story house, E-1, and the two-story house, E-2 at various stages of construction are shown in

Figures 3-2 through 3-4. Figure 3-5 shows the completed house at Lancaster, L-2. All houses were furnished and equipped with appliances, furniture, drapes, rugs, dishes, etc. The test houses were constructed in accordance with the drawings in Figures 3-7 through 3-15.

In addition to the three test houses, the Bowling Alley on the Base was selected as a structure with a representative long-span roof. Instruments were installed to measure and record the response of the roof structure and the building frame to sonic boom. The Bowling Alley (E-3) is shown in Figure 3-6. The structural frame is composed of three steel rigid frames plus column-beam framing for the north wall. The roof and exterior walls are fluted steel decking. Details of the framing are shown in Figure 3-16.

The instrumentation of the test structures is discussed in the following chapter.

#### TABLE 3-1

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# CONSTRUCTION MATERIALS HOUSES E-1, E-2, and L-2

Mud Sills Pressure Treated Foundation Grade

Redwood

Floor Joists Douglas Fir Construction Grade

Sub Floor 5/8" Plyscore Plywood

Trusses 2" x 4" "Gangnail" Wood Trusses

Wallboard 1/2" U.S. Gypsum

Studding Standard and Better Douglas Fir

Roof Sheathing I"  $\times$  6" Standard and Better Douglas Fir

Glass Double Strength Libby-Owens-Ford and

Pittsburg Plate Glass

Insulation 3-1/2" Owens-Corning Fiberglass with

Aluminum Foil One Face

Roof Shingles Asphalt 235#, U.S. Gypsum

All Concrete Local Aggregate 5 Sacks of Cement per

Yard

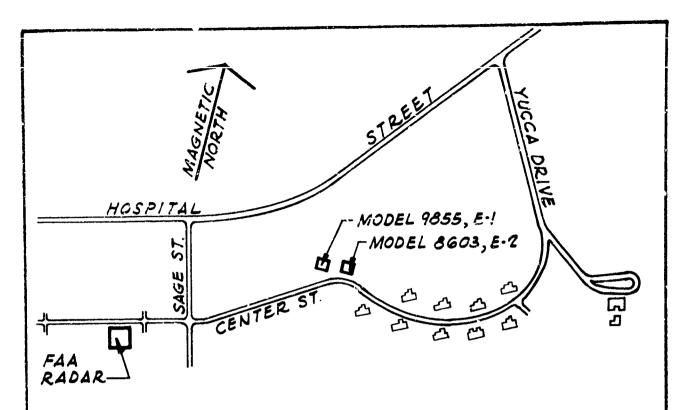
Siding Ship-lap Redwood

TABLE 3-2

ACTUAL CONSTRUCTION SCHEDULES

	House E-I	House E-2	House L-2
Notice to Proceed	<u>1966</u> 24 April	<u>1966</u> 24 April	<u>1966</u> I May
Foundation Concrete Placed	April	26 April	3 May
Floor Framing Started	73 x 1	27 April	7 May
Rough Plumbing Started	27 April	28 April	7 May
Brick Work Completed	2 May	3 May	10 May
Windows Installed	3 May	3 May	13 May
Completed Roofing	5 May	6 May	13 May
Siding Completed	6 May	6 May	14 May
Wall Board Completed	8 May	9 May	15 May
Cabinets Installed	II May	II May	16 May
Doors, Trim, Finish Hardware	12 May	12 May	16 May
Tile Work Completed	15 May	15 May	22 May
Utilities Connected	17 May	17 May	23 May
Finished Plumbing	18 May	18 May	25 May
Concrete Drive, Walks	20 May	21 May	21 May
Preliminary Final Inspection	26 May	26 May	28 May
Final Acceptance	l June	l June	l June

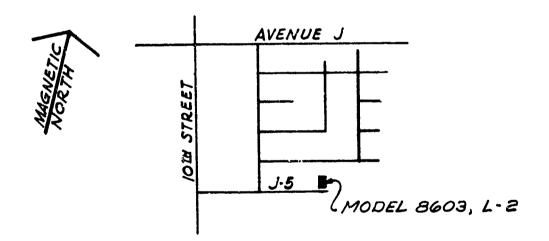
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# PLOT PLAN FOR STRUCTURES E-1 & E-?

SCALE: 500'

FIGURE 3-1a



# PLOT PLAN FOR STRUCTURE L-2 NO SCALE FIGURE 3-16

PLOT PLANS FOR STRUCTURES E-1, E-2 & L-2

FIG.

3-1



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FIGURE 3-2
TWO-STORY HOUSE E-2 START OF WALL FRAMING



FIGURE 3-3

TWO-STON: HOUSE E-2 START OF ROOFING

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FIGURE 3-4
TEST HOUSES E-1 AND E-2

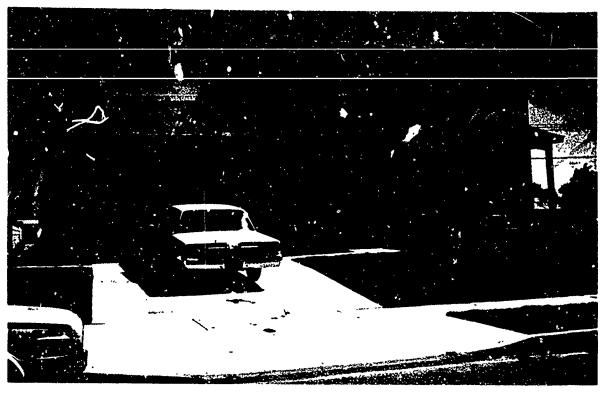
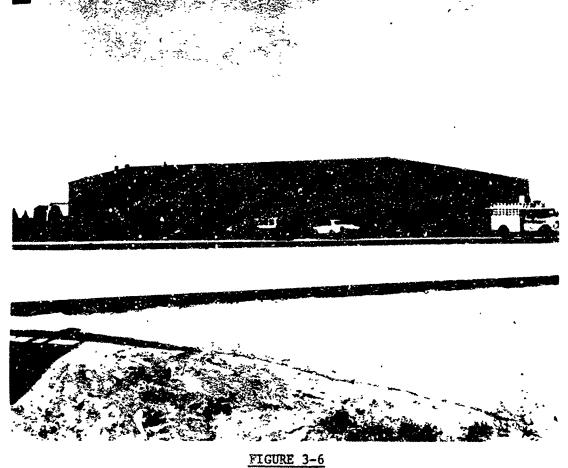


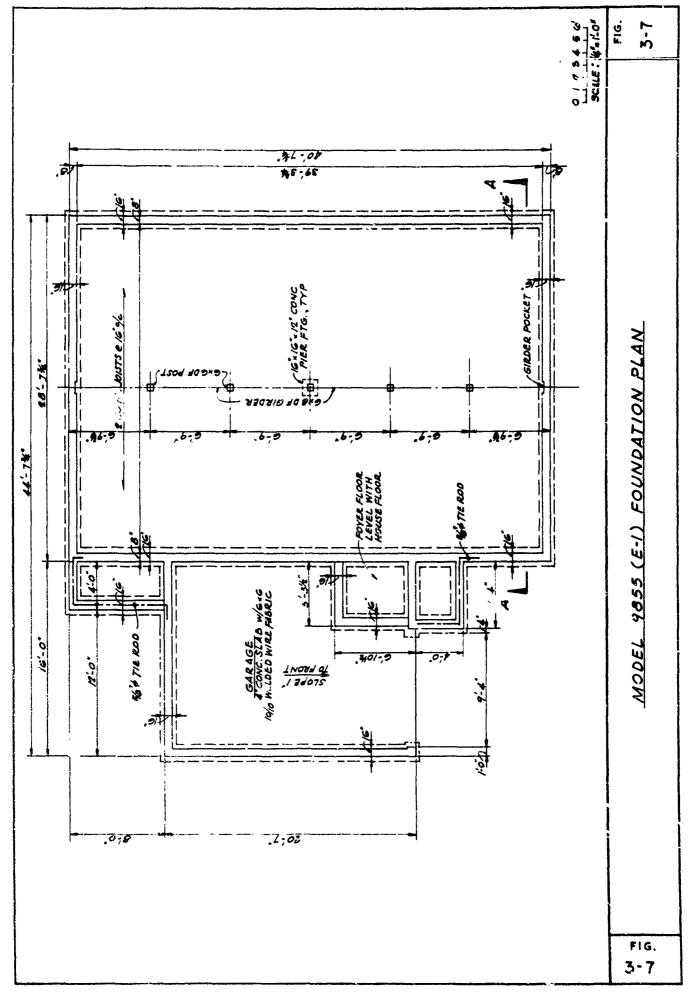
FIGURE 3-5

# TWO-STORY HOUSE L-2



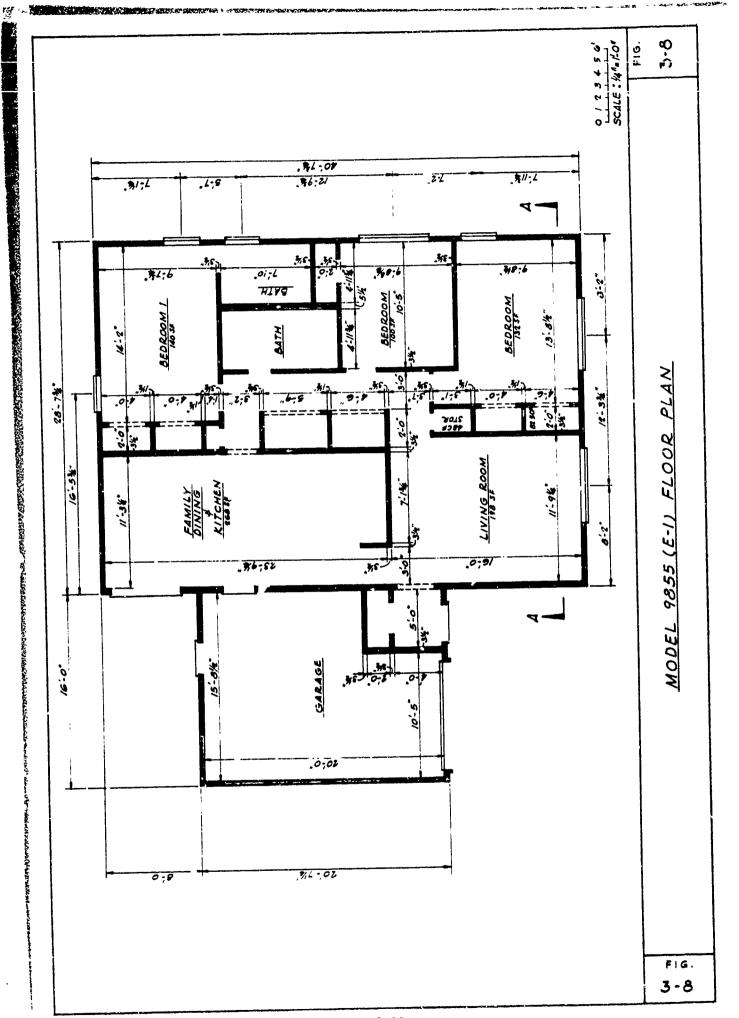
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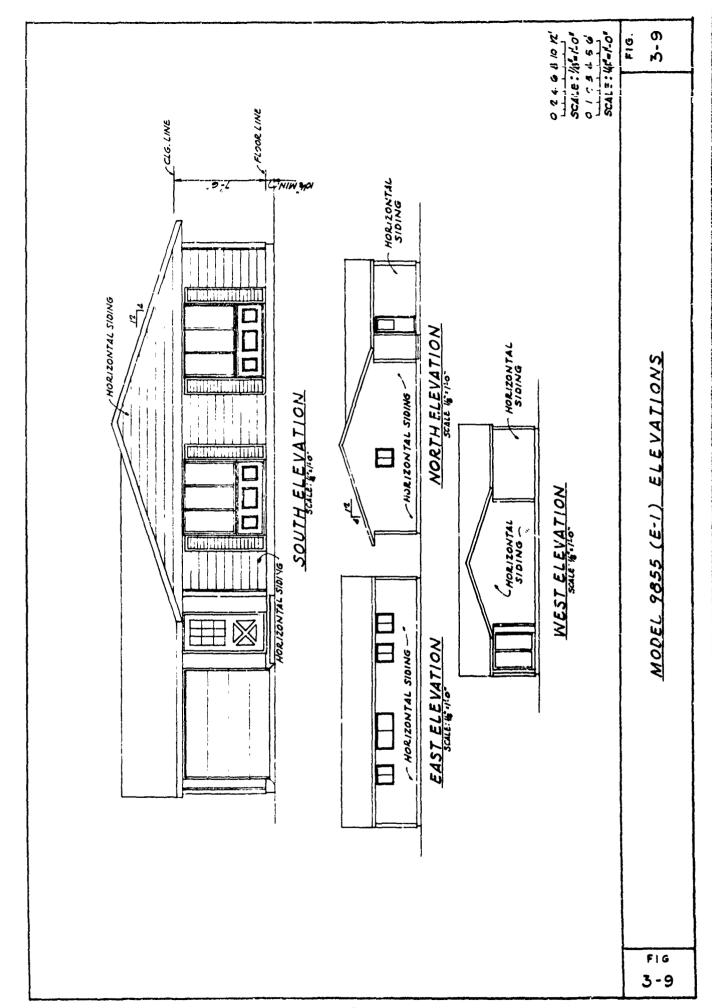
BOWLING ALLEY E-3, EDWARDS AFB

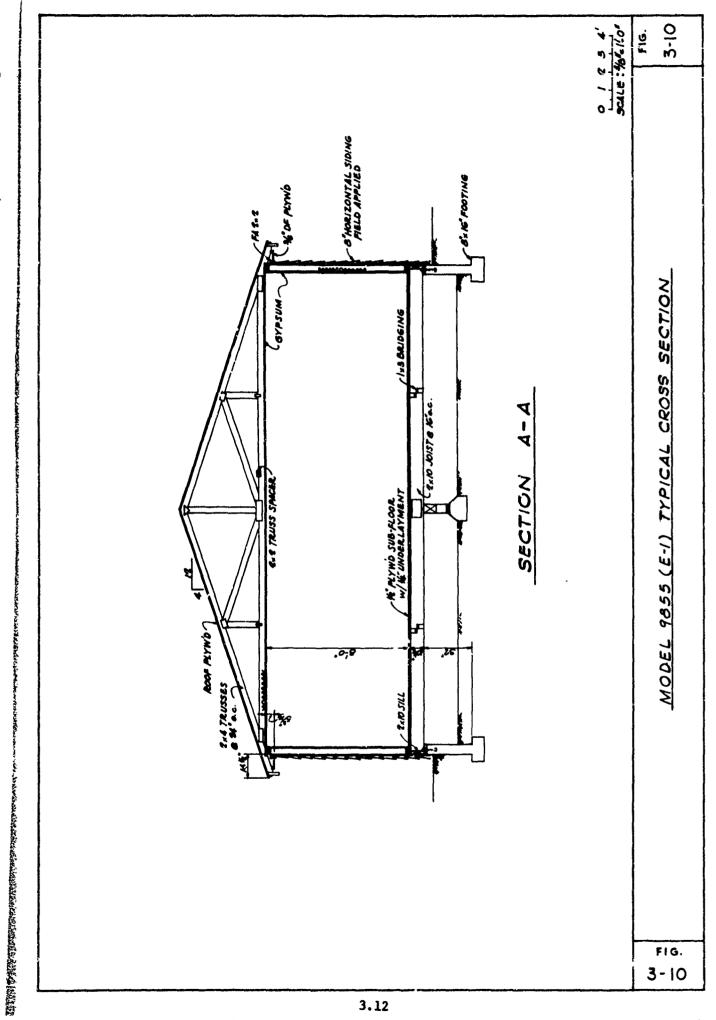


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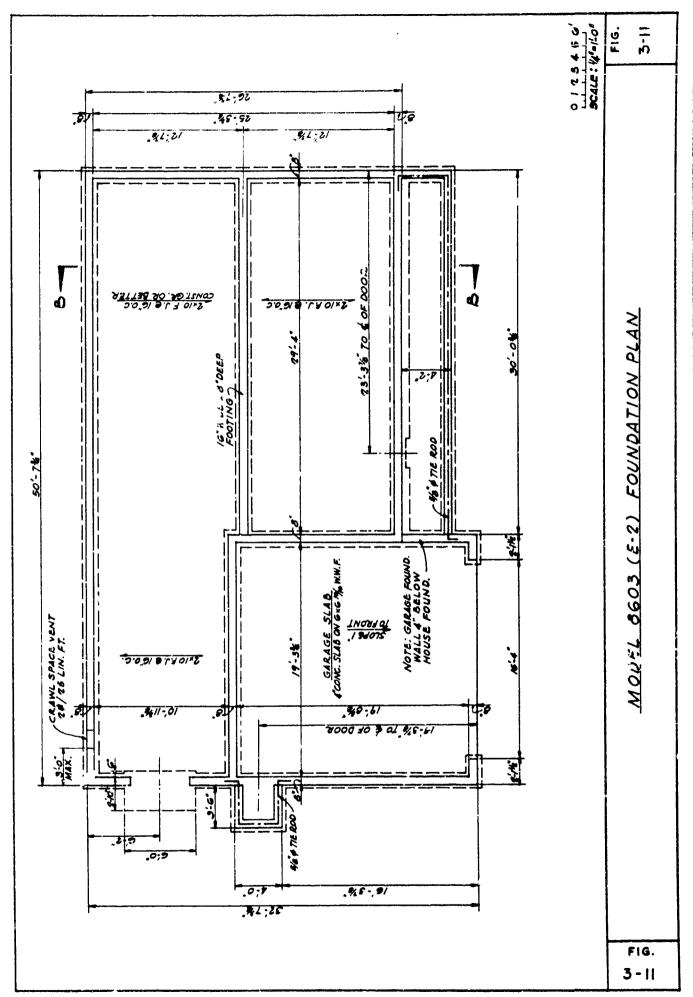
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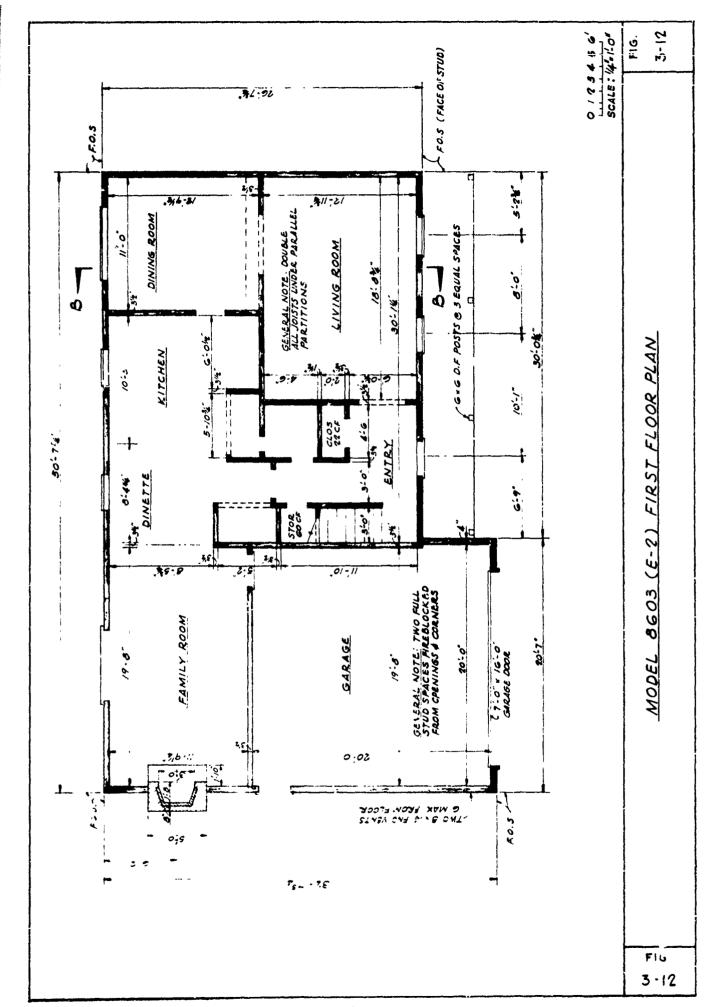
3.12



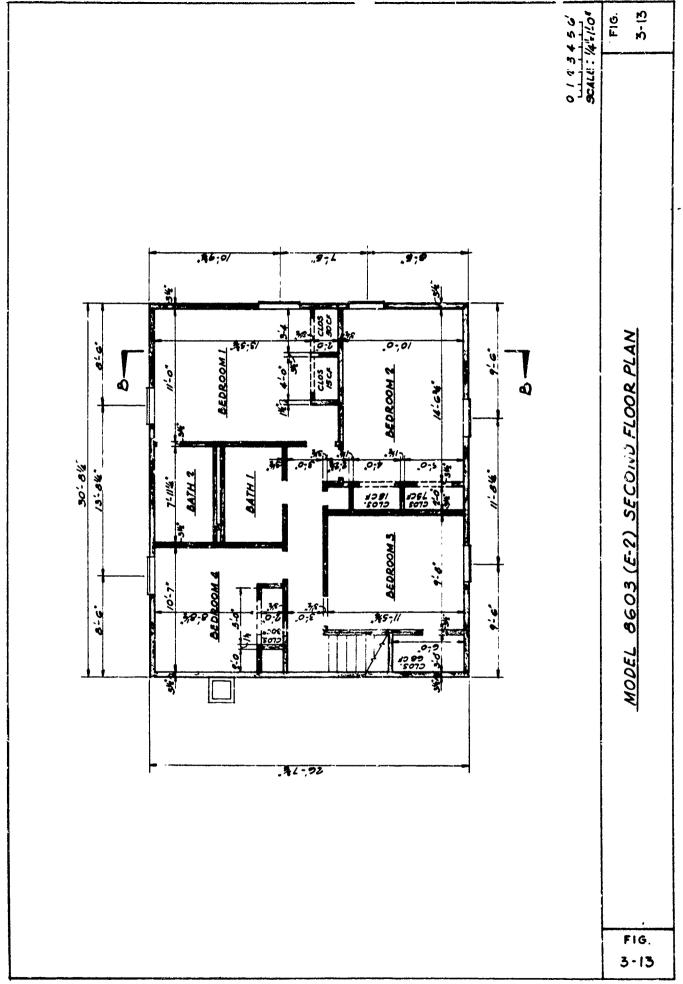
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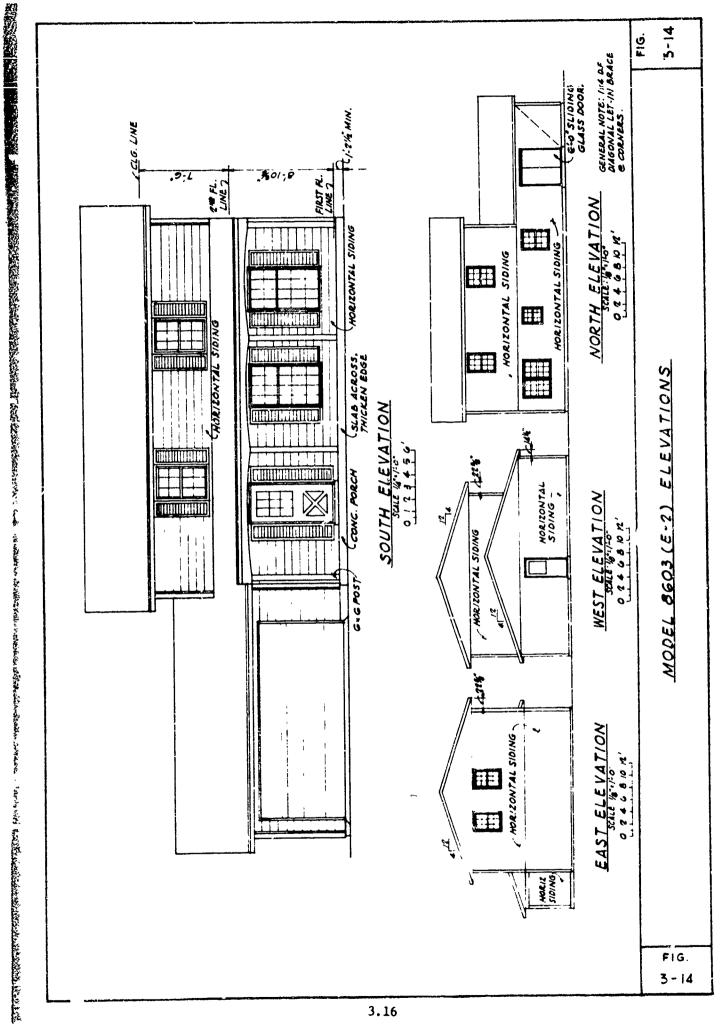
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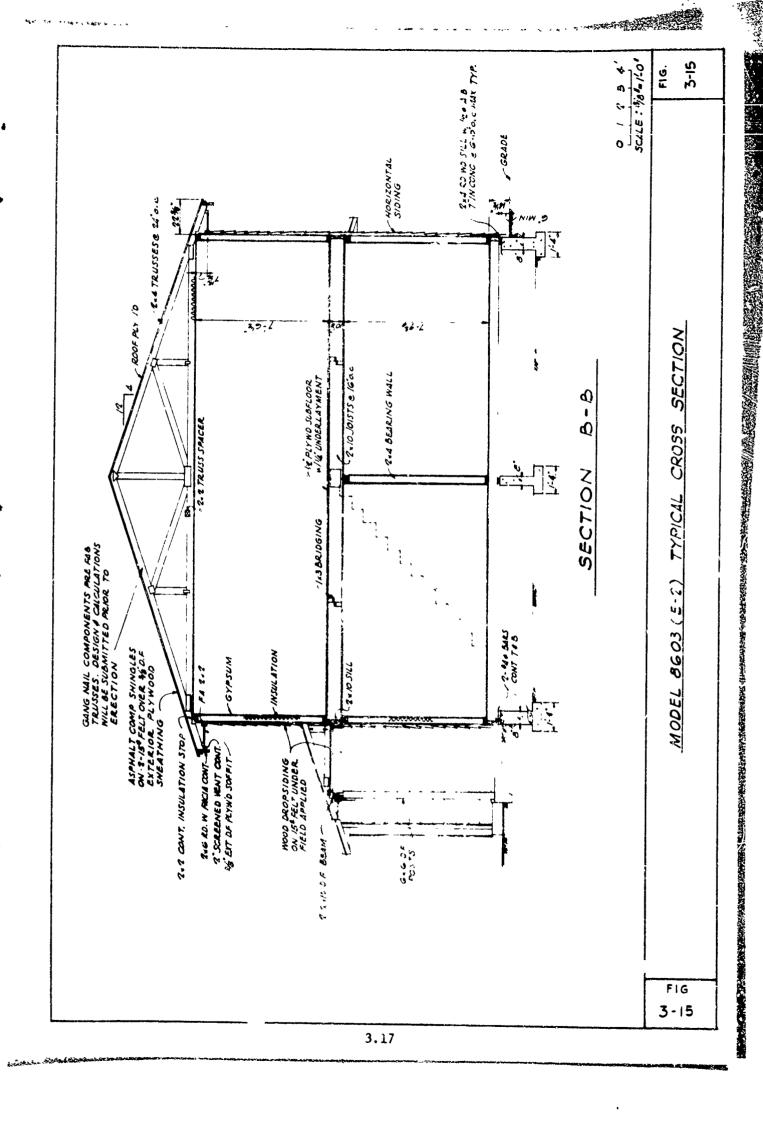


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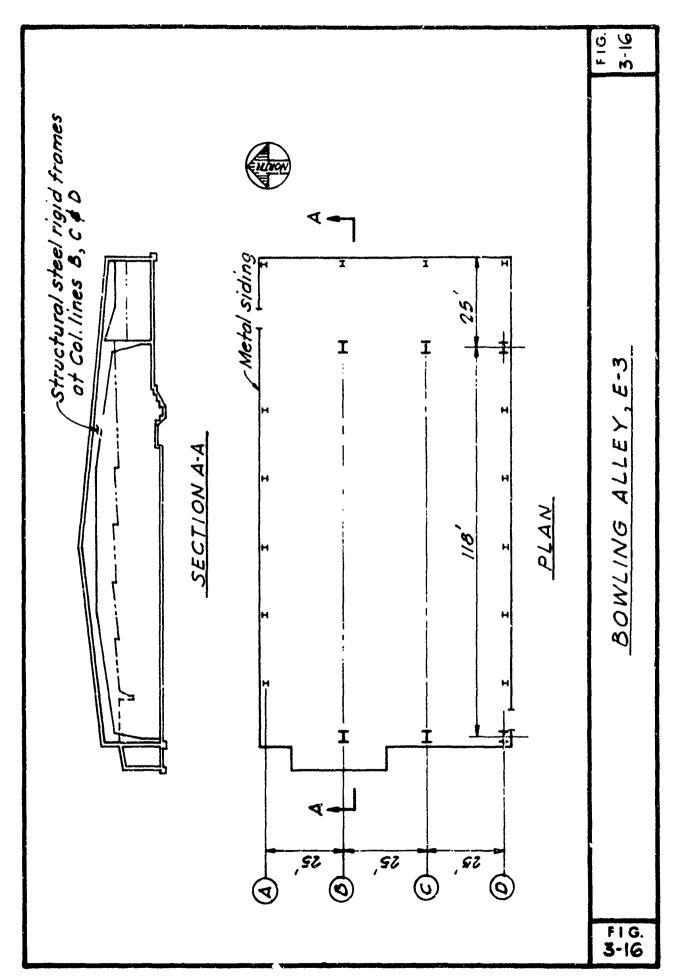


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#### IV. DATA RECORDING AND DATA REDUCTION

The facilities at Edwards Air Force Base were comprised of two test house structures; a one-story house, E-1, and a two-story house, E-2; and a third test structure, a Bowling Alley, E-3, which was located about two miles northwest of the test house area (Figure 4-1). In addition a test house structure, L-2, was leased in Lancaster for Phase I testing. The construction of the test houses has been covered in the previous chapter. Free field sonic boom signatures were recorded by six microphones located in a cruciform array 100 feet north of house E-2 (Figure 4-2).

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Three basic types of measuring instruments (transducers) were installed; microphones, accelerometers, and strain gages. Microphones were used to measure overpressures at ground level near the instrumented structures (free field signatures) and to measure exterior and interior overpressures on structure elements (loading signatures). Accelerometers and strain gages were used to measure the response of the structures and selected structure elements. Each instrument was selected to be compatible with the characteristics (frequency response and size) of the structure element on which it was mounted.

The signals generated by these transducers when subjected to sonic booms were recorded on analog magnetic tape by precision tape recorders. The recordings were reviewed shortly after each mission and minor modifications were made in the instrumentation when required. The quality of recordings and recoverability of data recorded during Phase II were extremely high. Practically all data were recoverable and over 90 percent of the records were of excellent quality.

#### DATA RECORDING

During Phase I, structural data were recorded in the four test structures, E-I, E-2, E-3 and L-2. All instrumentation except high frequency accelerometers was furnished, installed and operated in E-I, E-2 and E-3 by NASA. High frequency accelerometers in E-I and E-2, and all instruments in L-2 were furnished, installed and operated by Datacraft, Inc., under subcontract to John A. Blume and Associates Research Division (JABARD).

The arrangement of the instrumentation was modified for Phase II of the program to increase the effectiveness of the information obtained. The most important changes were the addition of loading microphones on the outside of houses E-I and E-2 and additional audio microphones inside E-I and E-2. During Phase I, the boom intensities and structural reactions at the Lancaster house, L-2, were often masked by natural phenomena due to the large lateral displacement of the aircraft and generally prevailing windy conditions. Therefore, measurements were not recorded at L-2 after Phase I because of the minimal information obtained.

AETRON Division, Aerojet-General Corporation, under subcontract to JABARD, operated instrumentation during Phase II in test structures E-I, E-2 and E-3. Equipment was checked out and necessary adjustments were made for Phase II operation during the last two weeks in October. Some of the transducers were rearranged in E-I and E-2 to meet Phase II requirements of other participants. Four additional microphone systems and two displacement transducers in E-2 and two additional microphone systems in E-I were furnished and installed. AETRON installed recording and signal conditioning equipment in a designated room at the Bowling Alley, connected it to transducers previously installed by NASA and then checked out and operated the ten transducer systems.

The Boeing Company, Airplane Division, under "ubcontract to JABARD, furnished, installed and operated twelve microphone systems located on the exteriors of E-I and E-2 to measure boom pressure loadings on these two structures during Phase II. Recording, signal conditioning, and direct write equipment were installed in the garage of E-2. Power for equipment was available in E-I and E-2 from power panels separate from those used for supplying power for lights, receptacles, and air conditioning in the two structures.

Lockheed California Company, under subcontract to JABARD, furnished, installed, and operated 18 free field pressure microphone systems that were located on the dry lake bed east of the test houses, Figure 4-1. These instruments were installed to meet the requirements of NASA and ESSA.

Final reports describing work done by AETRON and Boeing are on file with the National Sonic Boom Evaluation Office. AETRON's report is entitled "Final Report, Subcontract BR-AFSBR-III", and Boeing's report is

entifled "Test Support to Sonic Boom Program, (Sub) Contract BR-AFSBR-110".

Tables B-I to B-4 in Appendix B present listings of the locations of the instrumentation with their specifications, and Figures B-I to B-7 present plan and elevation sketches of the test structures showing locations of transducers for Phase II. Table B-5 lists the equipment used in the various instrumentation systems, and Table B-6 lists the transducer frequency response and accuracy.

A number of precautions were taken to minimize thermal drift in equipment subject to temperature changes. In test structures E-I, E-2 and E-3, power to all equipment was kept on continually so that temperature gradients in the equipment could stabilize. Racks were generally enclosed so that the temperature of the air immediately surrounding the equipment did not change too rapidly in case of a sudden change in ambien temperature. Power was also left on to minimize thermal shocks which tend to shorten component life. Instruments were calibrated according to the procedures outlined under Instrument Calibration Procedures, Appendix C.

CEC Model No. VR-3300 magnetic tape recorders were used for all instrumentation except the Boeing microphone system. This system utilized an Ampex CP-100 machine. Fourteen-track machines were used in and near the structures and seven-track machines on the large microphone arrays. Tape speed was 30 lps with FM recording. Center frequency was 54.0 kcps with an information frequency of 0-10 kcps ± 0.5 dB. The full-scale signal to noise ratio (RMS signal/RMS noise) was 43 dB. Harmonic distortion was 1.5%.

A time code was recorded on one channel of each analog magnetic tape. During Phase I and up to November 21, 1967 of Phase II, the time code was standard iRIG Format "B", Figure 4-3. During Phase II on November 21, 1967, a mission identification system was incorporated into the time code to provide positive mission identification on each analog tape. With this identification //stem, it was possible to insert mission numbers and aircraft designations into the IRIG "B" time code. The resultant time code (Figure 4-4) consisted of a normal time code for seconds, minutes, hours, and days; followed by the number after the dash of the mission number before the dash; followed by a blank; then units, tens, and hundreds of the aircraft designation; and the remaining three positions not assigned.

Start and stop times for accurately digitizing analog data were based on manual reading of direct-write oscillograph records. Nominal boom times were recorded from a time code translator located in test structure E-2 as a check on the values read from the oscillographs. Manual readout to the nearest second was required for booms. Noise recordings of a typical aircraft flyby included three minutes of uninterrupted aircraft noise with 75 seconds recorded before and after—9 aircraft passed overhead. Notation of start and stop times for box—records was provided by JABARD personnel. "Recorders On" signals were the responsibility of NASA and Edwards AFB control.

When "Recorders On" signals were heard at the test structures, the tape recorders were turned on by hand; and as previously mentioned, the digitizing start and stop times were based on manual reading of the oscillograph records. It was felt that these two hand operations could be eliminated if a signal were automatically generated a short time before the boom occurred at the test structures. This signal could then be used to start the tape recorders and digitizing automatically, in order to generate this signal, a microphone was located along the flight track about 100 feet northeast of E-2. This microphone functioned well and generated the signal as expected.

#### DATA REDUCTION AND DISSEMINATION

The JABARD Data Reduction and Dissemination Group (DR&D) during Phase II were responsible for performing certain preliminary data reduction; assembling, card punching and processing free field cruciform microphone data; card punching and processing mission logs and instrument location logs; digitizing free field and structure response data; duplicating certain analog tape records; and disseminating summaries of the data as directed to appropriate participants.

NASA was responsible for providing values of positive overpressures, rise time, boom duration and waveform as shown by the sample waveforms in Figure 4-5. These values were measured from oscillographic records of the free field cruciform array microphones. The data were supplied for inclusion into the data printout scheme setup and implemented by DR and D as soon as possible after missions were flown. NASA also reduced data from the radar plots for all missions and furnished DR and D a summary together with copies of the radar plots.

The data furnished to DR and D was logged daily and all information was punched on a series of six data cards so that they could be processed by computer and printed output furnished to participants. The information contained on each card and the arrangement of the data for Phase II is as follows:

# 1. Mission Log

- a. Date
- b. Mission
- c. Aircraft
- d. Altitude, 1000 ft., MSL\*
- e. Mach number (or speed kph for subsonic aircraft)\*
- f. EPR (take-off or landing)\*
- g. Heading\*
- h. Offset from track, left or right\*
- i. Observed boom time, or time overhead for subsonic aircraft, ZULU\*
- j. Remarks
- k. Card type identification number (1)

\*Over test structure E-2

## 2. <u>Digitization Log - Data</u>

- a. Data
- b. Mission
- c. Aircraft
- d. Digitizing start time
- e. Digitizing stop time
- f. Location
- g. Card type identification number (2)

### 3. Instrument Location Log

- a. Date
- b. Channel
- c. House number and instrument designation
- d. Instrument type
- e. Location

- f. Location number (0 = inoperative, I = 1st position, 2 = 2nd position, etc.
- g. Card type identification number (3)

# 4. Channel Calibration Log

- a. Mission
- b. Channel
- c. House number and instrument designation
- d. Pre-calibrations
- e. Post-calibrations
- f. Rur attenuation and gain setting
- g. hemarks
- h. Digitization sample rate, sps
- i. Digitization filter cutoff
- j. Card type identification number (4)

### 5 <u>Digitization Log - Calibrations</u>

- a. Date
- b. Channel
- c. House number and instrument designation
- d. Calibration type (pre or post)
- e. Digitizing start and stop times
- f. Digitization sample rate, sps
- g. Digitization filter cutoff, cps
- h. Card type identification number (5)

# 6. Summary of Cruciform Data

- a. Mission
- b. Channe
- c. House number and instrument designation
- d. Wave form type code number for pressure mikes, see Figure 4-5
- e. Peak amplitudes in psf
- f. Rise time, seconds
- g. Period or duration of N-wave in seconds
- h. Wave angle, degrees
  - Wave angle is the angle between the pressure wave front and the ground as determined from the cruciform array
- i. Wave ground speed, ft/sec
- j. Card identification number (6)

The Mission Log in chronological order for Phase I is given as Table D-I, Appendix D. The Phase II Mission Log in order of mission numbers is given in Table D-2. The Instrument Location Log for a typical day, 15 November 1966, is given as Appendix E. A copy of the summary of Cruciform Data is presented in Appendix F. Table F-I is the data for Phase I, EAFB; F-2 is for Phase I, Lancaster; and F-3 is for Phase II, EAFB. The data are arranged in order by mission number to facilitate their use with the Mission Log. A description of the N-wave and its characteristics are given in Figure 4-5. Cards 2, 1 and

5 were primarily for use during digitizing of the analog data.

In addition to the data punched on the series of six data cards, an Analog Tape Log and a Digital Tape Log were prepared containing the following information:

#### I. Analog Tape Log

The purpose of this log was to record the information contained on each analog tape. There is one master copy of each log plus one copy of the appropriate log is filed with each analog tape. The log for each tape is as follows: (Numbers in parenthesis refer to data care ambers.)

- a. Heading card, containing analog tape number, date, tape recorder number, and total number of missions
- b. Channel locations (Card 3)
- c. Pre-calibration digitization start-stop times (Card 5)
- d. Mission identification (Card I)
- e. Mission digitization start-stop times (Card 2)
- f. Channel calibrations (Card 4)
- g. Post calibration digitization start-stop times (Card 5)

A sample of the Analog Tape Log for a typical day and tape recorder is given in Appendix G.

#### 2. Digital Tape Log

The analog tape records all channel data, whereas the digital tape contains only selected channels. The digital tape log is similar to the analog tape log, but contains the necessary identification for only those channels that have been digitized.

The free field cruciform array analog tapes were digitized using the facilities available at Edwards AFB. The analog to digital conversion (A/D) equipment at Edwards AFB was capable of digitizing six channels of data at a sampling rate of 5000 samples per second per channel. The computer facilities consisted of an IBM 7094/44 direct coupled system. The raw digital cruciform tapes were in multiplexed form and a computer program was developed in order to provide a check of the digital data and to arrange the data in a readily usable form. This program de-multiplexed and arranged the data serially by mission and channel, evaluate the sinusoidal calibrations by a curve

fitting and averaging process, edited the digital data so that the final output was one second of data, converted the pressure data to pounds per square foot, located positive and negative peaks and computed the time interval between them, and stored identification information on the tape. A brief description of the format of the cruciform digital tapes is given below:

- 1. Sampling rate 5000 samples/second/channel
- 2. Number of words per record 920
- 3. Number of bits per word 24
- 4. Bit density 556 B.P.I.

Structural analog data for the test flights were converted to digital form to facilitate processing on a digital computer. Digitizing rates for the structural data are given in the following table:

# DIGITIZATION UIREMENTS

Instrument	Tape Recorder Number	Digitization Rate SPS	Filter Cutoff CPS
Low Frequency Accelerometers	TR-2	8000	
Low Frequency Accelerometers	TR-4	8000	
Low Frequency Accelerometers	TR-5	8000	
Loading Microphones	TR-2	8000	
Loading Microphones	TR-4	1600	
Loading Microphones	TR-5	8000	
Loading Microphones			
Channels 801-807	TR-8	8000	
Loading Microphones			
Channels 808-812	TR-8	1600	
Strain Gages	TR-2	1600	
Strain Gages	TR-5	1600	
Displacement Gages	TR-4	1600	
Cruciform Array			
Loading Microphones	TR-6	5000	1350

#### NOTES:

1) For tape recorders 2, 4, 5 and 8, the time code (tape channel 14) is digitized as a data channel and the sampling rate is 8000 sps.

#### Data Retrieval

All analog digital tapes have been numbered and cataloged and will be stored under controlled temperature and numidity conditions. Through the use of analog gital tape logs described in the previous section, a detailed record has been kept of the data recorded on each tape. This record includes instrument locations, calibrations, mission logs, and digitization start and stop times. Data cards for all logs, mission log, cruciform summary, etc., are also readily available.

Phase I tapes have been indexed and are presently being stored in a controlled temperature and humidity atmosphere.

The following is a summary of the location of data taken during the Edwards AFB Phase II test program:

John A. Blume & Associates Research Division 612 Howard Street San Francisco, California 94105

- 1. For tape recorders 2, 4, 5, and 8:
  - a. Analog tapes,
  - b. oscillographic recordings,
  - c. analog tape logs,
  - d. original calibration data she id
- 2. For tape recorder 6:
  - a. Copies of analog tapes,
  - b. oscillographic recordings for XB-70 missions,
  - c. raw digital tapes, and
  - d. final digital tapes and accompanying documentation.
- 3. Master copies of:
  - a. Mission Log,
  - b. Summary of Cruciform Data.

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- c. Instrument Location Log, and
- d. Analog Tape Logs for tape recorders 1, 2, 3, 4, 5, 6, and 8.

- 4. Data card decks for:
  - a. Mission Log,
  - Digitization Log Data,
  - c. Instrument Location Log,
  - d. Channel Calibration Log,
  - 3. Digitization Log Calibrations,
  - f. Summary of Cruciform Data, and
  - g. Analog Tape Logs for tape recorders 1, 2, 3, 4, 5, 6, and 8.
- 5. Copies of Radar Plots.

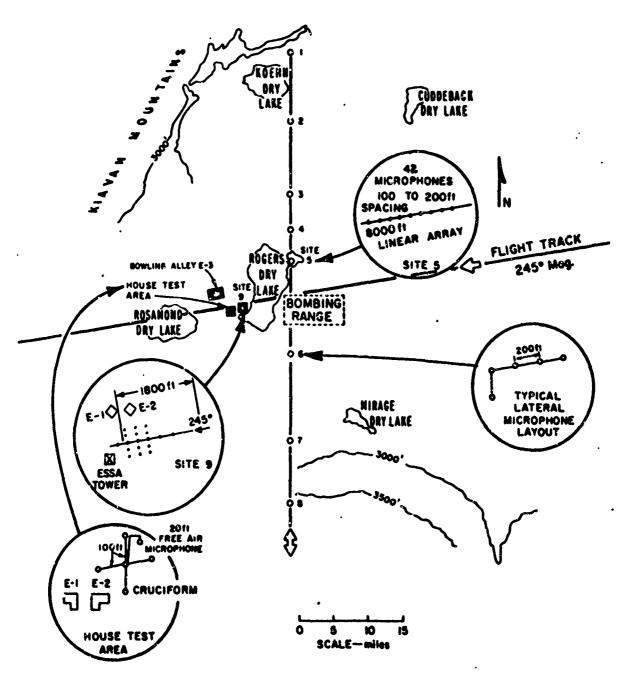
Stanford Research Institute 333 Ravenswood Avenue Menlo Park, California

- 1. For tape recorders 1 and 3:
  - a. Analog tapes,
  - b. oscillographic recordings,
  - c. analog tape logs, and
  - d. original calibration data sheets.
- 2. For tape recorder 6:
  - a. Copies of original digital tapes and accompanying documentation.

NASA, Langley Research Center Langley Station Hampton, Virginia

- 1. For tape recorder 6:

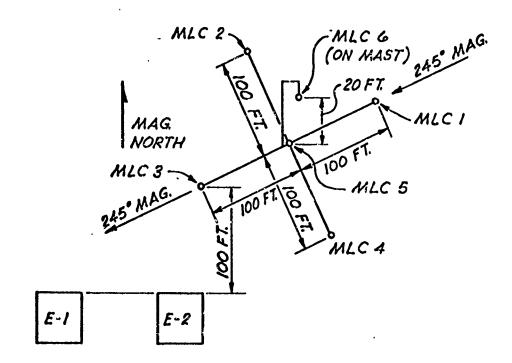
  - a. Original analog tapes,b. original calibration data sheets,c. oscillographic recordings, andd. original cruciform summary data sheets.



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FIG. 4-1 SCHEMATIC SHOWING TEST AREA, SONIC BOOM MEASUREMENT STATION
DEPLOYMENT, AND AIRCRAFT FLIGHT TRACK AND HEADING



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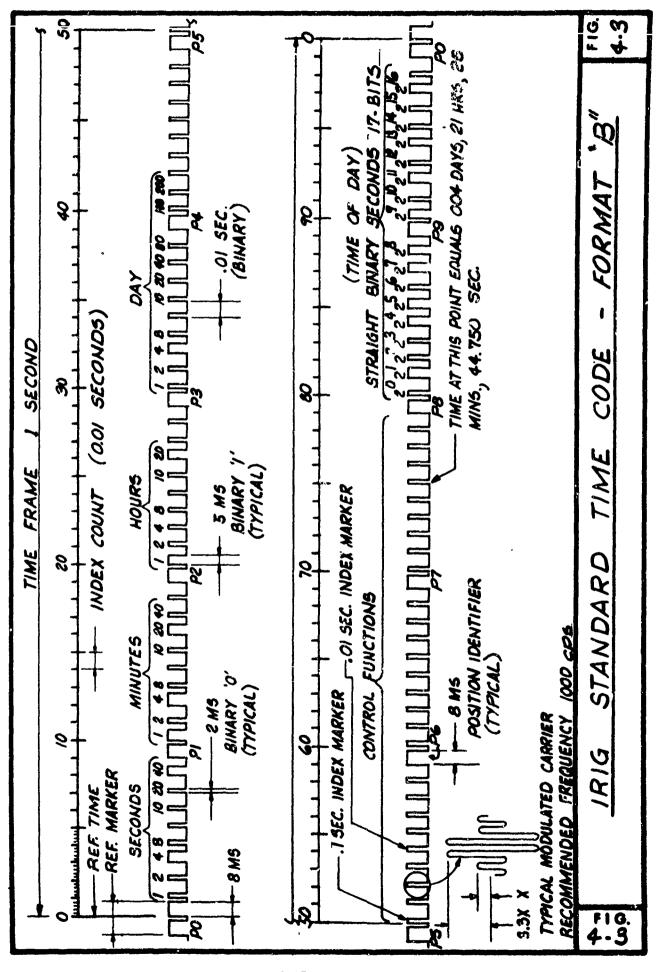
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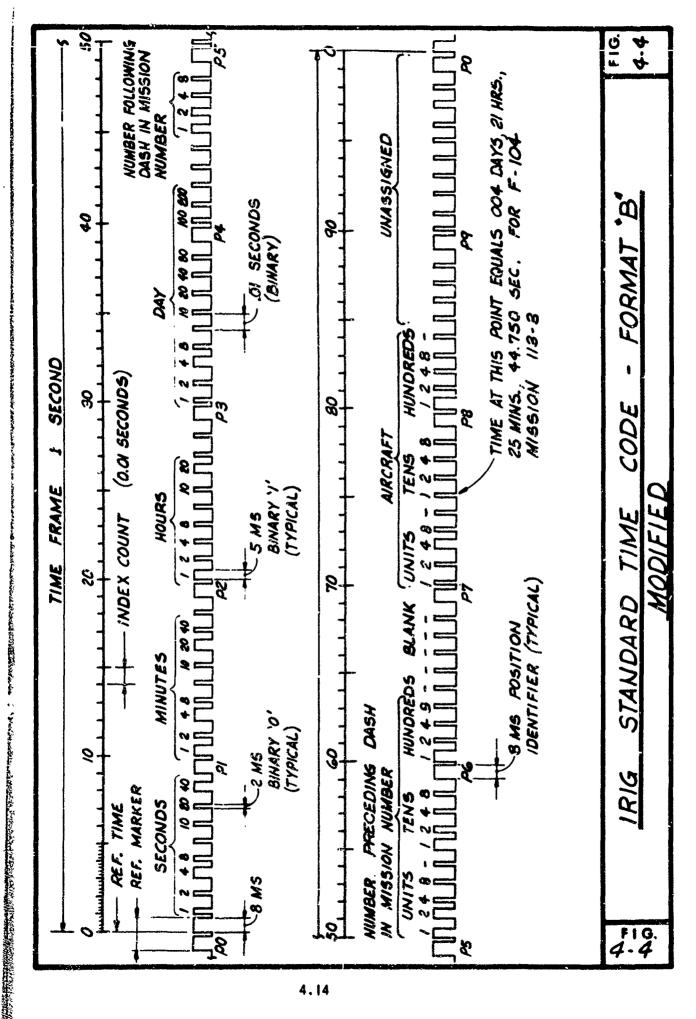
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Fig. 4-2 FREE FIELD MICROPHONES

(CRUC!FORM ARRAY)





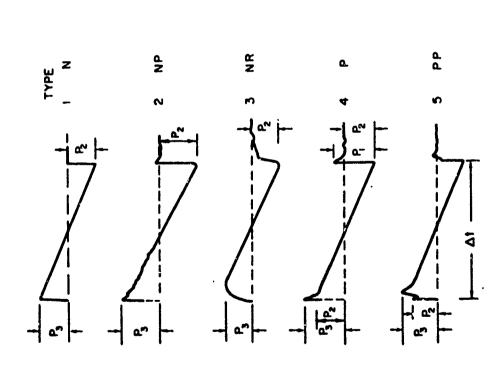


FIG. 4-5 SONIC BOOM WAVEFORM CATEGORIES

### V. ANALYSIS OF FREE FIELD SIGNATURE PARAMETERS

There were three objectives in the analysis of free field signature parameters. First, it was of interest to find if the channels were statistically equal or if the measured values of a parameter were independent of the channel on which it was measured and recorded. The immediate consequence of this study was to answer the question: Is it possible to use only one channel instead of five channels in recording free field signatures? Secondly, tests of equality of means were performed on free field signature parameters and on the Dynamic Amplification Factors (DAF) computed from free field signatures in order to determine which parameters most influenced the DAF. The results of these tests were needed in establishing a criterion for random sampling of missions in aurther studies. Thirdly, to determine the number of samples for structure response studies, a complete statistical description (mean, standard deviation and coefficient of variation) of the parameters was reeded.

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In the statistical analysis of free field signatures, each sonic boom mission was defined as a random phenomenon. The parameters of the free field signature (overpressures, duration and rise time) were defined as random variables. The measured values obtained from oscillographic prints of the signatures were defined as observations of random variables. The data used in the analyses were measured by five microphones arranged in a cruciform array located near structure E-2 for comparable missions of XB-70/B-58/F-104 aircraft (flights flown within a few minutes of each other). A channel as defined for the study of free field signature parameters, consisted of a measuring instrument (microphone), the signal conditioning and recording system, an oscillographic print of the record and the manual measurement of the parameter from this print.

An analysis of variance was the statistical test from which it was possible to obtain simultaneously the results for the equality of channels and the equality of means for each parameter and a complete statistical description of each parameter. With this test it was assumed that the channels and the missions were chosen at random. Random sampling meant that each item had an equal probability of being chosen. It was reasonable to assume that the channels in the E-2 cruciform array were selected at random from a number of channels used to measure and record free field sig-

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natures in this experiment. It was also reasonable to assume that the missions were selected from a large number of possible missions with different combinations of altitude, offset, speed and weight of the aircraft which would create similar signatures. In he analysis of variance the hypothesis that the effects (channel and mission) were not present was tested against the alternate hypothesis that the effects were present. When the result of a test showed that an effect was present, a factor was calculated to determine which missions or channels were different. The hypotheses were tested at different confidence levels to make sure that the results were not influenced by lower decimal values of the data. The hypotheses were tested in this chapter at the 95 percent confidence level.

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The free field signature parameters studied were: Peak positive everpressure  $(P_1)$ , the absolute value of peak negative overpressure  $(P_2)$ , the rise time  $(T_1)$ , the time from start of boom to the negative peak  $(T_2)$ , and the ratio of the absolute value of peak negative overpressure to peak positive overpressure  $(P_2/P_1)$ .

The tests for channel effects of free field signature parameters produced the following results. There is a channel effect in  $P_2$  for the XB-70 and B-58 missions and in the ratio  $P_2/P_1$  for the XB-70 missions. Similar results have been found using a different test 29. The difference in channels in each case was found between channels 605 and 607. This did not mean that the data from those channels should have been rejected, but that the coefficient of variation within each mission was composed of a coefficient of variation due to the boom and of a coefficient of variation due to the instruments. It was observed that, in most cases, the coefficient of variation of the data was primarily influenced by the coefficient of variation of the boom. The channel effect could be explained. As noted previously, a channel as used for the free field signature parameter study consisted of a measuring instrument (microphone), a signal conditioning and recording system, an oscillographic print of the record and a manual measurement from the print. The microphones used for free field signature measurement were modified to extend the low frequency response to 0.02 cps. The channel effect in  $P_2$  for the XB-70 missions was large enough to affect the ratio  $P_2/P_1$ , but was not large enough for the B-58 missions to affect the same ratio. Also the channel effect in P2 was not present for the F-104. missions. As the average frequency (I/ $T_2$ ) of  $P_2$  for XB-70 missions was

3.64 cps, for B-58 missions 6.1 cps, and for F-104 missions 12.8 cps, it could be inferred that the channel effect increased as the frequency response decreased below the normal lower limit of the microphone. It could also be inferred that one channel would have been sufficient to measure free field signature parameters when the measured response frequency was within the normal range of the microphone. Based on microphones used and data recorded, it was concluded that to adequately measure the free field signature parameters more than one microphone was needed for XB-70 and B-58 missions, but only one was needed for F-104 missions.

The tests of equality of free field signature parameters for different missions produced the following results. The magnitude of peak positive overpressure (P<sub>1</sub>) and of the absolute value of peak negative overpressure (P2) decreased as the altitude and/or lateral offset of the aircraft increased, as can be determined from Tables 5-1, 5-2, and 5-3. The averages of the parameters were compared using the factors at the bottom of the table. These factors indicated when two averages were different at the 95% confidence level. For example, in Table 5-1, the average positive overpressure (P,) of mission 5-2 was equal to 1.20 psf and of mission 10-1 to 2.41 psf. The difference between the two averages was !.21 and was greater than 0.3, the factor at the bottom of the table. Therefore, the average P. of mission 10-1 was greater than the average P, of mission 5-2 at the 95% confidence level. The ratio of the absolute value of negative overpressure to positive overpressure ( $P_2/P_1$ ) decreased as the offset of the aircraft increased. This result was evident only for the XB-70 missions as illustrated in Figure 5-1. There was only a small difference in the averages of rise time  $(T_1)$  and time from start of boom to negative peak  $(T_2)$  recorded for overhead and offset missions for each type of aircraft. This result can be determined from Tables 5-1, 5-2, and 5-3.

The coefficients of variation of the free field signature parameters calculated for all missions listed in Tables 5-1, 5-2, and 5-3 were:

	Coeff	Coefficient of Variation of			
Aircraft	Pı	P <sub>2</sub>	Т,	т <sub>2</sub>	P2/P1
XB-70	20%	31%	54%	10\$	17%
B-58	35%	25%	64%	<b>6</b> %	14%
F-104	20%	18\$	45%	10\$	10%

The number of missions needed for a comparative study of predicted and measured response was a function of the coefficient of variation of the data. From the above table it was evident that the number of missions needed would be different depending on which coefficient of variation was used. It was therefore necessary to know which of the free field signature parameters studied most influenced the DAF computed from free field signatures. Thus an analysis of the variance test was performed on maximum DAF's computed from free field signatures to study again the channel effect and the mission effect. The natural frequencies of the DAFs studied were, for the XB-70 missions 2.66, 3.23 and 3.93 cps, and for the B-58 missions 3.93, 4.78 and 5.81 cps. These natural frequencies were chosen as they were in the range of maximum DAF values.

There were no channel effects found in computed DAFs . Even if  $P_2$  and the ratio  $P_2/P_1$  affected the forcing function used to determine the DAF, the impulse response function and the damping (2%) were sufficient to eliminate the channel effect found in  $P_2$  and  $P_2/P_1$ . Therefore, the data from a single channel would have been adequate to evaluate the DAF computed from free field signatures.

The results of the tests of equality of maximum DAFs for different missions indicated the following. The magnitudes of the maximum DAFs for each aircraft were different and occurred at different natural frequencies. In comparing the difference in DAF magnitude with the difference in free field signature parameters, it was found that the magnitude of the DAF decreased as the ratio  $P_2/P_1$  decreased for XB-70 missions; Figure 5-2. It was found that the natural frequency at which the maximum DAF occurred decreased as  $T_2$  increased for the XB-70 missions, as shown in Figure 5-3. The effect of  $T_2$  was also evident in a visual comparison between DAF envelopes for eight XB-70 missions, eight B-58 missions and five F-104 missions in Figure 5-4. From this figure it was evident that duration of sonic boom affected the range of structure natural frequencies over which maximum dynamic amplification occurred. Since boom duration increased with aircraft size, sonic booms from large aircraft such as the XB-70 and the future SST will affect a greater range of structure elements than sonic booms from smaller aircraft such as the B-58 and the F-104. Previous studies 18 have indicated that rise time also affects the magnitude of the DAF. However, with the data recorded, this effect could not be verified. It was therefore concluded that the ratio  $P_2/P_1$  caused the major effect on the magnitude of the DAF and that the number of missions needed in further studies of structure response should be determined from the statistical description of the ratio  $P_2/P_1$ .

The number of missions needed to study the response of structure elements was obtained from statistical sampling techniques. <sup>16</sup> The sample size varied according to the degree of precision desired in the results and the confidence level of the conclusions about the results. The following table demonstrates this process.

Degree of Precision in Results	Confidence Level	Number o	of Mission	ns Needed
in %	in <b>%</b>	XB-70	B-58	F-104
10	95	П	8	4
15	95	5	4	2
20	95	3	2	1
10	90	8	6	3
15	90	4		2
20	90	2	2	1

The degree of precision in the results meant that the results obtained from the analyses of the sampled missions would be within 10% (or 15% or 20%) of the results which would have been obtained if all missions had been analysed. The probability that the results were within such a percentage was given by the confidence level. The degree of precision in the results and the confidence level were chosen according to the pertinence of the study.

### SUMMARY OF FINDINGS

The free field signature parameters were analysed to determine if the channels were statistically equal or if the measured values of a parameter were independent of the channel on which it was recorded; to determine which parameters most influenced the Dynamic Amplification Factor (DAF); and to determine the number of samples necessary for studies of structure response data. The analytical techniques were such that the findings are stated with

a 95 percent confidence level; that is, there is a 95 percent probability that the findings are correct. Following are the findings resulting from these analyses:

- I. For the XB-70 missions the peak negative overpressure,  $P_2$ , and the ratio of the absolute value of peak negative overpressure to peak positive overpressure,  $P_2/P_1$ , were not independent of the channel on which they were recorded. All other parameters studied (positive overpressure,  $P_1$ , rise time,  $T_1$ , and the time from start of boom to negative peak,  $T_2$ ) were independent of the channels.
- 2. For the B-58 missions  $P_2$  was not independent of the channel on which it was recorded. All other parameters studied  $(P_1, T_1, T_2, \text{ and } P_2/P_1)$  were independent of the channels.
- 3. For the F-104 missions all parameters  $(P_1, P_2, T_1, T_2, \text{ and } P_2/P_1)$  were independent of the channels on which they were recorded. A single channel would have been adequate to measure free field signatures.
- 4. The magnitude of  $P_1$  and of the absolute value of  $P_2$  decreased as the lateral offset and/or altitude of the aircraft increased.
- 5. The ratio  $P_2/P_1$  decreased as the offset of the aircraft increased for XB-70 missions.
- 6. There was little difference between rise times  $(T_i)$  of overhead and offset missions for each type of aircraft.
- 7. There was little difference between times from start of boom to negative peak  $(T_2)$  of overhead and offset missions for each type of aircraft.
- 8. The Dynamic Amplification Factors (DAF) computed from free field signatures were independent of the channel the signatures were recorded on. Therefore, a single microphone would have been sufficient to evaluate DAFs.
- 9. The magnitude of the maximum DAF decreased as the ratio  $P_2/P_1$  decreased.
- 10. The natural frequency at which the maximum DAF occurred was a function of the time from start of boom to negative peak  $T_2$ . As  $T_2$  increased, the maximum DAF occurred at a lower natural frequency.

- II. Sonic booms from large aricraft such as the XB-70 and the future SST will affect a greater range of structure elements than will sonic booms from smaller aircraft such as the B-58 and the F-104.
- 12. The number of missions needed in the study of the response of structure elements varied depending on the degree of precision in the results and on the confidence level.

CONTRACTOR WESTERNAMENTS

SUMMARY OF FREE FIELD SIGNATURE PARAMETERS

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# AIRCRAFT XB-70

Average 7 2 x 10 -1 sec	2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.	2.98 2.99 3.13 6.02
Average T 1 Sec	w o 4 w 4 w 4 0 w 4 4 4 4 4 4 4 4 4 4 4 4 4	. 0. 4. 10. 4. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.
Average P <sub>2</sub> /P <sub>1</sub>	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.03 0.83 0.84 0.07
Average F <sub>2</sub> psf	2.61 2.45 2.24 6.00 6.00 6.00 6.00 6.00 6.00 6.00 6.0	0. 1. 8. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.
Average P l psf	2.29 2.29 2.39 2.09 2.09 2.09	2.23 2.29 0.30 0.30
Offset Feet	L10,300 L37,600 L 900 R12,900 R71,300 R32,100 L13,300 L 200	υψ
Mech Top		1.80 1.80 1.80 1.80 1.80 1.80 1.80
Aititude Feet	7, 17, 18, 18, 18, 18, 18, 18, 18, 18, 18, 18	14-1 59,700 1.80 15-1 60,600 1.80 16-2 59,700 1.80 13-2 60,300 1.80 Iwo parameters are different when the in their averages is equal to or great
Mission	44464849 <u></u> 5	15-1 16-2 13-2 Two param

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SUMMARY OF FREE FIELD SIGNATURE PARAMETERS

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ge Average  T2  Sec × 10 Sec	1.65	1.49	1.57	1.74	1.72	55 63	1.64	0.0
Average T x 10 <sup>-3</sup> sec	5.0	13.0	<u>6</u> 	3.2	4 N 4 8 N 0	4 4 W	4.0	5.2
Average P <sub>2</sub> /P <sub>1</sub>	0.77 0.98 0.72	0.89	0.88 0.71 0.95	0.80	0.97 0.79 0.83	0.70 0.87 0.94	0.76	0.14
Average P2 psf	2.37 2.26 1.85	2.01	1.15 1.25 2.27	2.60	 8 6 8 8 8	1.85 2.04 2.09		0.18
Average Pl psf	3.08 2.32 2.55	2.25	1.31 1.75 2.39	2.71 3.27	1.86 2.39 2.21	2.24 2.25	2.61	1.37
Offset Feet	L 7,200 L 7,500 R 7,800	R 1,900 R63,300	R40,400 R38,700	R 1,700	R 800 L 2,100 L 2,500	R 4,200 0 R 3,000	L 700	the difference greater than
Mach	 4 n n		- 65 - 65 - 65	1.32	65 65 65	53 65 65	1.65	ferent when qual to or
Altitude Feet	32,400 33,000 32,400	32,000	35,500 35,800 35,500	40,400 32,400	40,200 39,200 35,900	38,800 39,600 39,700	39,100	Iwo parameters are different when the in their averages is equal to or great
Mission	- 4 W	47	<u> </u>	<u>6</u> 6 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7	7-2- <u>7</u>	15-2	<u>-</u>	Two par in thei

TABLE 5-3

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SUMMARY OF FREE FIELD SIGNATURE PARAMETERS

# AIRCRAFT F-104

Mission	Altituds Feet	Mach	Offset Feet	Average Pl psf	Average P <sub>2</sub> psf	Average p./r.	Avorage T x 10 <sup>-3</sup> sec	Average T2 X 10 1 sec
4-	18,600	1.30	R 2,200	3.24	2.75	0.87	2.5	0.81
2-4	17,600	_ 동	1	3.5	2.79	0.80	<b>6</b> :	0.79
¥	17,800	<u>ድ</u>		2.36	2.36	<u>-</u> 8	4.4	0.77
۹ ۳-	21,100	1.14		1.54	08.1	76.0	7.4	0.85
	20,600	 64.		2.01	2,03	<u>. 0</u>	4.5	0.73
12-3	22,030	1.42	R 6,700	2.10	8	0.97	5.2	0.76
<del>[3-3</del>	20,000	1.40		2.01	1.92	0.96	5.3	0.74
<u>+-3</u>	21,400	8		2.10	2.15	1.07	3.6	0.86
5-3	20,200	- 40		2.31	2.09	0.91	3.8	0.73
<u>16-3</u>	20,600	- 6		2.10	1.85	0.92	2.7	0.76
13-3	20,600	1.40		1.95	1.89	0.98	4.5	0.78
Two paramin their	Iwc parameters are different when the differe in their averages is equal to or greater than	ferent when qual to or g	Two parameters are different when the difference in their averages is equal to or greater than	0.43	9.14	0.13	0.4	0.19
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TABLE 5-4

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SUMMARY OF DAFS COMPUTED FROM FREE FIELD SIGNATURES

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Average DAF @ 3.93 cps	2	0.15
Average DAF @ 3.23 cps		0.17
Average DAF @ 2.66 cps	1.64 1.52 1.52 1.55 1.68 1.86 1.86	91.0
Offset Feet	L 900 R12,900 R71,300 R68,200 R32,100 L13,300 L 200 R 6,400 R 9,500 L 100	quency) when ar than or
Mach	2.50 2.50 2.50 2.50 2.50 2.50 2.50 2.50	or the same frec erages is greate
Altitude Feet	37,600 59,100 60,300 59,400 59,400 59,400 60,200 60,200 60,300	Two DAFs are different (for the same frequency) when the difference in their averages is greater than or equal to
Mission	2-4-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2	Two DAFs at the difference of

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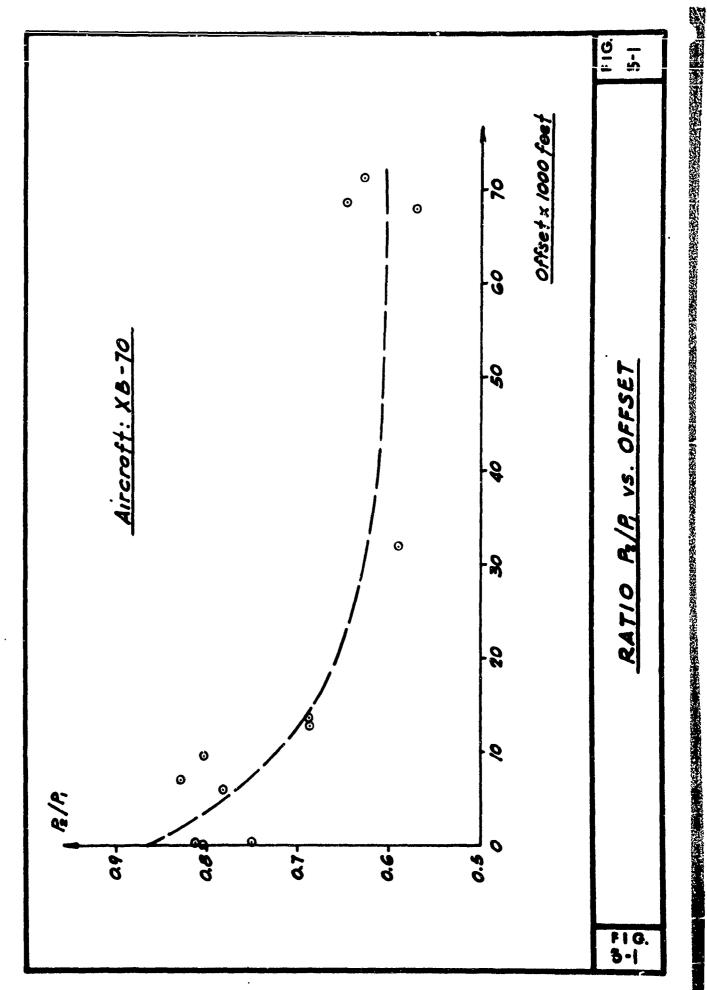
TABLE 5-5

SUMMARY OF DAFS COMPUTED FROM FIREE FIELD SIGNATURES

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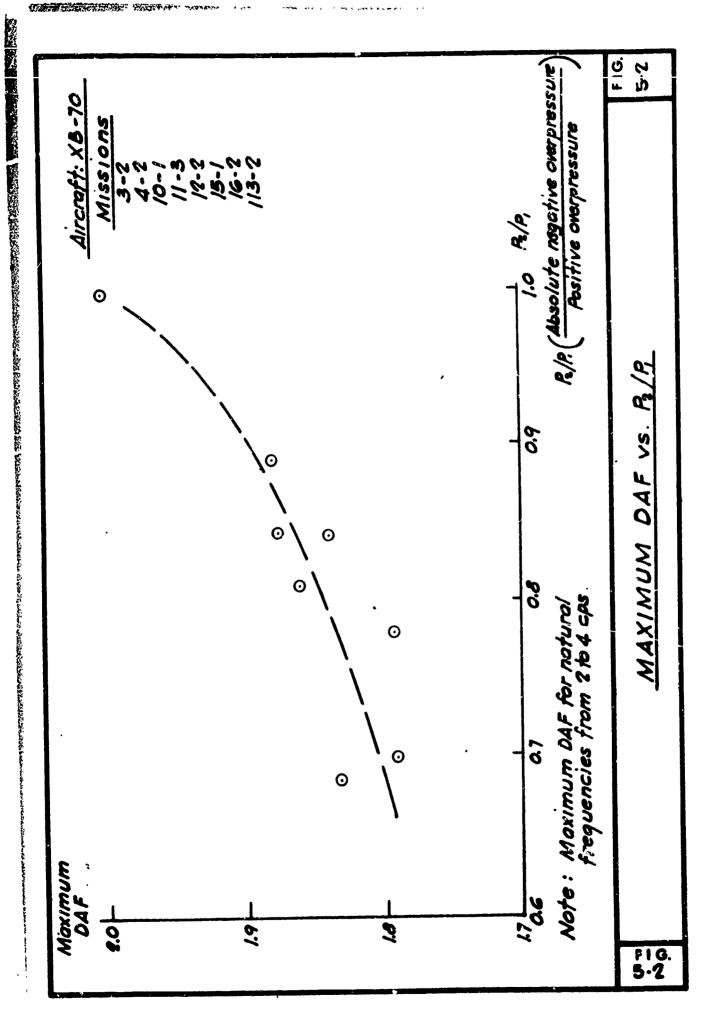
P-58	
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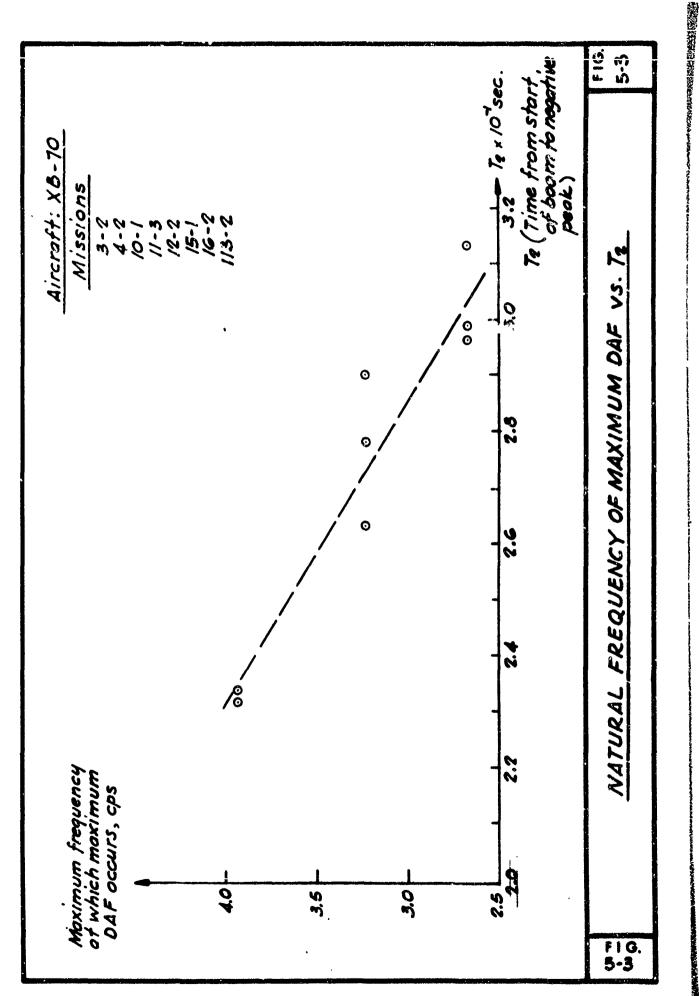
Mission	Altitude Feet	Mac.	Offset Feet	Average DAF 6 3.93 cps	Average DAF DAF	Average DAF 6 5.81 cps
<u>~</u>	32,400	~ . ~ .	R 7,800	1.32	09:1	1.73
<u>† Y</u>	36,300 36,300	·	R63,300	6	2.16	%:- %:-
<u>P</u>	35,500		2,300	1.63	1,87	. 84
2 2 2 2	40,400 32,400	.32	K - 700	 	.95	1.76
11-2	40,200	1.65	800 800	1.87	2.10	- 39
12-1	39,200	1.65	L 2,100	1.58	1.76	1.63
<u>Y</u>	35,900	1.65	L 2,500	1.47	1.73	1.71
15-2	39,600	1.65	0	59	84	85
-6	39,700	1.65	R 3,000	1.74	1.97	1.89
<u> </u>	39, 100	1.65	٦ م	1.48	1.69	1.66
Two DAFs the differ	DAFs are different (for the difference in their averages in to	or the same fred brages is greate	same frequency) when is greater than or	0.10	0.12	0.12



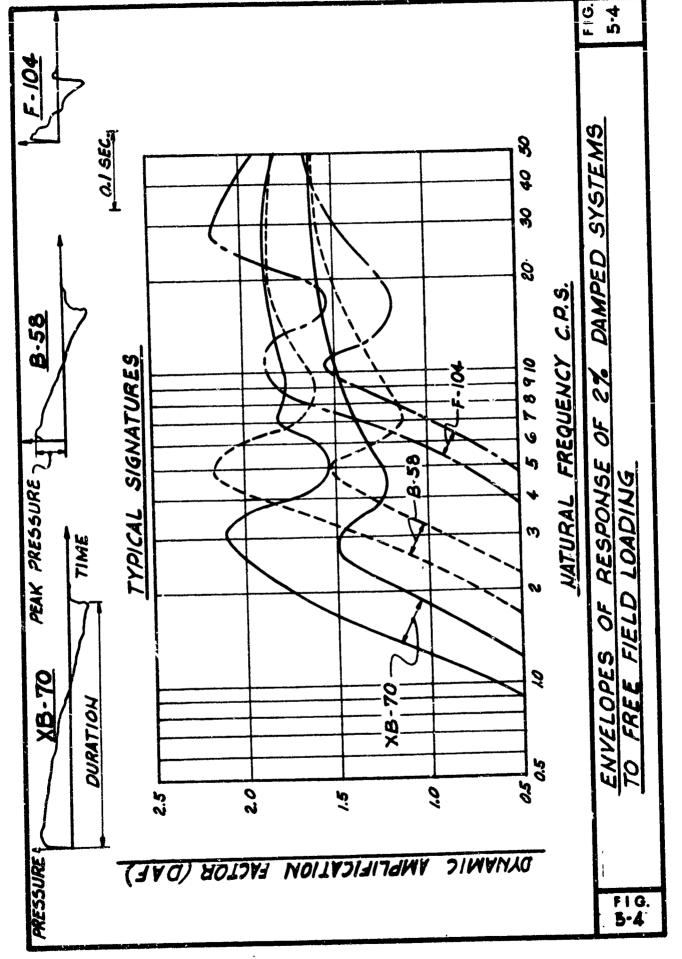
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# VI. EFFECTS OF FREE FIELD SIGNATURE PARAMETERS ON DYNAMIC AMPLIFICATION FACTORS

There were two objectives in making the study of the effects of free field signature parameters on DAF spectra. First, it was of interest to derive a wave model from free field signature parameters (over-pressures, rise time, duration) that could be used to make an accurate evaluation of the DAF and thereby eliminate the lengthy and costly process of digitizing tree field signatures that is presently needed to compute the DAF. Secondly, to gain a greater insight on the probable magnitude of the DAF for large lateral offsets of an aircraft, the effects of values of free field signature parameters beyond the range of the data recorded by the E-2 cruciform microphone array on the DAF were analysed using the wave model.

To fulfill the first objective, the derivation of a wave model, it was assumed that for purposes of calculating DAF a free field signature could be completely described by the parameters; peak positive overpressure  $(P_1)$ , rise time  $(T_1)$ , peak negative overpressure  $(P_2)$ , and time from start of boom to negative peak  $(T_2)$ . The total duration of the signature  $(\tau)$  was assumed to be equal to  $T_2$  plus  $T_1$ . Linearity was assumed between peak overpressure points, as illustrated in Figure 6-1. The averages of  $P_2/P_1$ ,  $T_1$  and  $T_2$  were computed for eight XB-70 missions, eight 8-58 missions and five F-104 missions. They were:

Alrcraft	P <sub>2/P1</sub> *	$\frac{T_{1} \times 10^{-3} sec}{}$	T <sub>2 x 10</sub> -1 sec
XB-70	0.773	5.15	2.88
B <del>-</del> 58	0.852	5.63	1.67
F-104	0.966 *absolut	4.66 e value	0.75

These average values were used in computing DAF spectra from the wave model. Two different methods of comparison were used to see if the DAF spectra calculated from the wave model were representative of the spectra computed from digitized free field signatures. A visual comparison of the spectra in Figures 6-2, 6-3, and 6-4 showed that the DAF spectra of the wave model were generally within the envelopes of the DAF spectra obtained from digitized free field signatures of the same missions. Then to mathematically substantiate the visual comparisons, a "goodness of fit" (chi-square) 14 test was per-

The second of th

formed. In this statistical test the average values of the DAF spectra computed from digitized free field signatures were compared with the values of the wave model DAF spectra at twenty-one different natural frequencies between I and 50 cps. The results of the test showed the fit is good at the 95% confidence level for all three types of aircraft. These results are it ustrated in the following tabulation of values of the test statistic computed from the data and the chi-square value. The test statistic must be smaller than chi-square value to conclude that the fit is good at the 95% confidence level.

Aircraft	Test Statistic	Chi-Square Value € 95% C. L.
XB-70	0.07	31.4
B-58	0.05	31.4
F-104	0.57	31.4

The small values of the test statistic were due to the small difference between the average values of the DAF spectra computed from digitized free field signatures and the values of the DAF spectra computed from the wave model. A value greater than 31.4 would have indicated that there was no fit at all. It was therefore concluded that a free field signature wave model described by  $P_1$ ,  $P_2$ ,  $T_1$  and  $T_2$  could be used to accurately evaluate the DAF computed from digitized free field signatures. It was further concluded that, for future work with free field signatures, knowledge of the values of  $P_1$ ,  $P_2$ ,  $T_1$  and  $T_2$  would be sufficient for obtaining DAF spectra that would be representative of those obtained directly from free field signatures. The digitization of free field signatures could therefore be eliminated.

The wave model was then used to fulfill the second objective of this study which was to investigate the effects of values of free field signature parameters beyond the range of the data recorded by the E-2 cruciform array on the DAF spectra. First, different values of  $P_1$ ,  $P_2$ ,  $T_1$ ,  $T_2$  were used to confirm that the ratio of the absolute value of peak negative overpressure to peak positive overpressure ( $P_2/P_1$ ) influenced the magnitude of maximum DAF. The same input values were then used to study the influence of  $T_1$  on the magnitude of the DAF and finally to obtain DAF spectra for values of  $T_2$  larger than those measured and recorded by the E-2 cruciform microphone array.

It was found that as the ratio  $P_2/P_1$  increased the magnitude of the maximum

DAF increased. This result was derived for natural frequencies of 4 to 7 cps, and a wave duration of 0.2 sec. as indicated in Figure 6-5. Similar results were found in Chapter V for the XB-70 missions. It was also found that, for the XB-70 missions, the ratio  $P_2/P_1$  decreased with increasing lateral offset of the aircraft. It was therefore evident that one effect of increasing lateral offset was to cause a corresponding decrease in the magnitude of the maximum DAF.

In previous studies  $^{32}$  it was found that  $T_1$  increased with increasing lateral offset, and that the two limiting shapes of a free field signature could be an N wave with zero rise time, and a sinusoidal pulse, Figure 6-6. The effect of  $T_1$  on the DAF was therefore studied within these limits. To generalize the findings, the ratio of rise time to duration  $(T_1/\tau)$  was used in the study instead of the value of  $T_1$ . The results were plotted in Figure 6-7. It was found that as the ratio  $T_1/\tau$  increased, the magnitude of the maximum DAF increased. For this study a wave duration of 0.2 sec. and a ratio  $P_2/P_1$  equal to I were used and the values of maximum DAF were computed for natural frequencies of 4 to 7 cps. Therefore it was concluded that increasing the lateral offset of an aircraft increased the maximum DAF values. The total influence of lateral offset on the magnitude of the DAF could not be determined. There were insufficient recorded data to definitely indicate which of the two ratios,  $P_2/P_1$  or  $T_1/\tau$ , was predominant. Analyses of more data recorded at large lateral distances from the flight track are needed.

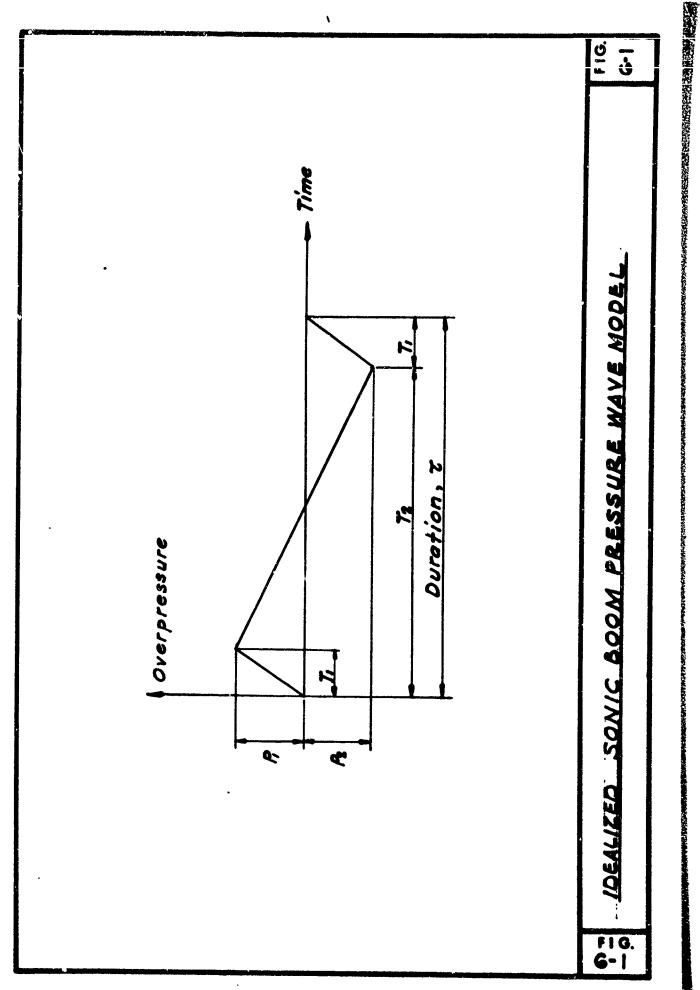
Throughout this study the DAF spectra were computed with a damping coefficient of 2%. The effect of different damping coefficients was studied for different wave durations. The results plotted in Figure 6-8 were computed for a wave duration of 0.3 sec. with a rise time equal to 2% of the wave duration and a ratio  $P_2/P_1$  equal to 1. The DAF values plotted were calculated for a natural frequency of 3 cps. It was for a that the magnitude of the DAF decreased as the damping coefficient increased.

It was assumed that free field signatures for aircraft larger than the XB-70 would be similar in shape. By using the wave model DAF spectra were then computed for durations of 0.4 and 0.5 sec. and different combinations of ratio  $T_1/\tau$  and  $P_2/P_1$  as illustrated in Figures 6-9 and 6-10. As found in Chapter V, durations of 0.4 and 0.5 sec. increased the range of structure natural frequencies over which maximum dynamic amplification occurred.

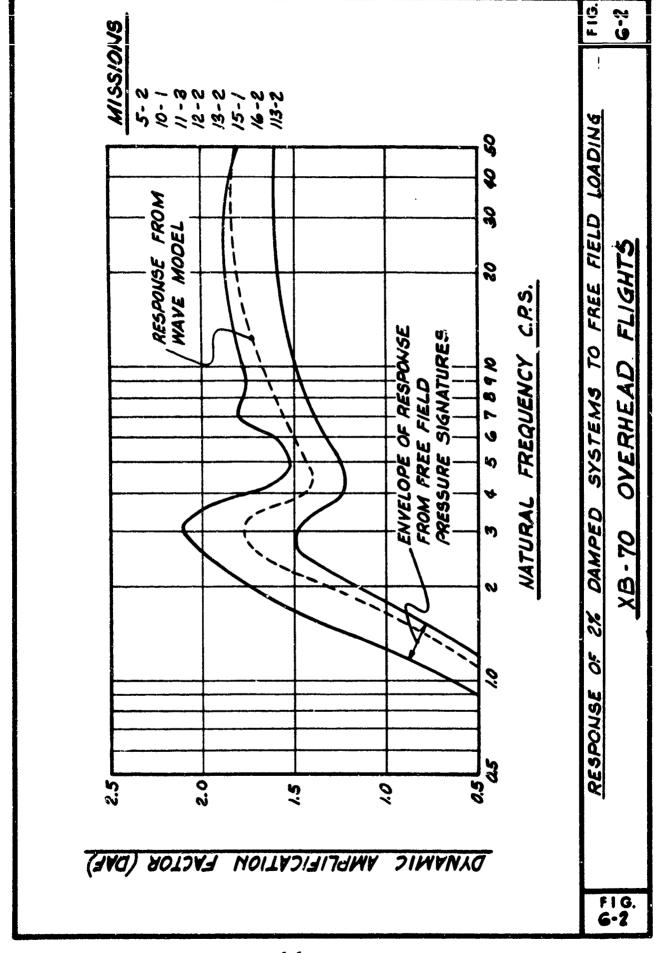
### SUMMARY OF FINDINGS

The objectives of this chapter were first, to derive a wave model in order to eliminate the digitization of free field signatures presently needed to compute DAF spectra and second, to study the change in magnitude of the maximum DAF for different large lateral offsets of an aircraft. The data used in the analyses were measured during Phase II by five microphones arranged in a cruciform array located near structure E-2 for the comparable missions of XB-70/B-58/F-104 aircraft (flights flown within a few minutes of each other). The following findings resulted from these studies:

- I. The DAF spectra obtained using a wave model described by free field signature parameters  $P_1$ ,  $P_2$ ,  $T_1$  and  $T_2$  were equal at the 95 percent confidence level to the DAF spectra obtained from digitized free field signatures.
  - 2. Using the derived wave model it was found that:
    - a. the magnitude of the maximum DAF decreased as the ratio  $P_2/P_1$  decreased,
    - b. the magnitude of the maximum DAF increased as the ratio  $T_{\parallel}/\tau$  increased, and
    - c. the magnitude of the maximum DAF decreased as the damping coefficient increased.
- 3. In the analysis of the effects of lateral offset of aircraft, the ratio  $P_2/P_1$  in the recorded free field signatures caused the predominent effect on DAF. The recorded signatures showed little change in rise times  $(T_1)$  or in durations  $(\tau)$  for overhead and offset missions for each type of aircraft. Therefore the influence of lateral offset on DAF spectra was limited to the effect of the ratio  $P_2/P_1$ .



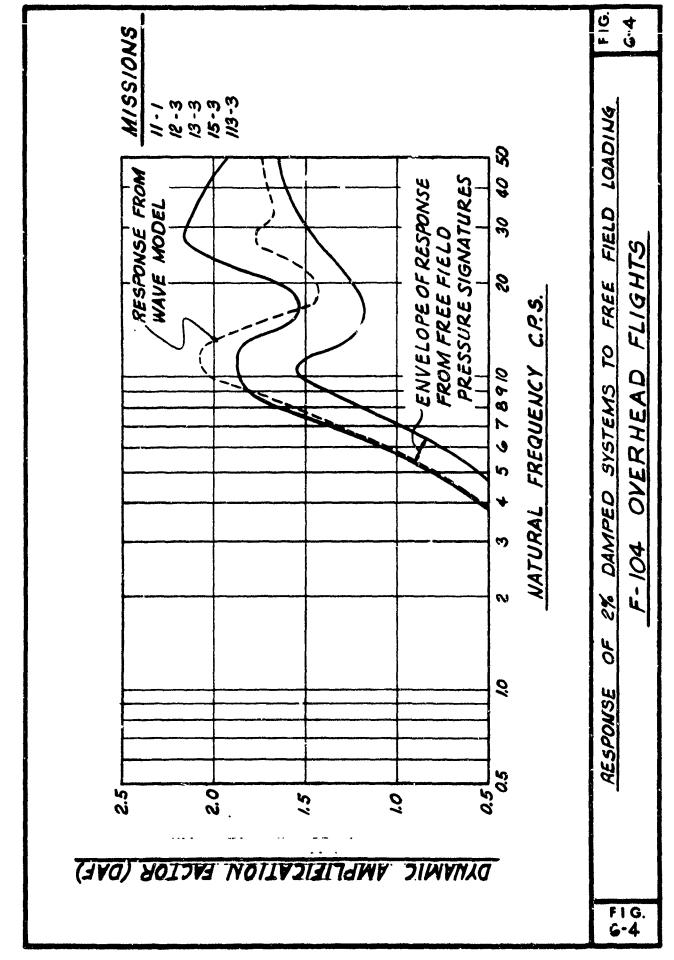
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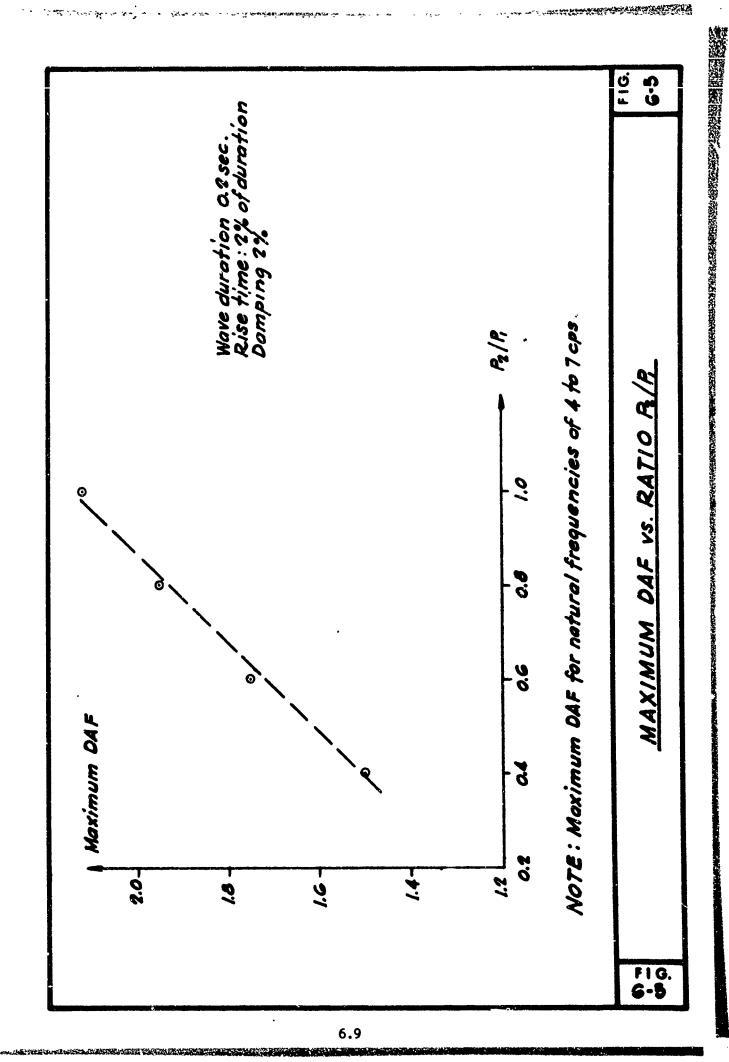


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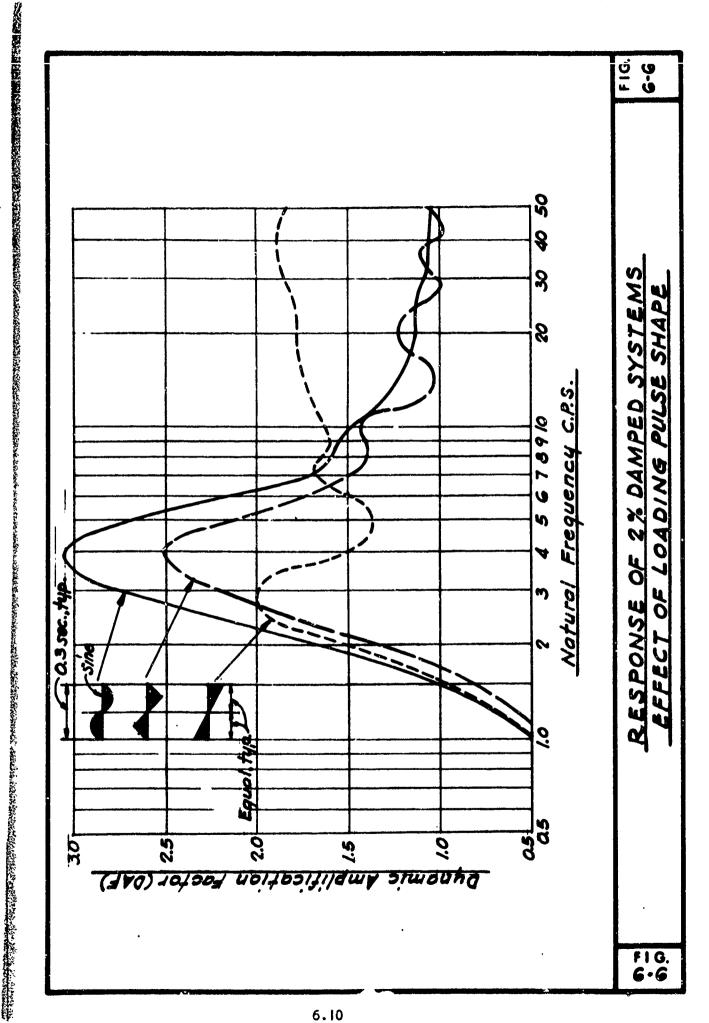
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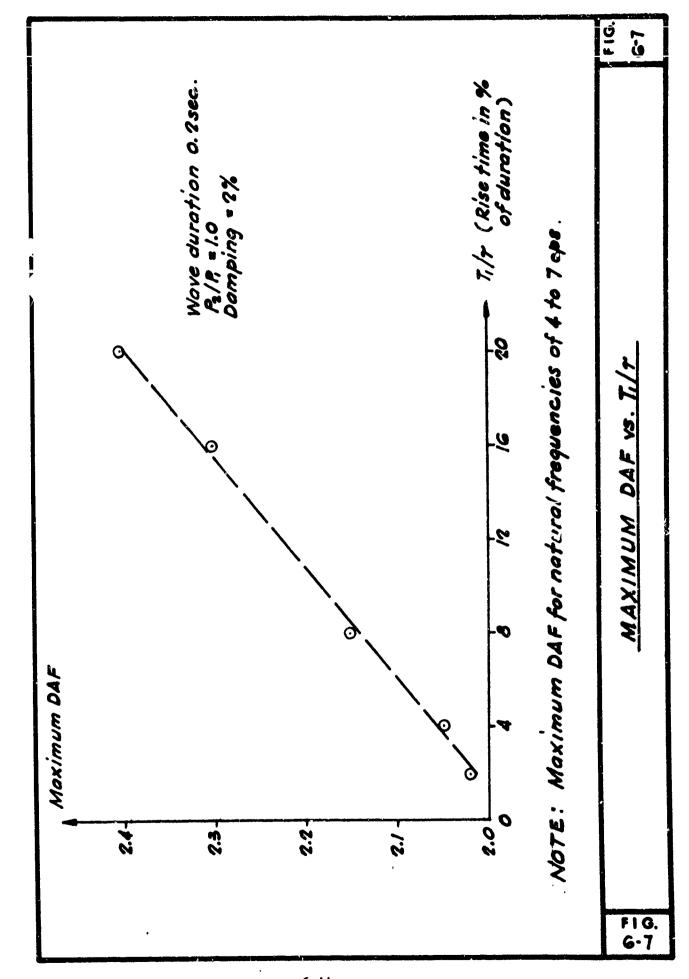




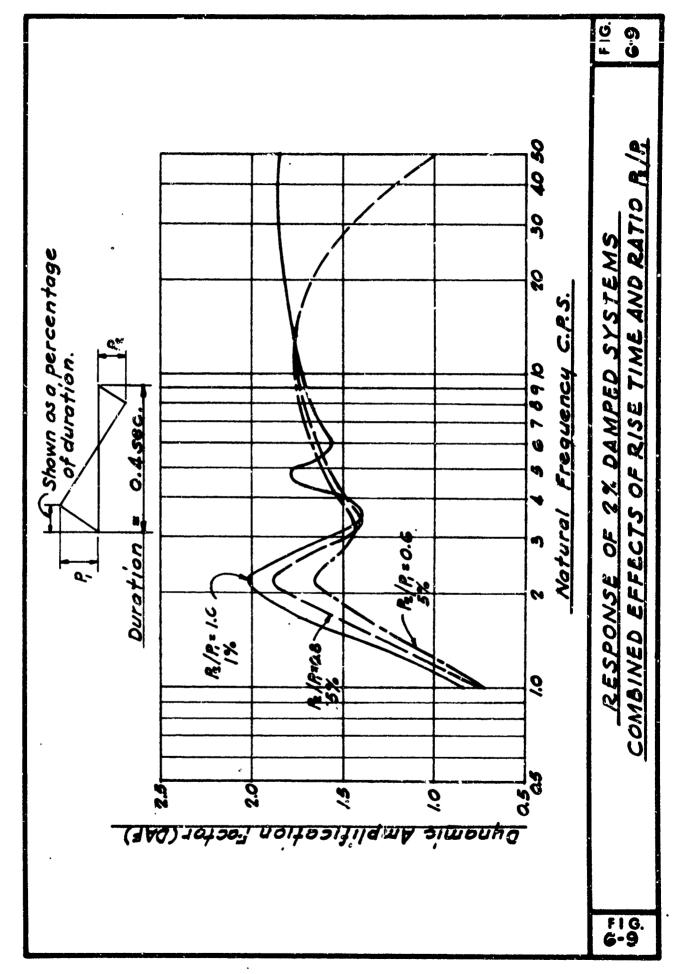
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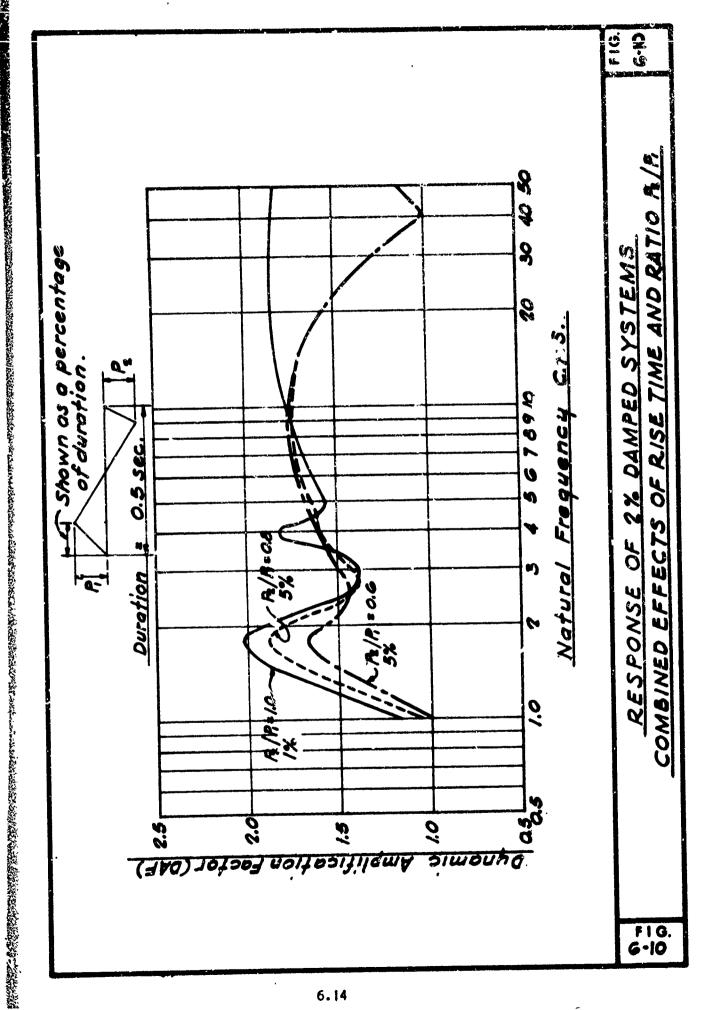




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# VII. ANALYSIS OF STRUCTURAL RESPONSE DATA - PLATE RESPONSE

The analysis of structural response data was divided into two sections: plate response and racking response. Plate response was defined as the lateral deformation of individual structure elements and was primarily of a bending mode. Racking response was defined as the deformation of the structure as a whole and was primarily of a shearing mode. The analysis of the plate response data is covered in this chapter and racking response will be discussed in the following chapter. This chapter is divided into three sections:

A) Wall Plate Response in Test Houses E-1 and E-2; B) Window Plate Response in Test House E-1; and C) Response of the Roof Frame of the Bowling Alley, E-3.

# A. WALL PLATE RESPONSE IN TEST HOUSES E-1 AND E-2

The analysis of the wall plate response in Test Houses E-1 and E-2 considered three typical walls: the east wall of bedroom number one in House E-2 (BRI-1); the east wall of the dining room in House E-2 (DR-2); and the north wall of bedroom number one in House E-2 (BRI-2). Predicted displacements were computed and compared with measured displacements for the three walls. Predicted displacements were computed using two sources of loading data: peak overpressures and DAF spectra obtained from free-field signatures and peak overpressures and DAF spectra obtained from net pressure signatures. The effects on plate response of flight track offset, Mach number and aircraft vector were also investigated. In addition, the results of the Phase II tests were compared with those from previous tests.

### INSTRUMENTATION

Accelerometers were installed on the east wall of BRI-I, the east wall of DR-2, and the north wall of BRI-2 to determine the plate response of typical walls in the test houses. These accelerometers were mounted at mid-height of the center stud of each wall. In addition to the accelerometers, pressure microphones were installed on the inside and outside of these walls so that the actual loading or net pressure on the walls could be determined. The locations of these instruments are shown in Figure 7-1; their characteristics are listed in Appendix B.

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NOTE: All figures and tables are placed at the end of this chapter. For the altitude, Mach number, offset, etc., of aircraft missions used in this chapter, see Tables 5-1, 5-2, and 5-3.

### TEST RESULTS - MEASURED DISPLACEMENTS

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The analog magnetic tape recordings of the accelerometer data were converted to digital form as discussed in Chapter IV. The digital records were then numerically integrated twice to obtain displacements. Peak displacements determined for the BRI-I, DR-2, and BRI-2 walls are listed in Tables 7-1, 7-3, and 7-5, respectively, under the heading of Measured Displacements. Displacements were calculated for comparable XB-70, B-58, and F-104 overhead missions (flights flown within a few minutes of each other) for all three of the walls, and for XB-70 flights of different Mach number and offset for the BRI-I and DR-2 walls.

Acceleration and corresponding displacement records for the DR-2 wall for typical missions of XB-70, B-58, and F-104 aircraft are shown in Figures 7-2 through 7-7. Similar displacement records for Phase I are shown in Figure 7-8. Note that the displacements obtained during Phases I and II were of similar magnitude for similar overpressures.

Displacements of DR-2 for typical missions of XB-70, B-58, and F-104 aircraft were superimposed on the actual or net loading signatures on the wall, Figures 7-9, 7-10, and 7-11. (Net pressure was the outside pressure minus the inside pressure, and was the actual pressure on the wall). Note that the pattern of displacements corresponded closely to the shape of the net pressure signature.

The effects on structural response of the offset of the aircraft flight track was determined from plots of displacement versus offset for XB-70 missions for BRI-I and DR-2, Figures 7-12 and 7-13. There was an indication that response decreased with an increase in offset.

The curves in Figures 7-12 and 7-13 indicated that plate response decreased slightly with an increase in Mach number. The average peak plate displacement of DR-2 for overhead XB-70 missions at Mach 1.8 was 0.316 inches and for those at Mach 2.5 was 0.283 inches, which was a decrease in response of about 10 percent. The free field cruciform microphone microphone data indicated that the average wave angle (angle between wave front and ground) was 43° for those XB-70 missions at Mach 1.8 and 28° for those at Mach 2.5.

The effect of aircraft vector (angle between aircraft flight track and structure element) on plate response was studied by comparing the measured displacements for overhead flights for DR-2 and BRI-2. The DR-2 wall was subjected to a vector that was nearly head-on (flight track nearly perpendicular to wall surface) and the BRI-2 wall was subjected to a vector that was

nearly side-on (flight track nearly parallel to wall surface). For overhead missions of the XB-70, B-58, and F-104, the average displacements of DR-2 were 0.0317, 0.0318, and 0.0223 inches respectively; and of BRI-2 were 0.0152, 0.0165, 0.0115 inches respectively. Thus, the displacements due to a side-on vector were approximately fifty percent of the displacements due to a head-on vector. Similar results were also found at White Sands.<sup>3</sup>

## PREDICTED DISPLACEMENTS

The predicted plate displacements for the three walls in the test houses were computed using methods explained in Appendix A and Equation A-3:

$$\Delta = \frac{P}{K} DAF = \Delta_{\text{static}} DAF$$
 (A-3)

where

 $\Delta$  = Predicted dynamic displacement,

P = Total load on the element,

K = Element stiffness, and

DAF = Dynamic amplification factor

In the following discussion, the methods used to determine the variables, P, K, and DAF are briefly summarized.

The first variable considered was the stiffness, K, of the wall plates. Three methods to determine the stiffness of the walls are presented here for the east wall of DR-2. It was assumed that the instrumented stud acted as a simple beam and spanned between the top and bottom plates.

The first method used to calculate the stiffness of the DR wall of E-2 was an approximate approach discussed in Reference 4. The stiffness was given as:

$$K = 76.8 \frac{E1}{L^3}$$
 (7-1)

where

1 = Moment of Inertia of 2 x 4 stud = 6.45 in<sup>4</sup>,

E = Elastic modulus of wood =  $1.76 \times 10^6$  lb/in<sup>2</sup>, and

L = Length of stud = 7.5 ft.

Substituting these values,

K = 1200 lb/in.

A second approach to the calculation of the stiffness was outlined in Reference 5, and K was computed from:

$$T = 2\pi \sqrt{\frac{mK_{lm}}{K}}$$
 (7-2)

which can be rewritten as:

$$K = \frac{mK_{\parallel m} 4\pi^2}{\tau^2} \tag{7-3}$$

where:

T = Natural period of the first mode of vibration of the structural element and was determined to be approximately 0.05 sec. from actual tests of the wall and from the integrated accelerometer records,

m = Mass of the wall tributary to the stud = 1.23 lb- $sec^2$ /ft.

K<sub>Im</sub> = Load-mass transformation factor which relates the simple beam (stud) to the lumped mass single degree of freedom system and is equal to 0.78.

Substituting these actual values,

$$K = 1250 lb/ln.$$

Using a third approach, the total predicted displacement was obtained from Equation A-4, Appendix A, for the special case of a simply supported beam with a uniform mass and load<sup>5</sup>. The displacement was given by:

$$\Delta(x) = \frac{4q}{m\pi} \sum_{n=1}^{\infty} \frac{1}{n\omega_n^2} \left( DAF \right)_n \sin \frac{n\pi x}{L}$$
 (7-4)

where

 $n = 1, 3, 5, \ldots$  and indicates the various mode shapes,

x = Distance from support to point of interest,

q = Load in 1b/ft on the element,

m = mass of the element, and

$$\omega_{\rm n} = \frac{{\rm n}^2 \pi^2}{12} \sqrt{\frac{\rm EI}{\rm m}} \tag{7-5}$$

Note that since n appears in the denominator higher modes were relatively unimportant. Taking n = 1, this equation was rewritten as:

$$\Delta(x) = \frac{4qL^4}{\pi^5 El} \text{ (DAF) (sin } \frac{\pi x}{L} \text{)}$$
 (7-6)

Letting x = L/2 and noting that the total load on the element P = qL,

$$\Delta = \frac{4PL^3}{\pi^5 EI} \text{ (DAF)}$$

or

$$\Delta = \frac{P}{K} \times DAF$$

$$K = \frac{\pi^5 \bar{E}i}{4} = 76.5 \text{ El}$$
 (7-8)

which gives the same result as previously determined.

in a similar manner the stiffnesses of BRI-1 and BRI-2 were determined. The following values of stiffness were used in the computation of the predicted displacements: BRI-1, K = 1930 lb/in; DR-2, K = 1200 lb/in; and BRI-2, K = 1200 lb/in.

It should be noted that in the computation of the stiffness as outlined by the first method, the moment of inertia, I, was taken as that of the stud only. The amount of contribution of the interior and exterior wall surfaces to the stiffness of the assembly was unknown and could only be estimated. It was reasonable to assume that there is no contribution from the loose fitting, lapped exterior siding for small displacements. If it were assumed that the gypsum board and the stud were adequately connected, and that  $E_{\rm wood} = 1.77 \times 10^6 \; {\rm lb/in}^2$  and  $E_{\rm gypsum} = 4.42 \times 10^4 \; {\rm lb/in}^2$  (Reference 2), then the moment of inertia of the assembly (7.18 in<sup>4</sup>) was about ten percent higher than the value for the stud alone (6.45 in<sup>4</sup>). The actual value of I was probably somewhere between these two values. The moment of inertia of the stud alone was used in this analysis, and the use of the higher value would not appreciably effect the results.

Once the stiffness, K, was established, it was necessary to determine P and the DAF. The total load, P, on the stud was assumed to be a uniform load acting on the entire length of the stud. For a load of I psf, the load on each wall was determined as:

$$P = (1.0 \text{ lb/ft}^2)(\frac{16}{12} \text{ ft}) (7.5 \text{ ft}) = 10 \text{ lb.}$$

The static displacement was evaluated for a 1 pst load. For BRI-1,

$$\Delta$$
static = 5.2 x  $10^{-3}$  in/psf

and for DR-2.

$$\Delta$$
static = 8.4 x 10<sup>-3</sup> in/psf.

The above two walls, BRI-I and DR-2, were both subjected to head-on vectors whereas the BRI-2 wall was subjected to a side-on vector. Previous tests at White Sands indicated that the plate displacement due to a side-on vector is approximately 50 percent of the displacement due to a head-on vector. Therefore the predicted displacements for BRI-2 were multiplied by a factor of 0.50. The static displacement for a one psf load for BRI-2 was then

$$\Delta_{\text{static}} = 4.2 \times 10^{-3} \text{ in/psf.}$$

With the above unit values for  $\Delta_{static}$ , it was possible to compute  $\Delta_{static}$  for the actual boom pressure, and the predicted displacement was  $\Delta_{static}$  times the appropriate DAF.

It was assumed in computing the predicted displacements that the wall studs acted as simple beams with a span equal to the distance from the bottom plate to top plate. The displacements of the supports were negligible. For example, results of studies of ground motion caused by sonic booms during Phase II indicated that the maximum ground velocity caused by a sonic boom of approximately 2 psf was less than one percent of the peak velocity of the DR-2 wall. Studies of racking response data (Chapter VIII) indicated that the peak racking displacement of the roof line of E-I was less than 10 percent of the peak displacement of the BRI-I wall. Therefore the displacement of the supports of the wall studs were not utilized in determining the predicted displacements.

The predicted displacements were computed for two different cases of loading. The first case considered was the net pressure loading, which was the outside pressure minus the inside pressure. The load P was determined from the peak net pressure and the DAF spectrum was obtained from the net pressure signature. Outside, inside, and net pressure signatures for the DR-2 wall for typical missions of XB-70, B-58 and F-104 aircraft were compared, Figures 7-14, 7-15, and 7-16. DAF spectra determined from net pressure signatures for DR-2 for typical missions are given in Figure 7-17. Note that the net pressure signatures were slightly distorted N-waves with the negative pulse being somewhat extended, and that this extension only slightly affected the DAF in the frequency range of these walls.

The second case considered was the free field pressure loading. The load P was determined from the average peak positive overpressure from the five free field cruciform microphones, and the DAF used was the average DAF obtained from the free field pressure signatures. (Envelopes of DAF curves from free field signatures have been presented in Figure 5-4.)

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A third case considered was the exterior loading. For this case, the load P could be determined from the peak exterior pressures on the wall and the DAF obtained from the exterior loading signatures. DAF spectra for exterior loading for the DR-2 wall for typical missions are given in Figure 7-18. Comparison of the DAF spectra from exterior loading, Figure 7-18, with those from free field signatures, Figure 5-4, and those from net pressure loading, Figure 7-17, Indicated that for the frequency range of 10-40 cps, all three

spectra were approximately equal. For this reason, it was decided that a detailed analysis of exterior loading was not warranted, so the study was concentrated on net and free field loading.

Predicted displacements based on free field and net pressure signatures were listed in Tables 7-1 through 7-4 for the east wall of BRI-1 and the east wall of DR-2. Predicted displacements based on free field pressure signatures only were listed in Table 7-5 for the north wall of BRI-2. Values in these tables were computed as previously indicated. The static displacements for a 1 psf load were multiplied by the appropriate values of overpressure and DAF to obtain the predicted displacements. For example, for X3-70 mission 13-2 (Table 7-1), the predicted displacement for BRI=1 was:

$$\Delta = (5.2 \times 10^{-3} \text{ in/psf}) (2.00 \text{ psf}) (1.79) = 0.0186 \text{ in.}$$

#### COMPARISON OF PREDICTED AND MEASURED DISPLACEMENTS

Predicted displacements were plotted versus measured displacements for the BRI-1, DR-2, and BRI-2 walls in Figures 7-19 through 7-25. Predicted displacements based on free field signature data versus measured displacements for comparable overhead missions of XB-70, B-58, and F-104 aircraft for the three walls were plotted in Figures 7-19, 7-22, and 7-25. Predicted displacements based on free field signature data values versus measured displacements for XB-70 missions at different offsets and Mach numbers were plotted in Figures 7-20 and 7-23 for the BRI-1 and DR-2 walls respectively. Figures 7-21 and 7-24 show predicted displacements based on net pressure signature data versus measured displacements for the BRI-1 and DR-2 walls.

It was observed from these figure, that measured response or displacement compared closely with predicted response based on both free field and net pressure signature data for overhead and offset flights and for flights of different Mach number. The ratios of the predicted displacement to the measured displacement were computed and listed in Tables 7-1 through 7-5. The average of the ratios for each aircraft are also given. For the case of the predicted displacements computed from free field signature data, the over-all average ratios of predicted to measured displacement were equal to 1.03, 1.05, and 1.00 at the 95 percent confidence level for the BRI-1, DR-2, and BRI-2 walls respectively. For predicted displacements computed from net overpressure data, the average ratios of predicted to measured displacement were equal to 1.00 at the 95 percent confidence level for both the BRI-1 and DR-2 walls. These

values, summarized in Table 7-6, were based on the use of a "t-test". The degree of precision in these results and the probability that the results have this degree of precision were summarized in the table on page 5.5.

#### SUMMARY OF FINDINGS

This section presented the results of the analysis of the wall plate response of three typical walls in test houses E-I and E-2. The walls considered were the east wall of bedroom number one in house E-I (BRI-I), the east wall of the dining room in house E-2 (DR-2), and the north wall of bedroom number one in house E-2 (BRI-2). Predicted displacements were computed and compared with measured displacements for the three walls. Predicted displacements were computed based on peak overpressure and DAF spectra calculated from free field signatures, and on peak overpressures and DAF spectra calculated from net pressure signatures. The effects of flight track offset, Mach number and aircraft vector on plate response were in estigated. In addition, the results of the Phase II tests were compared with those from previous tests. The following summary of findings resulted from these analyses:

I. Peak plate displacements of three typical walls in the two test houses were less than 0.034 inches for sonic boom overpressures of approximately 2 psf. Results from Phase I were of similar magnitude.

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- 2. The DAF spectra curves determined from the free field, exterior and net pressure loading signatures were in significant agreement for structure element natural frequencies from 10 to 40 cps.
- 3. There was an indication that plate response may decrease with an increase in offset for flights of the same altitude and Mach number.
- 4. Plate response decreased slightly with an increase in Mach number for flights of the same altitude and offset. An increase in Mach number from 1.8 to 2.5 for overhead flights of the XB-70 caused a decrease in plate response of approximately 10 percent.
- 5. Peak displacements of a wall subjected to nearly side-on vectors (flight track nearly parallel to the wall surface) were fifty percent of the displacements of an equivalent wall subjected to nearly head-on vectors (flight track nearly perpendicular to the wall surface). Similar results were also found at White Sands.

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7. Plate response could be adequately predicted using peak overpressures and DAF spectra calculated from free field signatures.

The following section, Part B, presents the results of the analysis of the window plate response data.

## B. WINDOW PLATE RESPONSE IN TEST HOUSE E-1

This section presents the results of the analysis of the response of one large window in test house E-I. The window considered was installed in lieu of the large garage door in the test house. The window was larger (nominal 8'-6" by 6'-6") than that normally found in houses and somewhat thinner (0.25") than glass installed for this size of opening. The window was subjected to sonic booms on a trailing vector for all flights. Predicted displacements based on peak overpressures and DAF spectra calculated from free field signatures were computed and compared with the measured displacements.

#### INSTRUMENTATION

The window was instrumented with a centrally located strain gage, SG-3. In addition, microphone ML5 was placed inside the garage and ML6 outside the garage to measure the local pressure signatures. The instruments were in these locations for approximately sixty percent of the XB-70/B-58/F-104 missions. A schematic of the test set-up is shown in Figure 7-26.

TEST RESULTS - MEASURED DISPLACEMENTS

The natural frequencies, p, of the E-I window were given by:

$$p = \frac{2}{\pi} \sqrt{\frac{D}{M}} \left( \frac{m^2}{L^2} + \frac{n^2}{b^2} \right)$$
 (7-9)

corresponding to a deflected shape

$$w = A \sin \frac{m\pi x}{L} \sin \frac{n\pi y}{b}$$
 (7-10)

where

w = displacement - a to the surface

D = window stiffness =  $\frac{E+^3}{12(1-v^2)}$  = 14,000 inch-pounds

 $E = modulus of elasticity = 10 \times 10^6 pounds/square inch$ 

t = window thickness = 0.25 inch

v = Poisson's ration = 0.24

b = dimension of the window in the vertical direction = 80.5 inches

L = dimension of window in the horizontal direction = 104.5 inches

M = mass per unit area = 3.28 pounds/square foot

n = number of half waves in the y direction

 $m = number of half waves in the <math>\times$  direction

The period, T, was

$$T = \frac{2\pi}{D} \tag{7-11}$$

Values of T corresponding to several mode shapes are given in Figure 7-27.

Strain displacements at the center of the window and the corresponding pressure signatures for three typical missions are shown in Figures 7-28, 7-29, and 7-30. It was evident from the strain records that the window response to the sonic booms caused by the F-104, B-58, and XB-70 was primarily in the first mode. The second symmetrical mode, which corresponded to two vertical nodal lines at the third point of the window, was also prese. (Figure 7-27). Values for the periods of the window for these two modes were found from the strain records to be 0.177 and 0.043 seconds which agreed quite closely with the calculated values of 0.170 and 0.043 seconds (assuming simply supported edges). The non-symmetrical modes corresponding to a nodal line through the center of the window could not be measured by the strain gage. If the boom overpressure were applied uniformly over the entire window, these mode shapes would not be excited. The amplitude of the

second symmetrical mode strain was never more than  $\pm$  20 percent of the first mode strain which meant that the corresponding displacement amplitude was 2.2 percent of the first mode displacement.

The recorded strains were converted to displacements as follows: For flexural deformation of the window, the strain in the "x" direction was

$$\varepsilon_{x} = -z \frac{\partial^{2} w}{\partial x^{2}} \tag{7-12}$$

where

z = distance from the middle surface of the window in a direction perpendicular to the plane of the window.

If the deflected shape of the window is taken as:

$$w = A \sin \frac{\pi x}{L} \sin \frac{\pi y}{L}$$
 (7-13)

which corresponds to the first mode response of the window to uniformly applied pressure, the strain at the exterior surface at the middle of the window (x = L/2, y = b/2, z = t/2), is

$$\varepsilon_{\chi} = A_{2}^{\dagger} \left(\frac{\pi}{l}\right)^{2} \tag{7-14}$$

Thus the peak displacement, A, was determined from the known strain values obtained from SG-3, as

$$A = \frac{2L^2}{\pi^2 +} \epsilon_{x} \tag{7-15}$$

Substituting actual values:

$$A = 8900 \ \epsilon_{\times} \tag{7-16}$$

Peak measured displacements for overhead and offset XB-70, B-58, and F-104 missions (including different Mach numbers) for the E-1 garage window are summarized in Table 7-7. Note that the response due to the B-58 was greater than the response due to either the XB-70 or F-104. This was expected, since the DAF spectra from B-58 signatures peaked at about 5 cps (Figure 5-4) and the frequency of the fundamental mode of vibration of this window was approximately 5.7 cps.

#### PREDICTED DISPLACEMENTS

In order to compute the predicted displacements utilizing equation A-3 ( $\Delta = \Delta_{\text{static}} \times \text{DAF}$ ) and free field signature data, certain factors that could affect a large flexible window on a trailing vector were investigated. The

relationship of DAF spectra obtained from free field signatures to DAF spectra obtained from the net (outside minus inside) pressure signatures was evaluated. The relationship of the free field overpressure signatures to the net pressure loading was examined. The investigation of the latter relationship was covered in two parts: outside overpressure versus free field overpressure; and net overpressure versus outside overpressure.

Relationship of " Spectra Computed from Net Ovarpressure and Free Field Signatures:

DAF spectra calculated from net pressure signatures on the E-1 window ware plotted in Figures 7-31, 7-32, and 7-33. It was observed that for the XB-70 and F-104 missions the DAF spectra curves in the range of the second mode vibration (23.2 cps) were higher than for  $^+$  first mode (5.9 cps). However, for the B-58 the DAF curve was higher to: the first mode for one case.

Comparisons of the DAF spectra determined from free field, outside and net overpressure signatures for typical missions were made in Figures 7-34 through 7-42, From these figures, it was apparent that in general no appreciable error would be introduced by substituting the DAF spectra from free field signatures for the DAF spectra from net overpressure signatures.

Figure 1 overpressure and Free Field Overpressure:

The relationship between outside overpressure,  $P_o$ , and free field overpressure,  $P_f$ , for a trailing vector depends on many factors: altitude, offset and Mach number for a given mission; orientation and geometry of the structure, etc. A detailed analysis of these factors and the development of a deterministic model based on theoretical considerations and/or empirical relationships was beyond the scope of this study. However, theoretical approaches to this problem have been discussed by Wiggins 25 and Zumwalt 26.

Since the outside overpressure was caused by a trailing vector of the sonic boom, no reflected waves were superimposed on the original N-wavs. It was reasonable to assume that the ratio  $P_{O}/P_{f}$  is always less than 1.0 for a trailing vector. A study of the overpressure signatures at test house E-1 indicated that in all cases the outside overpressure measured by ML6 was less than the free field signatures.

From the observed data, it was determined that the average ratios of  $P_0/P_f$  for XB-70, B-58, and F-104 missions were 0.80, 0.80, and 0.62 respectively, with extreme values less than 17, 24, and 26 percent of the average values.

A summary of the average ratios of  $P_0/P_f$  is shown in Table 7-8. The overall average of the ratio  $P_0/P_f$  was 0.76 and distribution of the pressure ratios are presented in Figure 7-43.

### Relationship Between Net Overpressure and Outside Overpressure:

The relationship between the free field overpressure and outside overpressure was determined from the experimental data in the preceding section. It was next necessary to determine the relationship between the outside overpressure and the net (outside minus inside) overpressure, so that the net pressure  $P_n$ , could be related to the free field overpressure.

Examination of the data indicated that the average ratios of  $P_n/P_o$  for XB-70, B-58, and F-104 missions were 0.51, 0.57 and 0.63 respectively, with extreme values less than 40, 25, and 16 percent of the average values. A summary of these ratios is shown in Table 7-8. The overall average ratio  $P_n/P_o$  for all missions was 0.57 and the distributions of the pressure ratios are presented in Figure 7-43.

The rise in overpressure within the structure was due to two effects: permeability, or the passage of the pressure wases through openings in the structure; and transmissibility, or increase in overpressure due to an inside volume change produced by the displacement of the structural elements enclosing the volume. In the case of the garage, it was helieved, but not proven, that the permeability was not as significant as the transmissibility because the garage structure was sealed against flow of air. Pieces co carpet were placed on the bottom of the doors to reduce leakage and ail doors were closed during the test mistions.

It was shown in tests conducted by Andrews Associates in Oklahoma City<sup>27</sup> that overpressures within a structure were directly proportional to the displacements of the roof of the structure. In these tests, the inside pressure fluctuations had the same relative magnitude and frequency as the roof deflections when the roof was excited by a harmonic disturbing force.

An analysis was made based on the assumption that the overpressure within the garage could be related to the displacement of the window since the garage was relatively air-tight and since the window was much more flexible than the inclosing walls and roof. It was also assumed that pressure times volume was a constant inside the garage. The inside overpressure due to the displacement of the plate glass window was then determined to be: (Refer to Appendix H for derivation)

$$P_i = P_0 = \frac{P_a B}{P_a B + \frac{\pi^2 V}{4 L B}}$$
 (7-17)

where

$$B = \frac{16L^4 \text{ (DAF)}}{D\pi^6 (1 + \underline{L})^2}$$
 (7-18)

V = enclosed volume

L = length of window

b = height of window

t = window thickness

v = Poisson's ratio

P<sub>a</sub> = atmospheric pressure

P<sub>i</sub> = inside overpressure

P = outside overpressure

DAF = Dynamic Amplification Factor

D = window stiffness =  $\frac{E+^3}{12(1-v^2)}$ 

E = modulus of elasticity

Substituting actual values,  $P_i$  = 0.20  $P_o$ . The net pressure was then  $P_n$  =  $P_o$  -  $P_i$  =  $P_o$  - 0.20  $P_o$  = 0.80  $P_o$ . This ratio was considerably higher than the ratio determined from the recorded data ( $P_n/P_o$  = 0.57). The difference between the calculated ratio of  $P_n/P_o$  and the experimentally determined ratio indicated that the prior assumptions were not entirely correct. An approximate analysis was then made that included the effect of the displacement of the garage walls and roof.

A ratio of  $P_n/P_o$  more nearly equal to the ratio obtained from the experimental data was obtained when the measured displacements of the E-I window (Table 7-1) were compared with the measured displacements of BRI-I wall (Table 7-I); it was determined that  $\Delta$ wall/  $\Delta$ window  $\approx$  0.1. The stiffnesses and orientation of the BRI-I wall were then compared with those of the garage wall. The relationship was expressed as:

 $^{\Delta}$ garage wall  $\approx C_1 C_2 C_3$   $^{\Delta}$ BRI-1 wall

 $C_1$  = Ratio of stiffnesses = (1930/1200)

C<sub>2</sub> ≈ Factor to account for fact that gara e walls were subjected to a side-on or trailing vector ≈ 0.5

C<sub>3</sub> = Ratio of moment of inertia of stud and gypsum board as a unit to moment of inertia of stud alone  $\approx 1.1$ .

where

Refer to Part A of this chapter for explanation of these values. Substituting,

 $\Delta$ garage wall  $\approx 0.9$   $\Delta$ BRI-1 wall  $\approx 1.0$   $\Delta$ BRI-1 wall

From the analysis in Appendix H, it was determined that the inside pressure,  $P_{\parallel}$ , was proportional to the change in volume, V, of the inside of the garage or  $P_{\parallel} \propto V$ . It was assumed that the volume change was in turn proportional to the peak displacement times the gross area of each element, and that the stiffness of the walls was approximately equal to the stiffness of the roof. Therefore, it was evident that

$$\frac{V_1}{V_2} = \frac{\Delta_1 A_1}{\Delta_1 A_1 + \Delta_2 A_2}$$

or  $V_2 = V_1 (1 + \frac{\Delta_2 A_2}{\Delta_1 A_1})$ 

where V<sub>1</sub> = Volume change due to window displacement

 $V_2$  = Volume change due to wall and roof displacement

 $\Delta_2/\Delta_1$  = Ratio of wall to window displacement as previously described in  $\approx 0.1$ 

 $A_2$  = Area of walls and roof  $\approx$  671 sq. ft.

A<sub>1</sub> = A: ea of window  $\approx$  58 sq. f

Substituting these values

 $V_2 \approx 2.2 V_1$ 

and

 $P_i \approx 0.2 (2.2) P_o = 0.44 P_o$ 

or

Pn = Po-P, \$ 0.56 Po

which was approximately equal to the value previously determined from the recorded data (0.57). It was assumed in this approximate analysis that the displacement coall the elements were in phase; that is, all of the elements deflect in, for example, at the same time.

While the previous analysis was admittedly approximate, it did illustres the fact that the inside pressure was related to the displacement of the structure elements. A detailed theoretical analysis taking into account the distribution of the pressure around the structure and the actua; stiffnesses of the different elements should yelld similar results and should

indicate an aircraft effect (different ratios of  $P_n/P_0$  for different aircraft).

One apparent contradiction to the hypothesis that the inside pressure was due primarily to the transmissibility of the structure was that an examination of the strain records and the pressure records indicated that these two curves had different characteristic shapes. However, it may be that all of the structure elements are in phase initially and produce the N-type inside pressure wave and then become out of phase. No data was available to prove or disprove this.

## Relationship Between Free Field and Net Pressures:

The ratio of the net pressure to the free field overpressure based on the recorded data was determined from  $P_n/P_f=(P_o/P_f)~(P_n/P_o)$  and the average values were 0.42, 0.46, and 0.40 for the XB-70, B-58, and F-104 respectively. The overall average value for all aircraft,  $P_n/P_f=0.43$  was used in the computation of the predicted displacements.

## Computation of Predicted Displacements:

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The predicted displacements were computed from methods explained in Appendix A and Equation A-3:  $\Delta$  =  $\Delta_{static}$  DAr

Assuming a uniform load, simply supported edges, and considering only the first mode, the static displacement was computed from formula  $^{28}$ :

$$^{\Delta}static = \frac{16PnL^{4}}{D\pi^{6} \left[1 + (\frac{L}{b})^{2}\right]^{2}}$$
 (7-19)

where L, D, and b are as previously defined, and Pn was the net pressure acting on the window. For a net load of 1 psf, and substituting actual values:

$$\Delta$$
static = 0.13 in/psf.

Predicted displacements of overhead and offset XB-70/B-58/F-104 flights (including different Mach numbers) were listed in Table 7-7.

#### COMPARISON OF PREDICTED AND MEASURED DISPLACEMENTS

Predicted displacements were plotted versus measured displacements for the E-I garage window in Figure 7-44. The ratios of predicted to measured displacements were computed and are listed in Table 7-7. It was observed from this graph and table that the measured and predicted values compare well for the E-I window for the XB-70, B-58, and F-104 missions. By the use

of a t-test it was determined that the average ratio of the predicted to measured displacement was equal to 1.05 at the 95 percent confidence level. The degree of precision in these results and the probability that the results have this degree of precision were summarized in the table on page 5.5.

Even though it was determined earlier that the second symmetrical mode displacement was only 2.2 percent of the first mode displacement for the window, a minor error was introduced into the determination of the window displacement by neglecting the second mode response of the window since the strain is magnified nine times for equal modal displacement amplitudes. Also, it was possible for the second mode strain to be in or out of phase with the first mode causing a net addition or subtraction to the first mode strain on the oscillograph trace. Thus the relatively small second mode displacement could introduce a ± 20% error in the determination of peak strain of the window. This could be the cause of much of the scatter in the compariso of predicted and actual displacements shown in Figure 7-44. Another possible source of error in the computation of predicted displacements was the fact that the small displacement theory was used. Similar computations using large displacement theory should result in smaller predicted displacements.

#### SUMMARY OF FINDINGS

This section presented the results of analyses of the response of the large window in the garage of house E-1. The following findings resulted from these analyses:

- I. Measured displacements compared well with predicted displacements computed from free field data when the free field overpressure data were reduced by an appropriate factor to account for transmissibility, geometry, and orientation of the structure. For the E-I window it was determined that the average ratio of the predicted measured displacement was equal to 1.05 at the 95 percent confidence level for XB-70, B-58, and F-104 missions.
- 2. Large glass windows such as the one in E-I garage respond to a sonic boom loading primarily in the fundamental mode of vibration. A minor excitation of the second symmetrical mode also occurs.
- 3. For the E-I garage window, the maximum stress determined from the strain data for the missions investigated was 790 psi. The corresponding theoretical predicted stress was 980 psi.

- 4. Greater response of the E-I window was measured for B-58 missions than for XB-70 and F-104 missions. This was expected since the DAF spectra curves obtained from B-58 signature data peaked at about 5 cps and the frequency of the fundamental mode of vibration of this window was approximately 5.7 cps.
- 5. Window plate response could be adequately predicted using peak overpressure and DAF spectra calculated from free field signatures. For windows located on the trailing vector side of the structure, the free field data must be reduced by an appropriate factor to account for orientation and geometry of the structure.

The following section, Part C, presents the results of the analysis of the Bowling Alley roof frame response data.

## C. RESPONSE OF THE ROOF FRAME OF THE BOWLING ALLEY, E-3

The Bowling Aliey, E-3, was located approximately two miles north of test houses E-1 and E-2. The building was 144' by 75' in plan with steel frames spanning approximately 118 feet and located at 25 ft. centers. The steel deck roof was supported by purlins at 5 ft. centers. This discussion presents the results of the analysis of the response to sonic boom loading of one of the long span roof frames.

#### INSTRUMENTATION

Strain gages, an accelerometer, and pressure microphones were installed on the steel frame. Strain gages (SIL and S2L) were located on the bottom flange at mid-span and at one quarter-span point. An accelerometer (A5V) was mounted on the bottom flange at mid-span to measure vertical accelerations. In addition, pressure microphones (M4 and M2) were located above and below the roof deck at midspan to determine the actual or net loading on the roof structure. The locations of these instruments are given in Appendix B. Free field data was not available at the Bowling Alley and the nearest instruments for measuring free field overpressures were located at the test houses.

#### TEST RESULTS

Peak strains in the bottom flange at midspan of the frame for several SB-70, B-58, and F-104 missions are listed in Table 7-9. The maximum stress

in the bottom flange was approximately 450 psi. Peak vertical displacements of the center of the building frame for XB-70 mission 12-2 and B-58 mission 12-1 were 0.19" and 0.11" respectively.

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Overpressure records for typical XB-70, B-58, and F-104 missions are shown in Figures 7-45 through 7-50. These figures show the outside overpressure (M4), inside overpressure (M2), and net overpressure 'outside minus inside). Note that the net pressure signatures were different from the typical free field signatures.

DAF spectra determined from these net overpressure signatures are shown in Figures 7-51, 7-52, and 7-53. Since the shape of the net overpressure signatures were different from those of the free field signatures, the corresponding DAF spectra were different.

In the previous sections on wall and window plate response, the measured displacements were compared with predicted displacements computed from equation A-3, Appendix A:  $\Delta = (P/K)\cdot DAF$ . For the case of the walls and windows, the stiffness, K, was readily determined. However, for the Bowling Alley, the quantities P·DAF based on free field and net overpressure data were compared. The calculated stiffnesses of the steel frame were based on numerous assumptions of connection rigidity and other factors, many of which were uncertain due to looseness of some connecting bolts, etc.

As previously mentioned, the displacements,  $\Delta$ , of the center of the frame were computed from

$$\Delta = \frac{P}{K} DAF$$

where

P = Total net load on the girder

DAF = Dynamic amplification factor obtained from the net pressure signature

K = Frame stiffness

which can be rewritten

$$K = \frac{P \cdot DAF}{\Delta}$$

Sinc the displacement  $\Delta$  was proportional to the strain  $\varepsilon$  for each aircraft,

$$\frac{P \cdot DAF}{C} = Constant = C$$

where P was taken as the peak pressure on the roof. This should, of course, be true for all aircraft and the average values of C obtained for the XB-70, B-58, and F-104 missions should be equal. Actual and average values of C for the three aircraft are listed in Table 7-9. By the use of a t-test, it was found that the average values for all three aircraft were equal at the 95% confidence level.

The quantity C was also computed for the free field signature data, where P was the average free field peak overpressure at test house E-2 and DAF was an average value from DAF spectra for the free field signatures at E-2. These values are listed in Table 7-10. The average values of C obtained from the free field data were not equal for the three aircraft, and also were not equal to the values obtained from the net overpressure data. The ratio of C based on free field signature data to C based on net overpressure data for each aircraft were compared (which was the same as comparing P·DAF). The predicted displacements using free field signature data would have been an average of 1.26, 2.03, and 1.91 times the predicted displacements based on net overpressure data for XB-70, B-58, and F-104 missions respectively. Inherent in this conclusion was the assumption that the free field signatures at the Bowling Alley were the same as the test houses.

#### SUMMARY OF FINDINGS

This section presented the results of the analyses of the response of a roof frame of the Bowling Alley, E-3, to sonic boom loading. The following findings resulted from these analyses:

- 1. The maximum stress due to sonic boom loading in the bottom flange of the building frame at mid-span was approximately 450 psi.
- 2. Peak vertical displacements of the center of the building frame for XB-70 mission 12-2 and B-58 mission 12-1 were 0.19" and 0.11" respectively. Free field peak overpressures near E-2 for these missions were 2.19 psf and 2.39 psf respectively.
- 3. The shape of net overpressure signatures on the roof of the Bowling Alley measurably differed from those for typical free field N-waves.
- 4. DAF spectra determined from net overpressure signatures differed from spectra determined from typical free field N-waves.

MEASURED RESPONSE VS PREDICTED RESPONSE EAST WALL BR-1, E-1 (CHANNEL 202) BASED ON FREE FIELD SIGNATURES TABLE 7-1

Average AP AP AM	66.0	1.08	1.22	1.02	60.1
AND MA	0.89	80	1.22	0.99 0.97 0.97 1.07	1.24
Measured Displacement AM inches	0.0208 0.0187 0.0211 0.0198	0.0179	0.0094 0.0108 0.0144	0.0205 0.0194 0.0193 0.0188 0.0184	0.0127 0.0129 0.0131 0.0136
Predicted Displacement AP inches	0.0186 0.0198 0.0210 0.0200	0.0194	0.0114 0.0128 0.0180	0.0203 0.0200 0.0187 0.0201 0.0192	0.0135 0.0132 0.0162 0.0132
DAF	1.79	1.72	1.67		1.25
Average Pressure P	2.00 2.18 2.29 2.20	2.19	1,32	2.39 2.21 2.34 2.25 2.25	2.02 2.01 2.31 1.95
Offset K ft	0 ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○ ○	0.2	71.3 68.2 32.1	W-W00.00	7.9 7.0 2.1
Mach	<u> </u>	2.5	22.5	26.5.5.5.	4444
Altitude K ft	60.2 60.6 59.7 60.3	60.3	60.3 60.0 59.4	35.25 35.25 35.25 35.25 5.35 5.35 5.35 5	22.0 20.0 20.2 20.5
Misson	13-2 15-1 16-2 113-2	12-2	7-3 8-3 9-1	8-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	12-3 13-3 15-3 13-3
Aircraft	XB-70	XB-70	XB-70	B-58	F-104

MEASURED RESPONSE VS PREDICTED RESPONSE

BASED ON NET PRESSURE SIGNATURES

EAST WALL BR-1, E-1 (CHANNEL 202)

2.30 1.50 2.32 1.41	K ft psf  6.4 2.30 1.50 9.5 2.32 1.41	Mach Offset P DAF  K ft psf  1.8 6.4 2.30 1.50 1.8 9.5 2.32 1.41	Altitude         Mach         Offset         P         DAF           K ft         K ft         psf           60.2         1.8         6.4         2.30         1.50           60.6         1.8         9.5         2.32         1.41	Mach Offset P DAF  K ft psf  1.8 6.4 2.30 1.50 1.8 9.5 2.32 1.41
2.30 2.32	K ff psf 6.4 2.30 9.5 2.32	K ff psf 1.8 6.4 2.30 1.8 9.5 2.32	K ff psf 1.8 6.4 2.30 1.8 9.5 2.32	K ft K ft psf 60.2   1.8   6.4   2.30 60.6   1.8   9.5   2.32
2.30 2.32	K ft psf 6.4 2.30 9.5 2.32	K ft psf 1.8 6.4 2.30 1.8 9.5 2.32	K ft K ft B 6.4 2.30 60.6 1.8 9.5 2.32	13-2   60.2   1.8   6.4   2.30   15-1   60.6   1.8   9.5   2.32
	K ## 6.4 9.5	Mach Offset K ft 1.8 6.4 1.8 9.5	K ft         K ft           60.2         1.8         6.4           60.6         1.8         9.5	Mission         Altitude         Mach         Offset           K ft         K ft         K ft           13-2         60.2         1.8         6.4           15-1         60.6         1.8         9.5
		Mach	K ft 60.2 1.8 60.6 1.8	Mission Altitude Mach  K ft  13-2 60.2 1.8 15-1 60.6 1.8

TABLE 7-3

MEASURED RESPONSE VS PREDICTED RESPONSE

BASED ON FREE FIELD SIGNATURES EAST WALL DR, E-2 (CHANNEL 404)

Average AP		<u>-0.</u>	1.13	4	<del>-</del> 00-1	4
9 N		.02	05	1.24	0.03	<u> </u>
Measured Displacement AM	inches	0.0298 0.0313 0.0339	0.0277 0.0260 0.0293	0.0152 0.0192 0.0272	0.0311 0.0323 0.0320	0.0215
Predicted Displacement AP	Inches	0.0300 0.0320 0.0342	0.0318 0.0306 0.0340	0.0169 0.0212 0.0298	0.0310 0.0332 0.0316	0.0246
DAF		1.79	1.75	1.72	1.67	i.46 I.35
Average Pressure P	psf	2.00 2.18 2.29	2.19 2.09 2.41	1.32	2.21 2.34 2.25	2.01
044581	<del>*</del>	4.0° 4.0° 6.0°	0.2 0.0 13.3	71.3 68.2 32.1	. 0.0 3.0	3.4
X 40		∞ ∞ ∞ 	2.5		55.55	44
Aititude	<b>☆</b>	. 60.2 60.6 59.7	60.5 59.4 59.7	60.3 60.0 59.4	35.9 39.6 39.7	20.0
Mission		13-2 15-1 16-2	12-2	7-3 8-3 9-1	- 2-7- - 2-2-	13-3
Aircraí		XB-70	XB-70	XB-70	B-58	F-104

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MEASURED RESPONDE VS PREDICTED RESPONSE
BASED ON NET PRESSURE SIGNATURES
EAST WALL DR, E-2 (CHANNEL 404)

Average AP AM		1.08	1.09	1.26
AM W		0.98	1.04	1.28
Measured Displacement AM	inches	0.0298 0.0313 0.0339	0.0311	0.0215 0.0231 0.0228
Predicted Displacement AP	inches	0.0346 0.0345 0.0332	0.0324 0.0390 0.0351	0.0275 0.0282 0.0290
DAF		1.66 1.62 1.62	1.55	1.40
Average Pressure P	pst	2.50 2.55 2.45	2.50 2.71 2.82	2.35 2.30 2.32
Offset	<b>★</b>	6.4 7.0	2.5 3.0	4 0 0.
Mach		8 8 8	1.65	444
Altitude	*  	60.2 60.6 59.7	35.9 39.6 39.7	20.0 20.2 20.6
Mission		13-2	13-1	13-7 15-3 16-3
Aircraft		XB-70	B-58	F-104

MEASURED RESPONSE VS PREDICTED RESPONSE

BASED ON FREE FIELD SIGNATURES

NORTH WALL BR-1, E-2 (CHANNEL 406)

Altitude
φ
<b>∞</b> .
35.9 1.65 2.5
_
39.7 1.65 3.0
20.0 1.4 3.4

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TABLE 7-6

# MEASURED RESPONSE AND PREDICTED RESPONSE

# TRUE VALUES OF THE RATIO OF PREDICTED

## TO MEASURED DISPLACEMENT AT 95% CONFIDENCE LEVEL

			Predicted Displacement
Wall	House	Pressure Signature	Measured Displacement
BR-I	E-1	Free Field	1.03
DR	E-2	Free Field	1.05
BR-I	E-2	Free Field	1.00
BR-I	E-1	Net	1.00
DR	E-2	Net	1.00

TABLE 7-7

MEASURED RESPONSE VS PREDICTED RESPONSE

BASED ON FREE FIELD SIGNATURES

E-i GARAGE WINDOW

		Average Pressure		Predicted Displacement	Measured Displacement	ΔΡ
Aircraft	Mission	P <sub>f</sub>	DAF	ΔΡ	ΔΜ	ΔΜ
		psf		Inches	Inches	
XB-70	4-2	2,64	1.30	0.198	0.152	1.30
	5-2	1.20	1.42	0.098	0.058	1.69
	8-3	1.38	1.42	0.112	0.065	1.72
	11-3	2.09	1.45	0.174	0.107	1.63
	12-2	2.19	1.40	0.i77	0.157	1.13
	16-2	2.30	1.50	0.198	0.153	1.29
	113-2	2.20	1.54	0.196	0.218	0.90
B-58	3-1	2.52	1.72	0.250	0.194	1.29
	4-1	2.11	2.02	0.246	0.246	1.00
	5 <b>-</b> 1	0.68	1.95	0.077	0.068	1.13
	8-1	2.40	1.85	0.254	0.290	0.87
	!1-2	1.86	1.92	0.206	0.204	1.02
	12-1	2.63	1.65	0.250	0.253	0.99
	16-1	2.60	1.90	0.285	0.244	1.17
	113-1	2.49	1.70	0.243	0.250	0.97
F-104	3-4	2.36	0.88	0.120	0.182	0.66
•	12-3	2.02	0.91	0.105	0.094	liz
	113-3	1.95	1.00	0.131	0.112	1.17

TABLE 7-8

PEAK FRESSURE RELATIONSHIPS FOR E-1 WINDOW

	P <sub>f</sub>	Po	P <sub>n</sub>	P <sub>C</sub>	P <sub>n</sub>	P <sub>n</sub>
Mission	psf	psf	psf ·	Pf	<u></u>	Pf
		F-104				
3-4	2.36	1.56	0.90	.66	.58	.38
11-1 12-3	2.01 2.02	1.56 1.02	1.14 0.60	.78 .50	.73 .59	.57 .30
14-3	1.95	1.10	0.75	.56	.68	.38
16-3	2.05	1.00	0.57	.49	. 57	.28
113-3	1.95	1.44	0.90	.74	.63	.46
Average				.62	.63	.40
		<u>9-58</u>				
3-1	2.52	1.68	0.78	.67	.47	.31
4-1	2.11	2.04	1.20	, 99	.59	.57
5-1	0.68	0.60	0,50	, 88	, 50	.44
6-1	1.30	1.28	0,68	99	. 53	.52
8-1 11-2	2.40 1.86	2.10 1.68	1.48 0.96	91	. 11 . 51	.62 .52
12-1	2.49	1.68	1.14	6,29	. 68	.46
14-2	2.63	1.87	0.98	71	,55	, 37
16-1	2.30	1.40	0,90	6.1	, 64	, 39
113-1	2.61	1.89	0,96	. 15	.51	. 37
Average				. 80	.51	.46
		XB-70				
4~2	2.64	2.64	1.04	. 17	.51	. 39
5-2	1.20	1.20	0.30	.80	.3;	. 25
6-2	1.85	1.85	0.90	.76	.64	.49
8-3	1.38	1.38	0.35	.73	. 35	.25
11-3	2.09	2.09	1.08	.93	.57	.52
12-2	2.19 2.20	2.19 2.20	1.11	.80 .77	.60 .59	.51 .46
14-1 16-2	2.30	2.30	0.99		.56	.44
113-2	2.20	2.20	1.00		.52	.45
Average				.80	.51	.42
Average for All	Missions			.76	.57	.43

TABLE 7-9
BOWLING ALLEY

# NET PRESSURE RESPONSE DATA

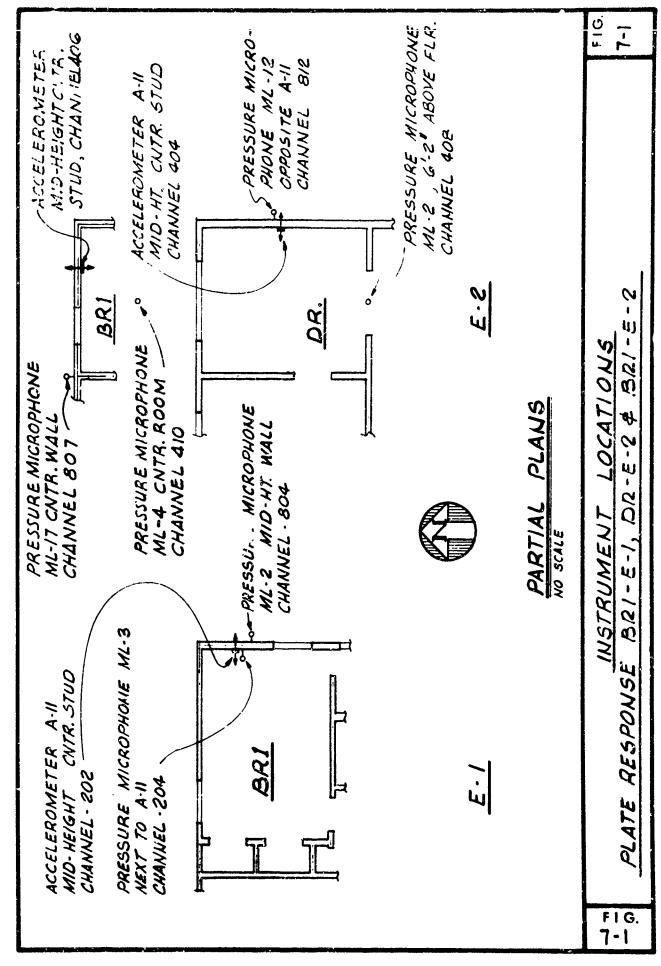
Aircraft	Mission	Strain E	Net Pressure P	Net DAF	C = P·DAF	Cave
		<u>μ ln/ln</u>	psf			
X8-70	2-1	4.9	0.69	1.30	0.183	0.189
	9-1	12.2	1.63	1.60	0.214	
	12-2	14.9	1.99	1.48	0.198	
	13-2	12.2	2.06	1.07	0.182	
	113-2	13.9	2.25	1.04	0.168	
B-58	2-3	12.2	2.52	0.72	0.149	0.174
	9-2	9.8	1.67	0.90	0.153	
	12-1	10.6	2.05	0.98	0.190	
	13-1	12.2	1.62	1.50	0.199	
	113-1	10.6	1.69	1.12	0.179	
F-104	2-4	4.9	1,06	0.95	0.205	0.210
	9-3	4.9	0.9ს	1.05	0.206	
	12-3	5.3	1.99	0.61	0.229	
	13-3	6.1	1.71	0.82	0.229	
	113-3	4.0	1.28	0.56	0.179	

TABLE 7-10

BOWLING ALLEY

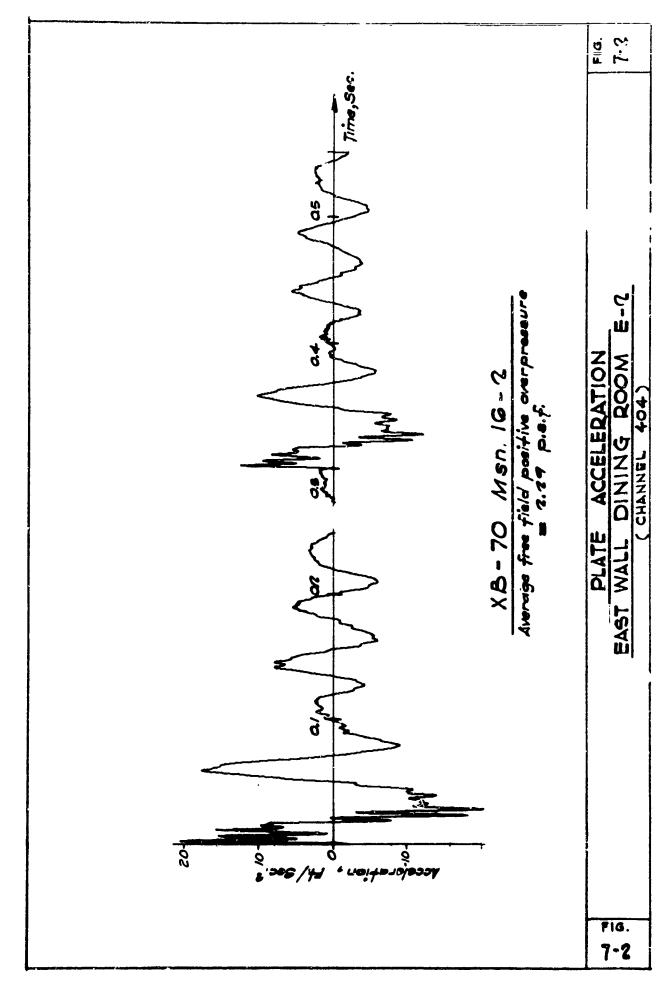
FREE FIELD RESPONSE DATA

<u>A:rcraft</u>	Mission	Strain 	Free Field Pressure @ E-2 P	Free Field DAF	$C = \frac{P \cdot DAF}{\varepsilon}$	C <sub>a<b>ve</b></sub>
		<u>μ in/in</u>	psf			
XB70	9-1	12.2	2.09	1.37	0.232	0,238
	12-2	14.9	2.19	1.47	0.216	
	13-2	12.2	2.00	1.55	0.255	
	113-2	13.9	2.20	1.57	0.247	
B-58	9-2	9.8	2.71	1.42	0.391	0.354
	12-!	10.6	2.39	1.51	0.340	
	13-1	12.2	2.21	1.65	0.300	
	113-1	10.6	2.61	1.56	0.383	
F-104	9-3	4.9	1.54	1.22	0.384	0.401
	12-3	5.3	2.10	1.02	0.404	
	13-3	6.1	2.01	1.01	0.335	
	113-3	4.0	1.95	0.5 '	0.485	

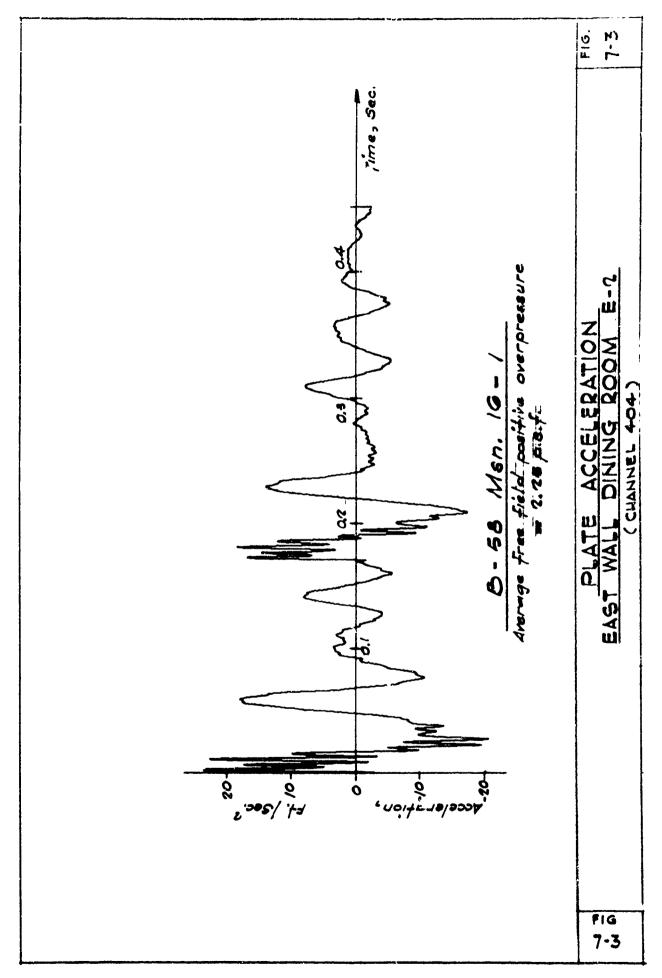


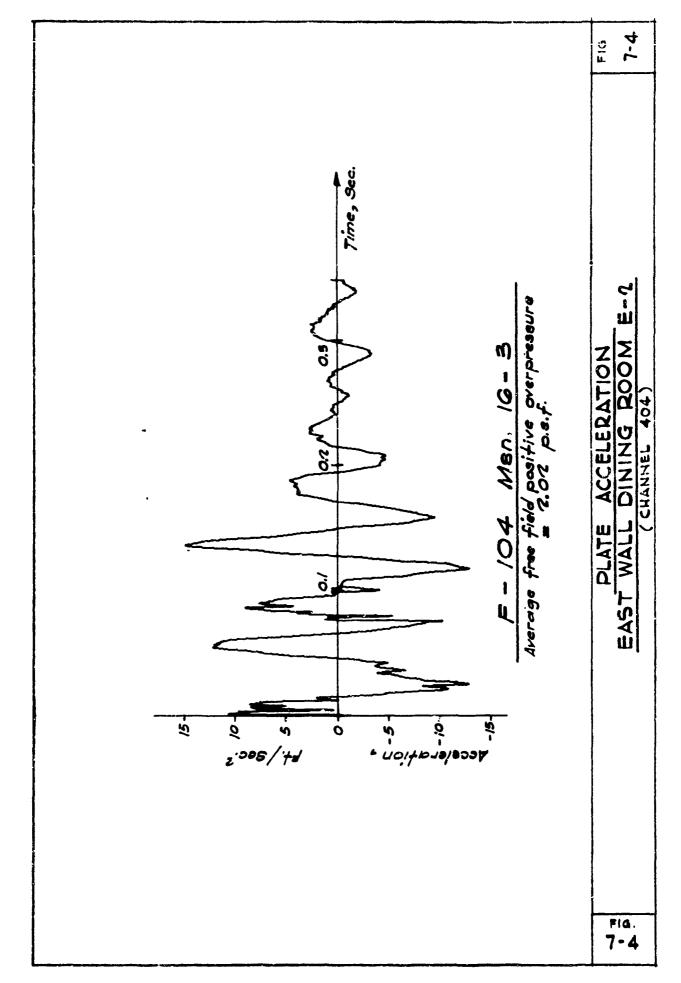
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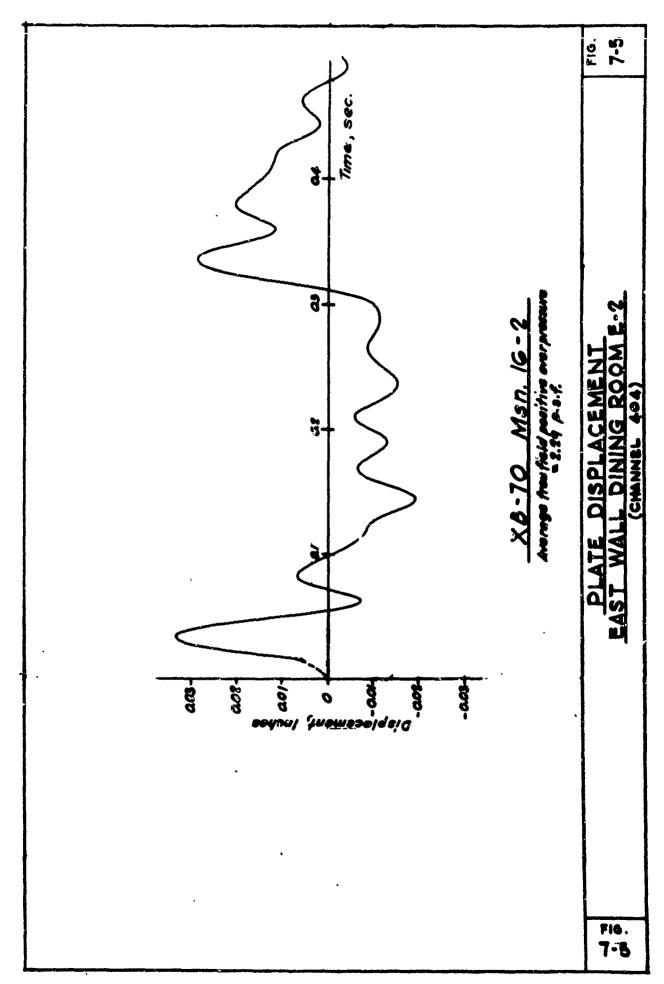
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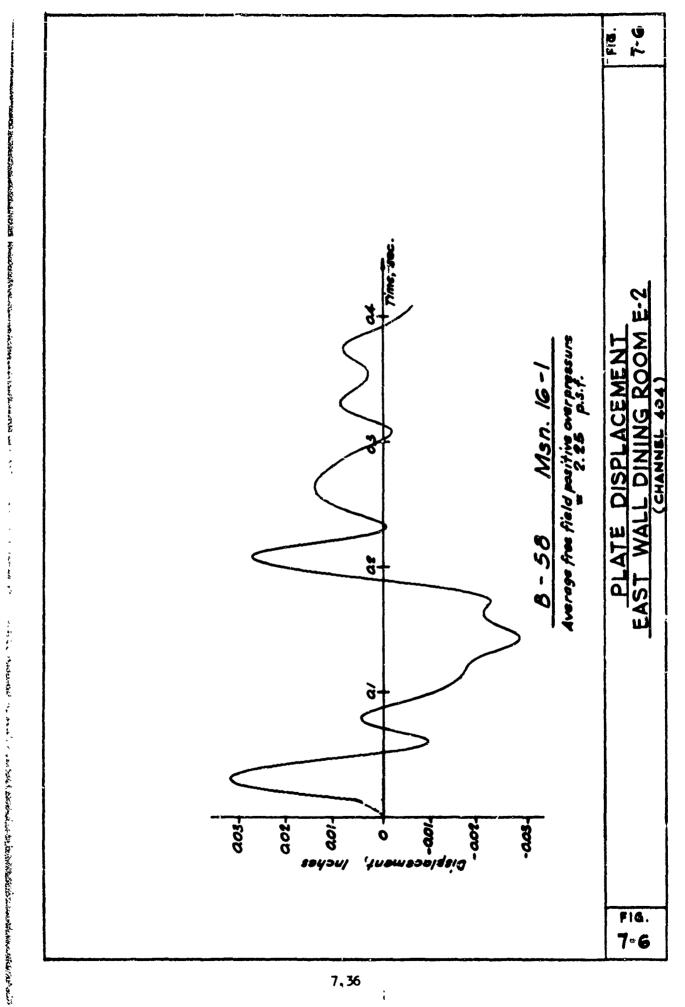
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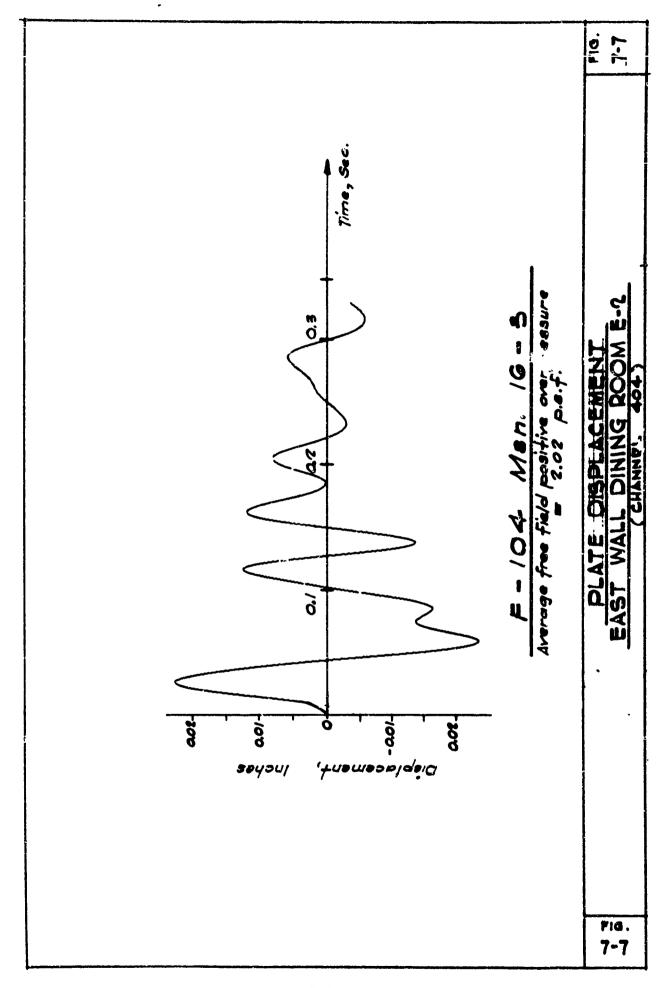






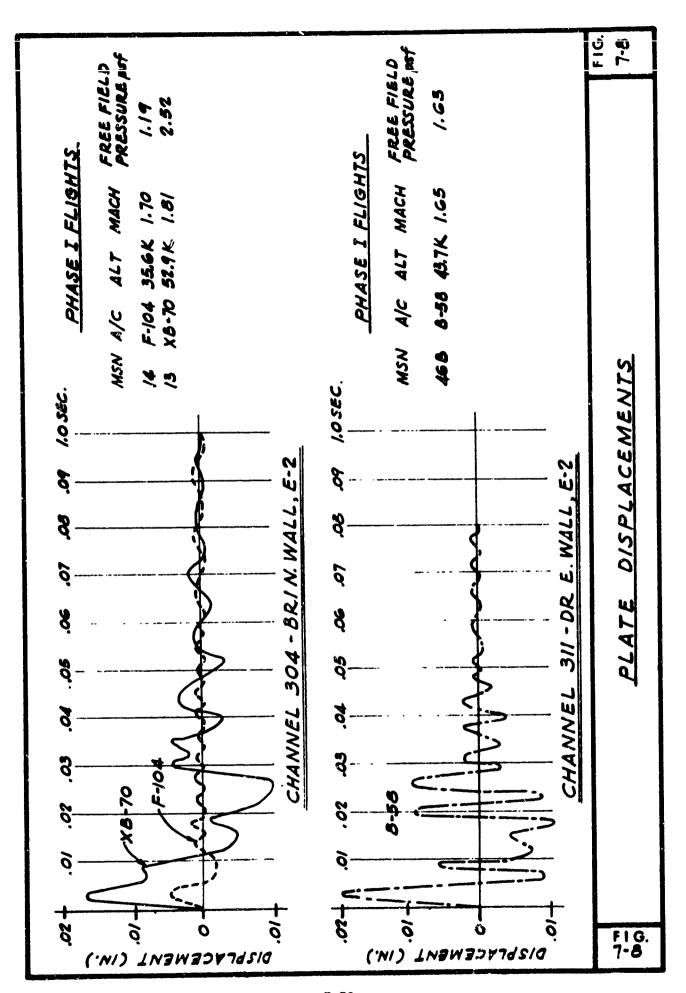
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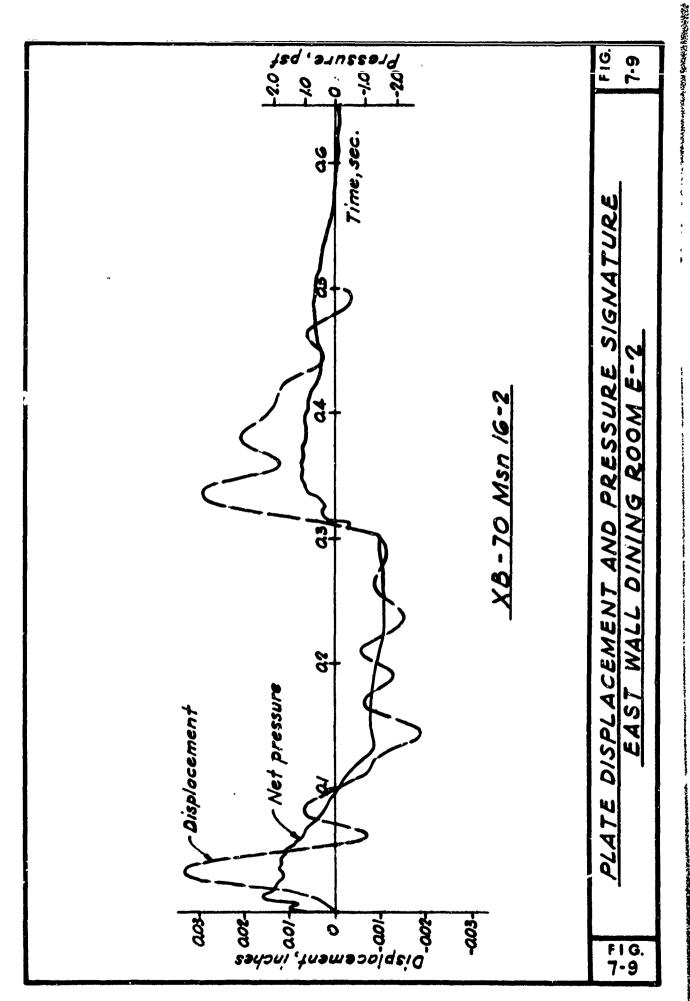
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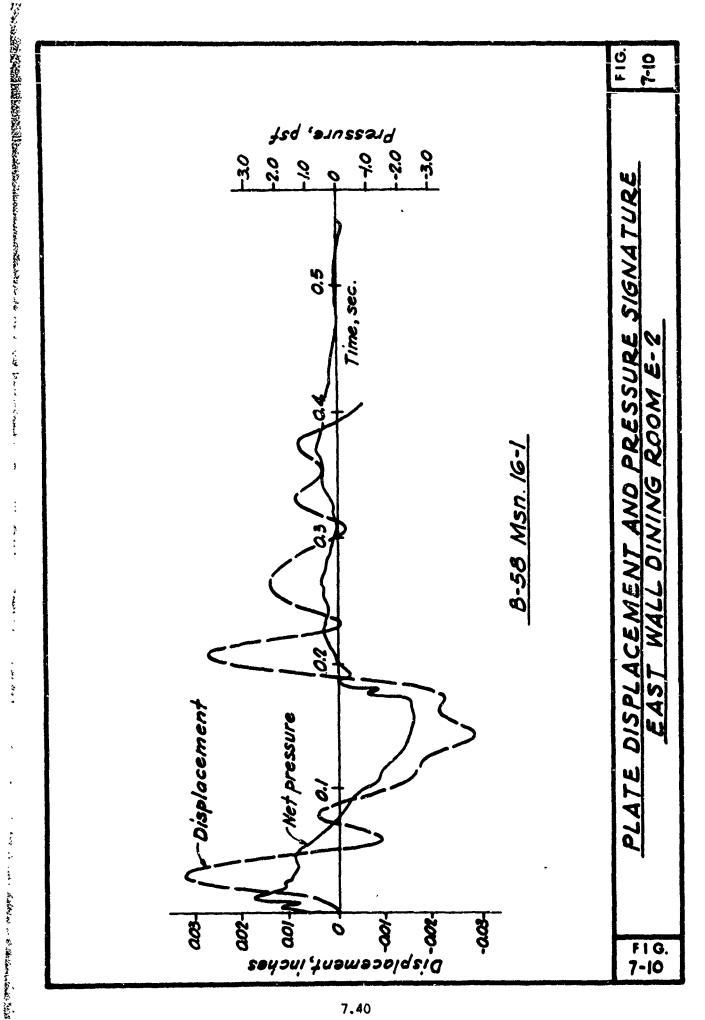


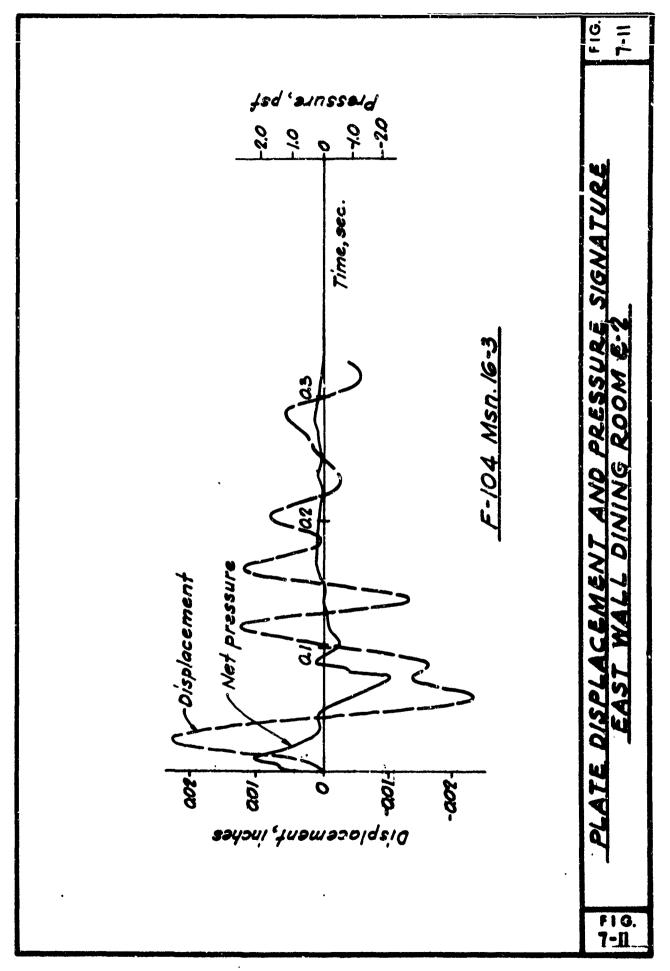
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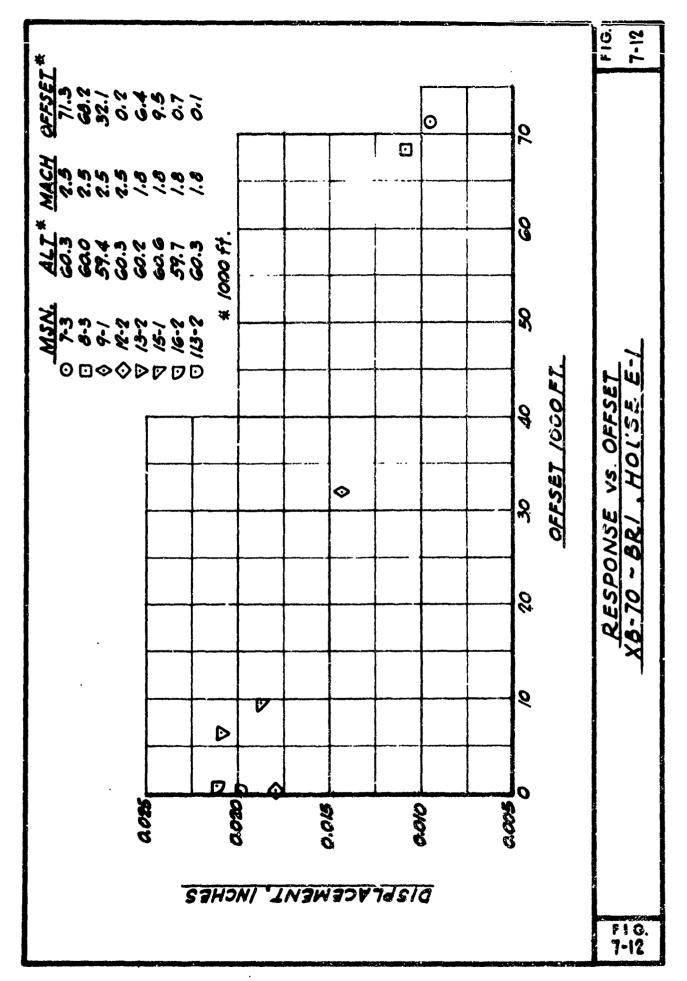
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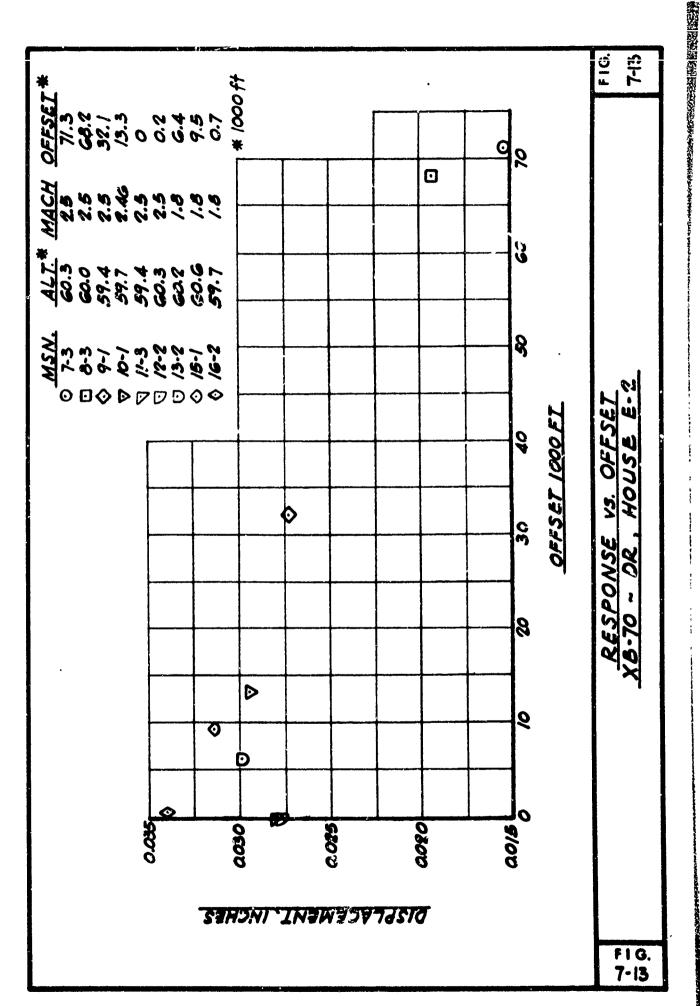


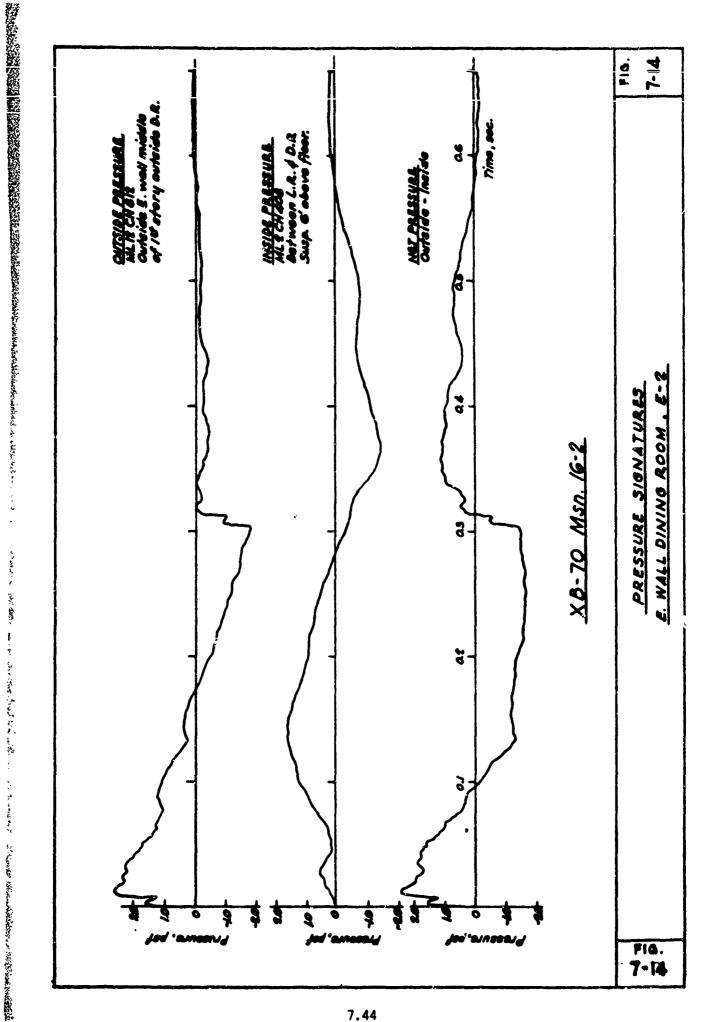
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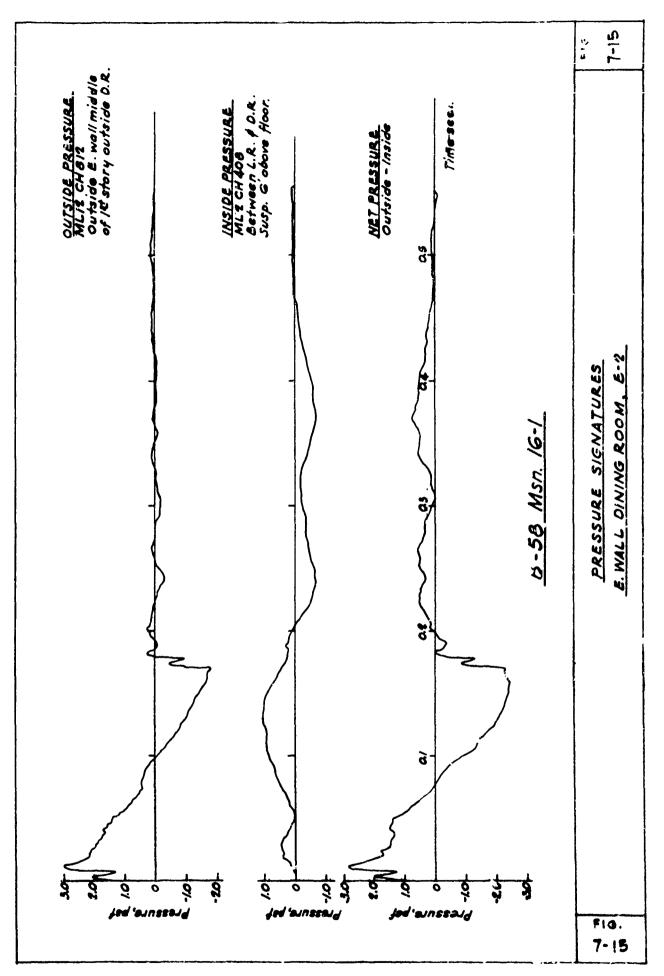








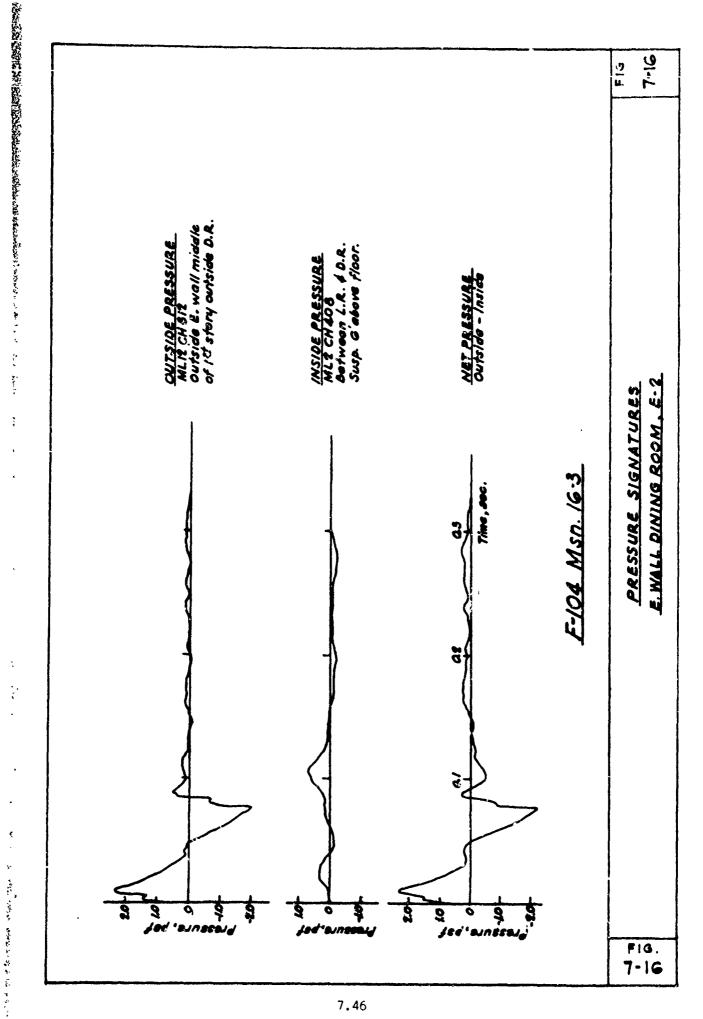


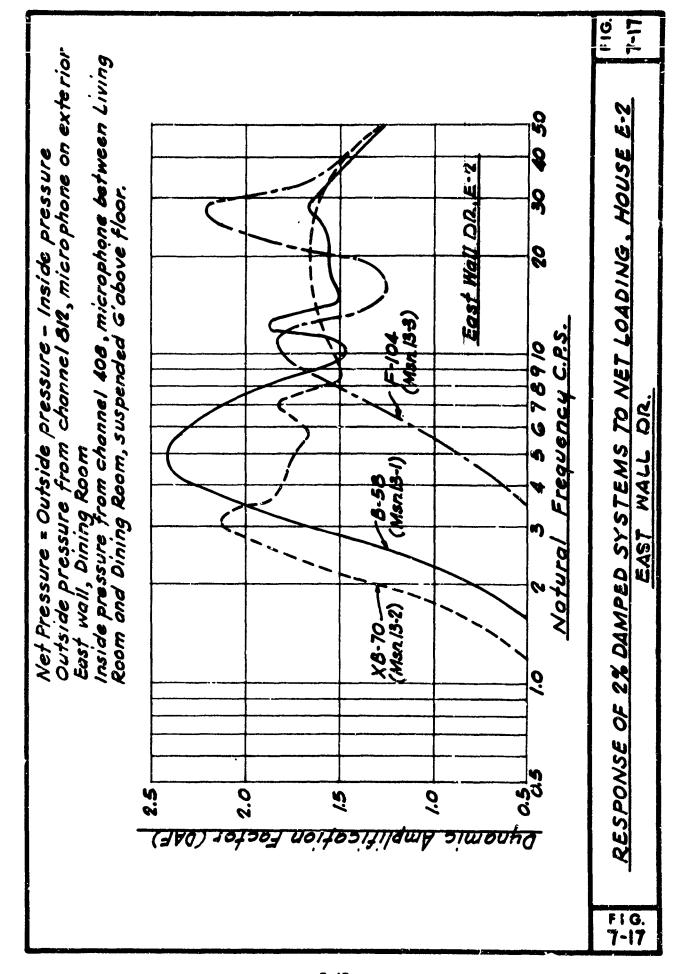


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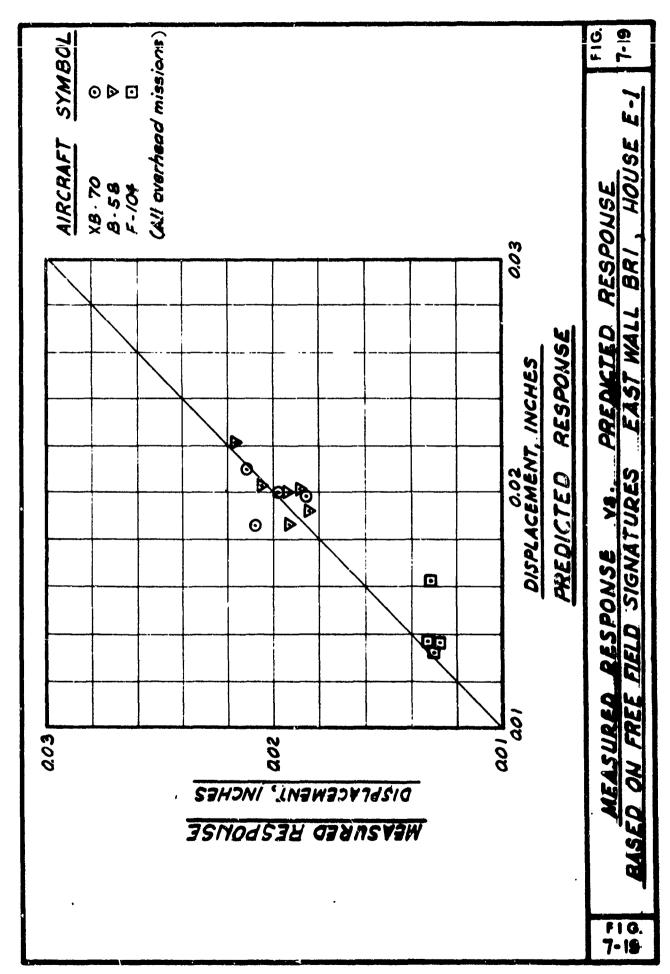
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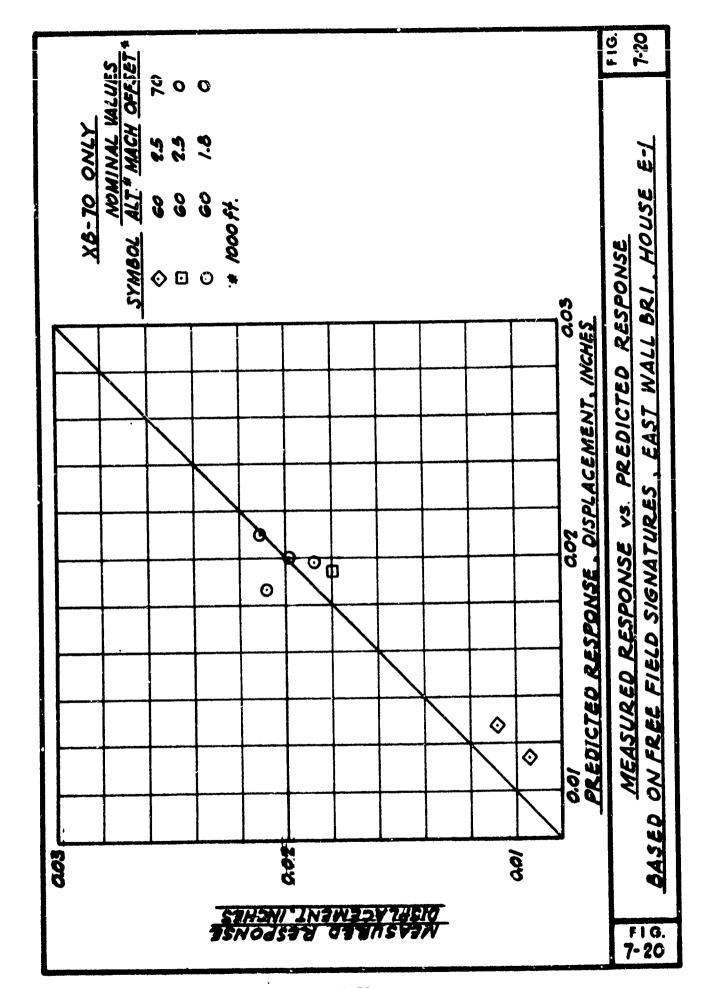
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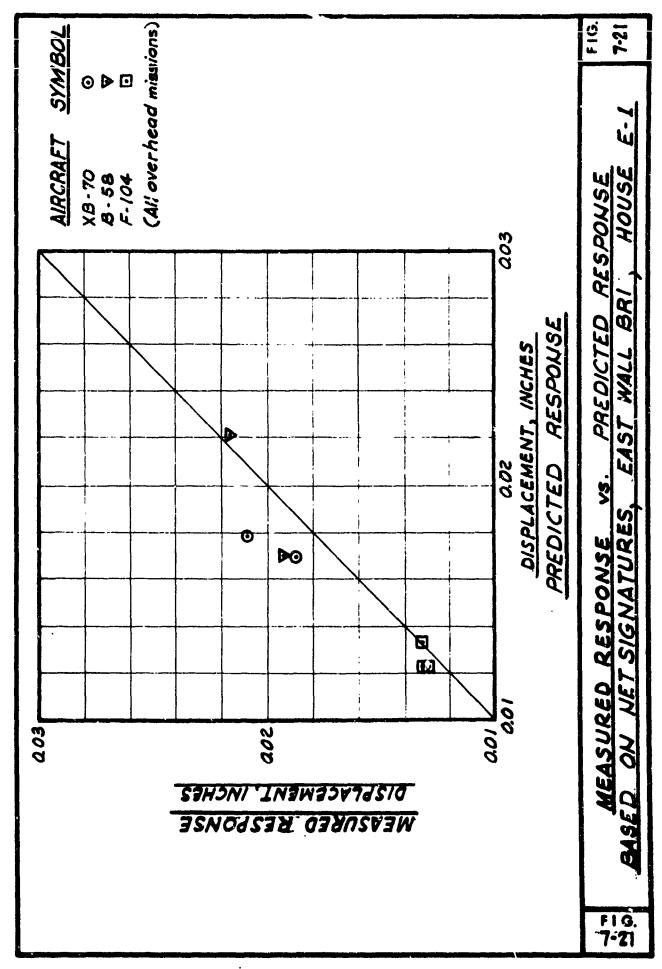
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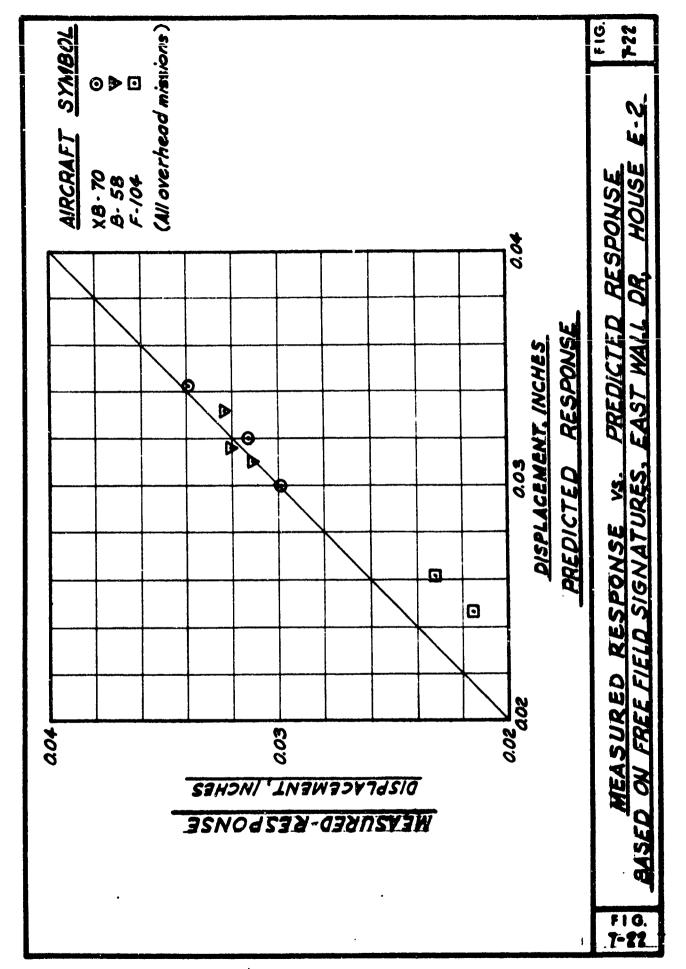
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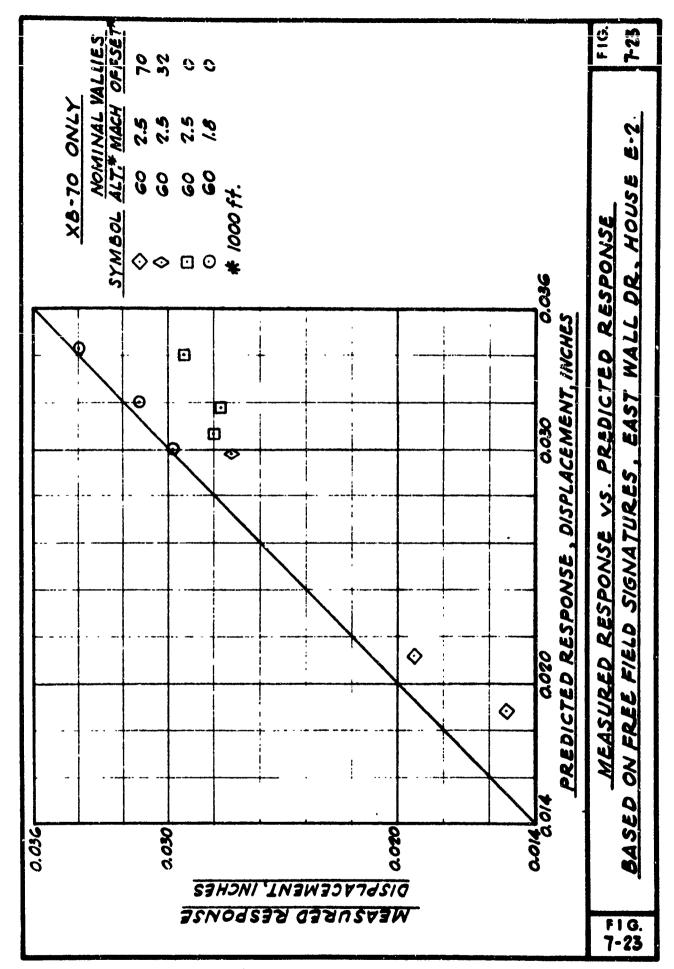


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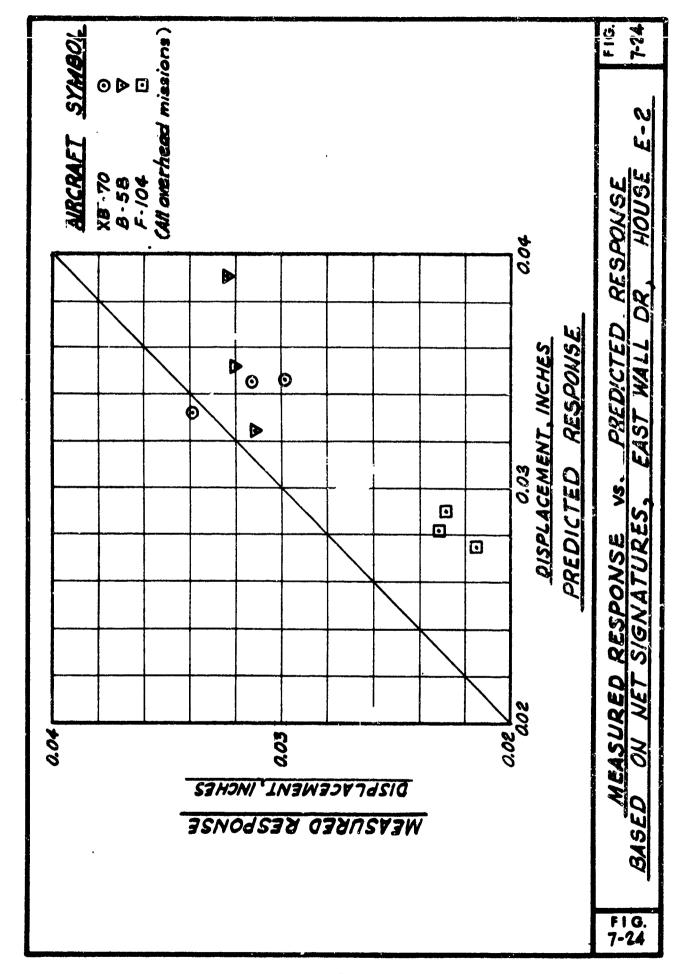


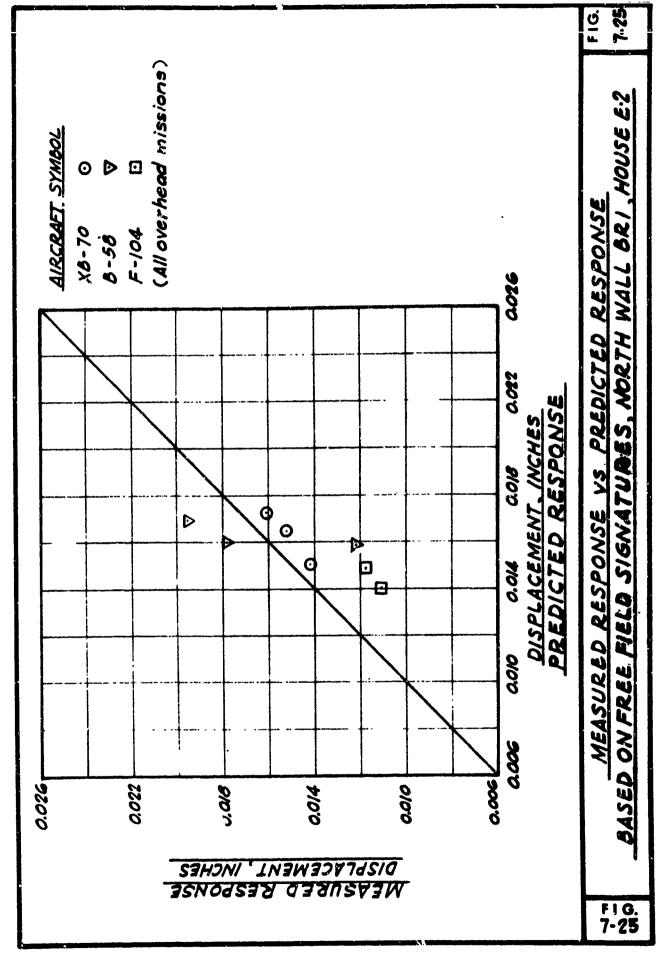
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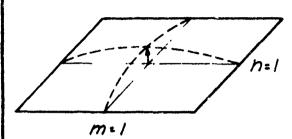


F [G. 7-26 CHANNEL 207 805 806 INSTIGMENT 56-3 ML-5 ML-6 INSTRUMENT LOCATION WINDOW - HOUSE 104.5° \$6.3 GARAGE FLIGHT PATH (245° MAG.) FIG. 7-26

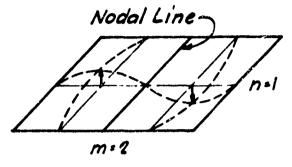
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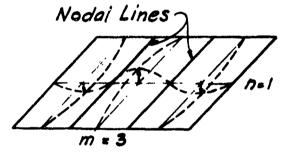
## NON- SYMMETRICAL MODES



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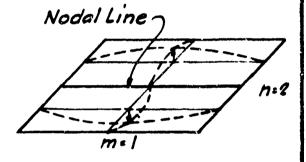


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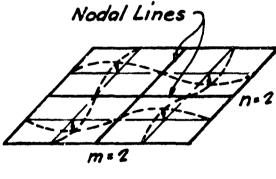


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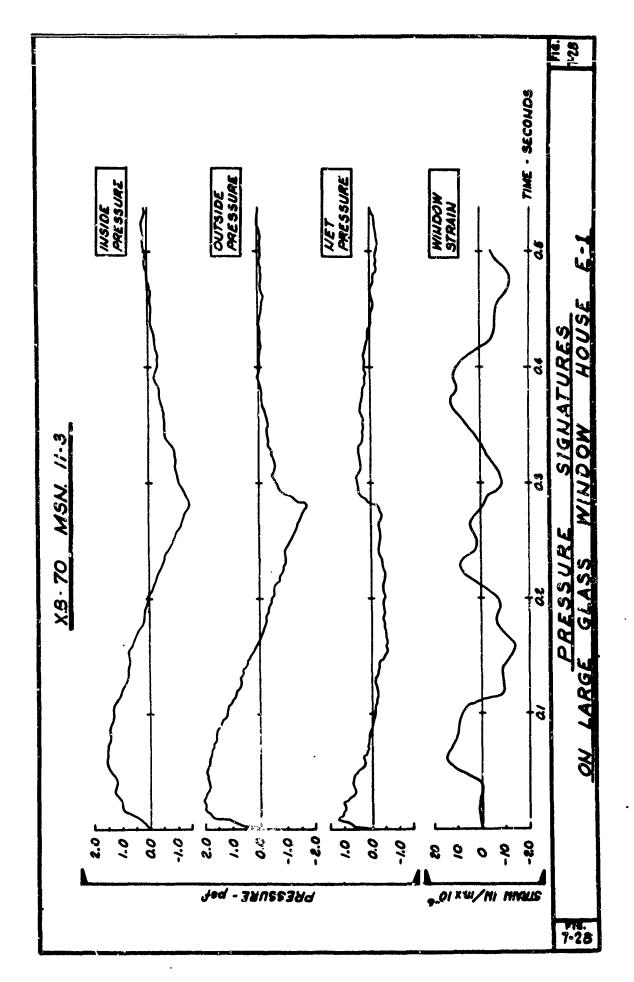
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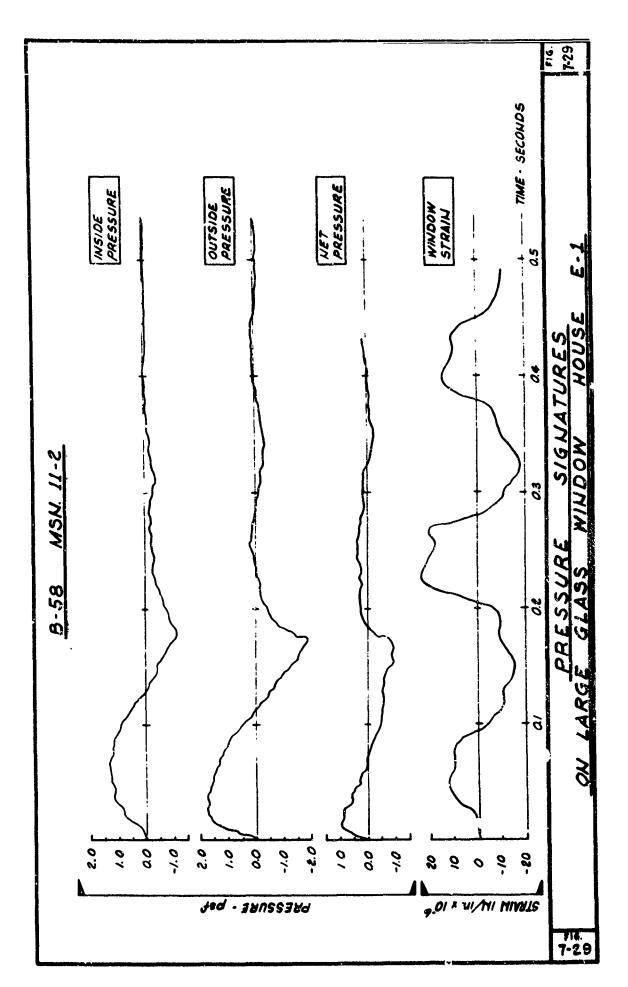


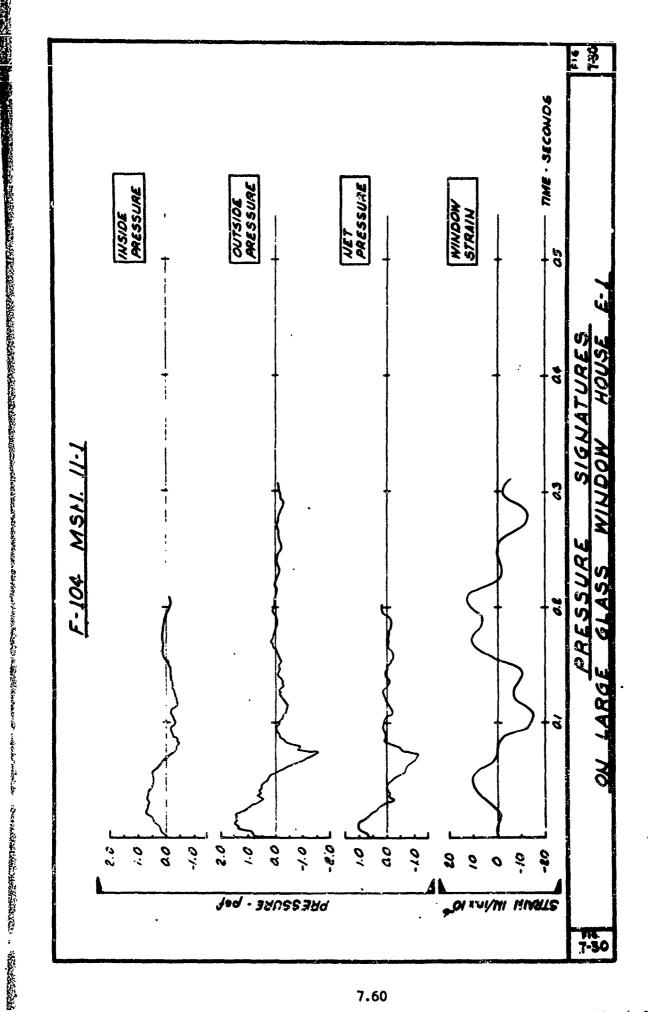
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NATURAL PERIODS AND CORRESPONDING MODE SHAPES FOR E-I WINDOW FIG.

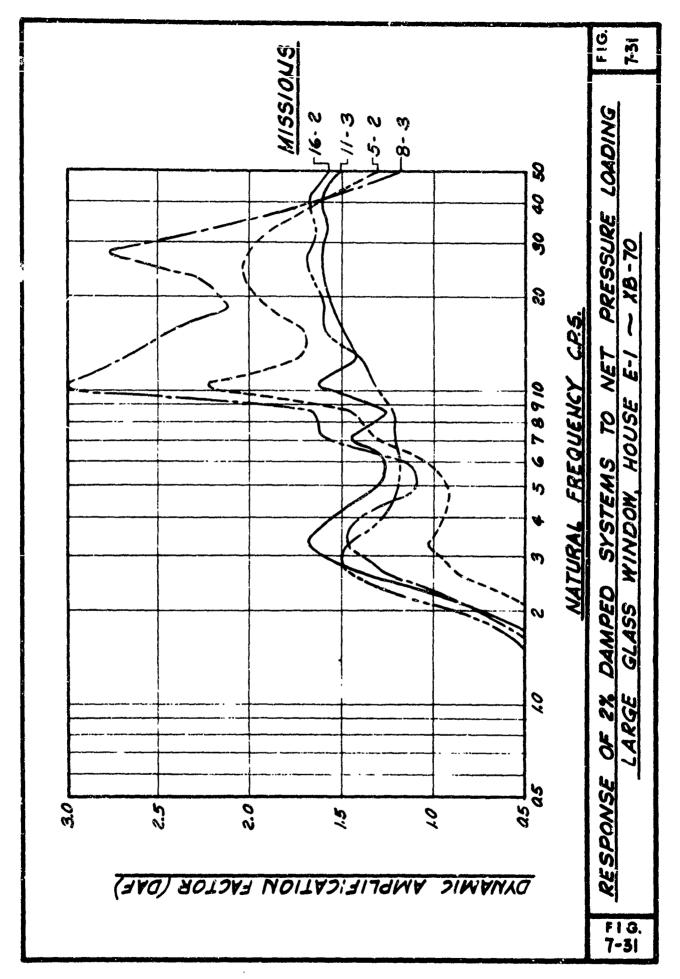
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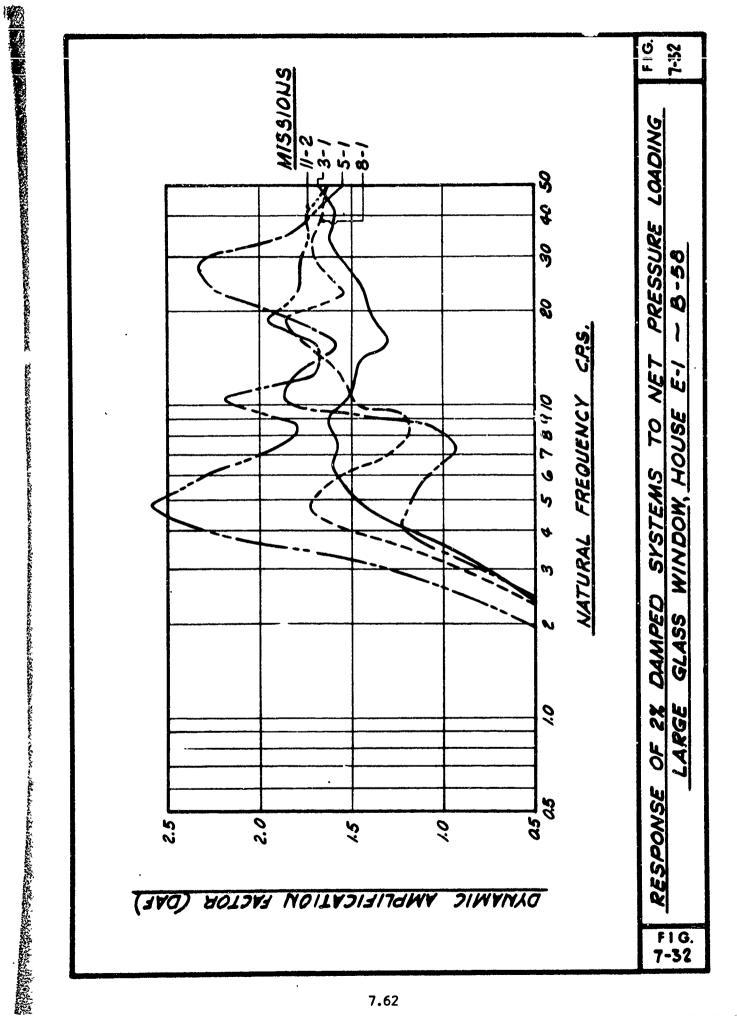






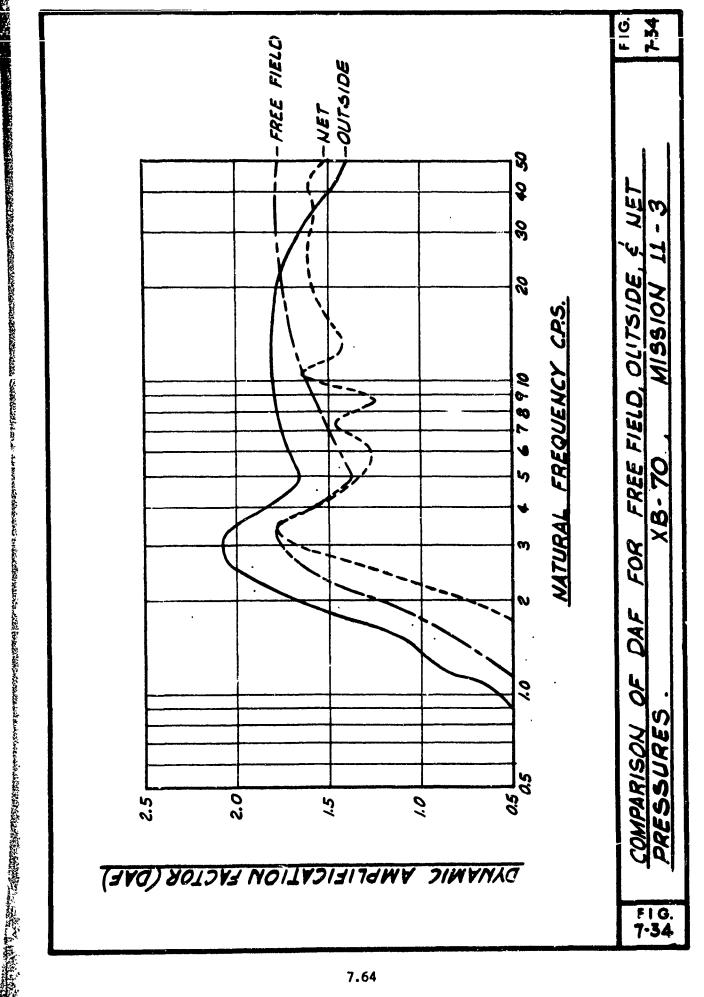
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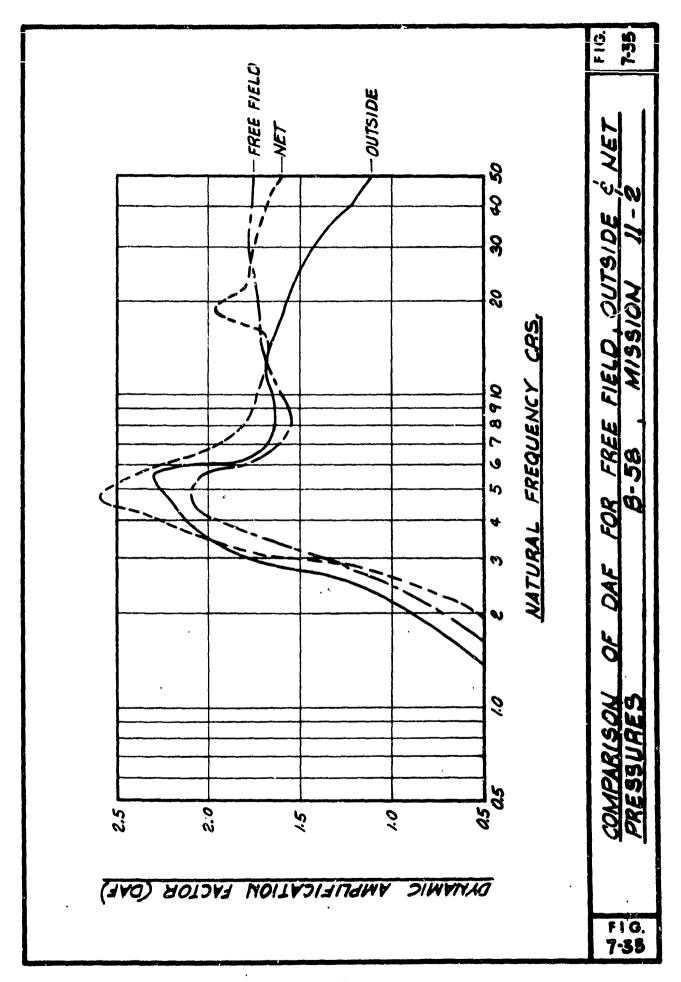




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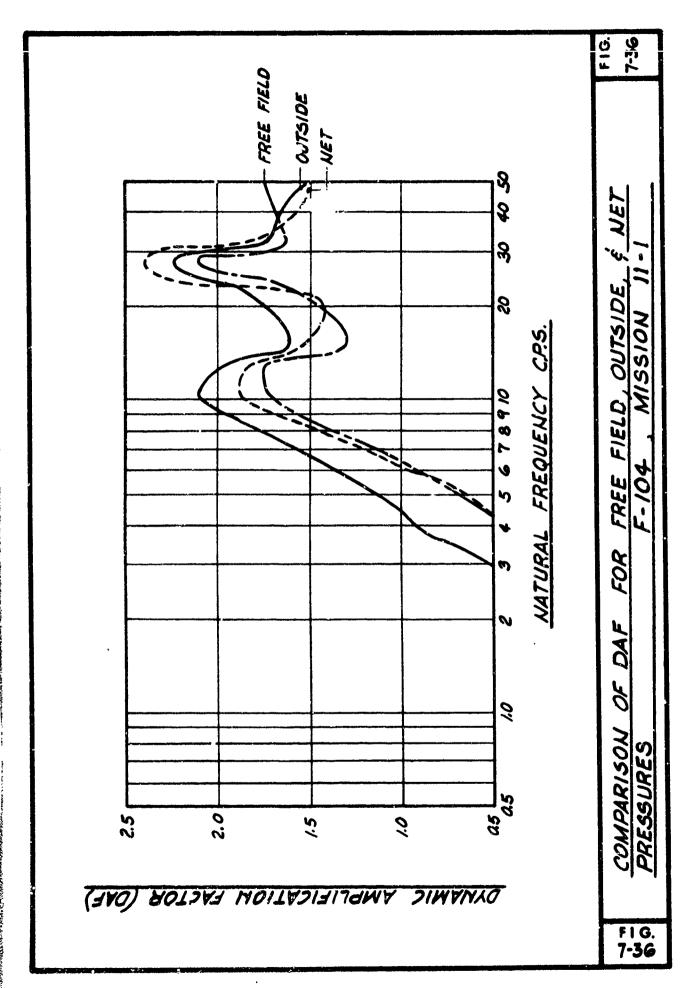
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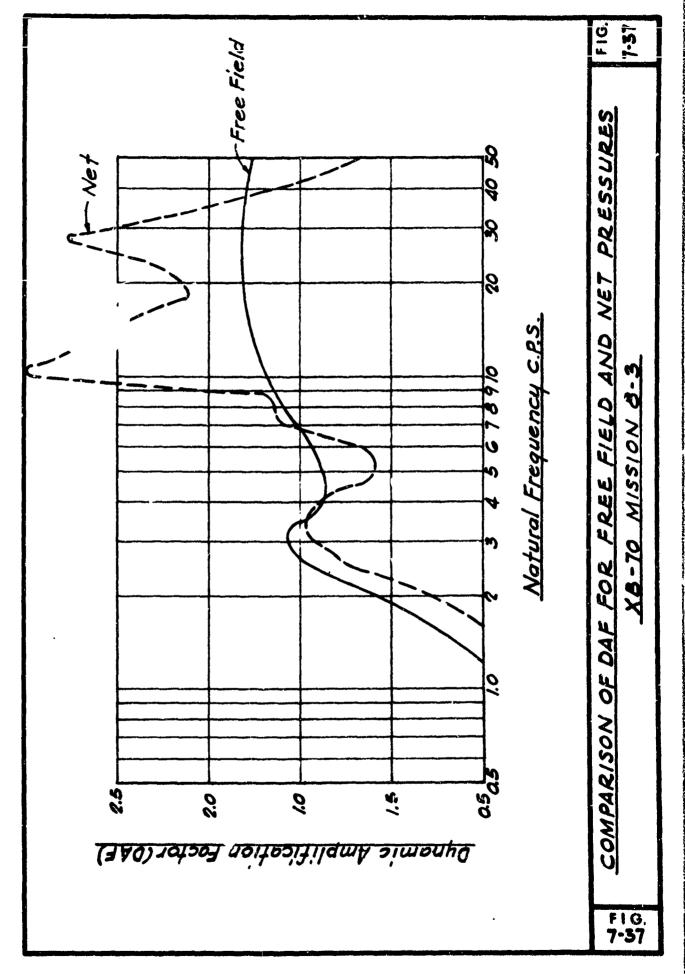


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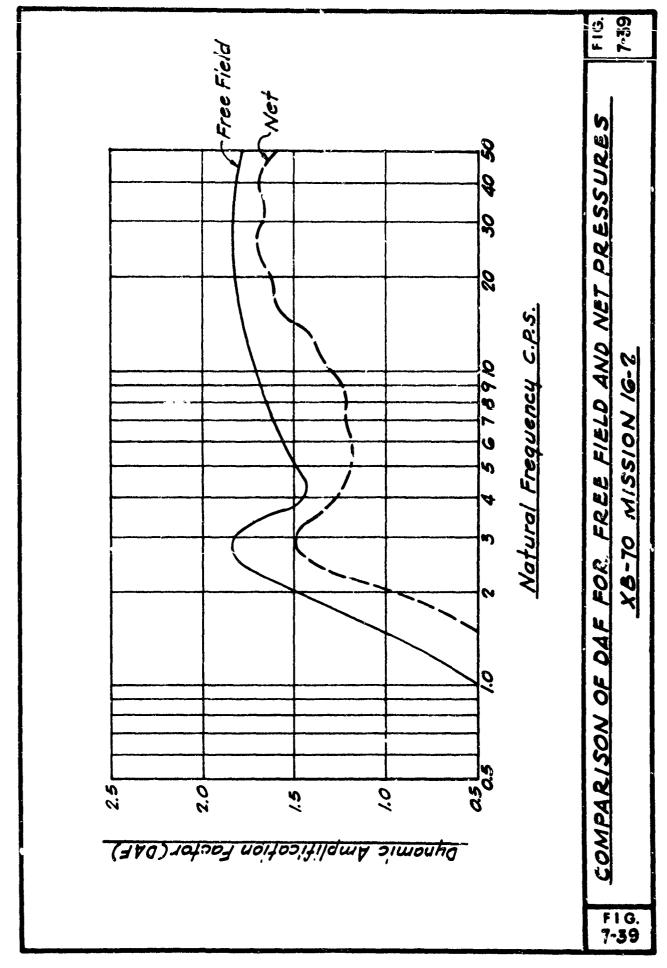
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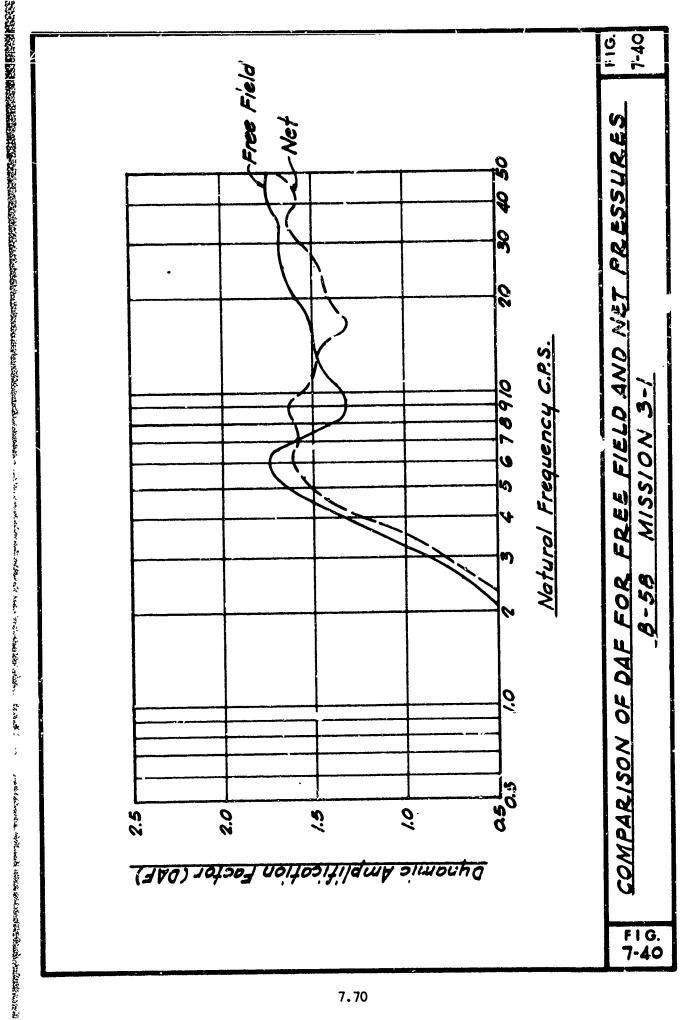
FIG. 7-38

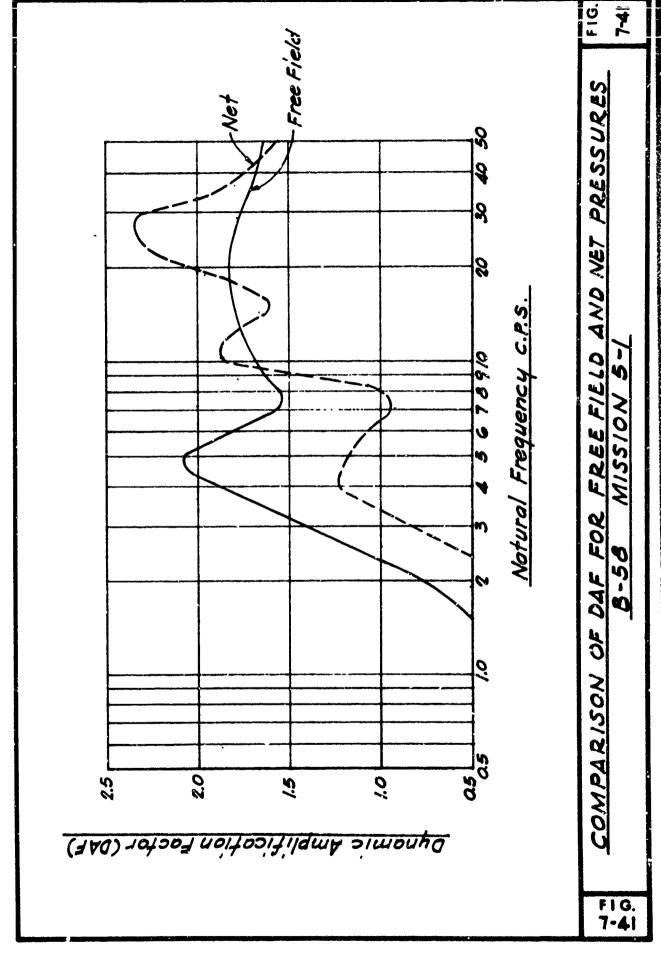
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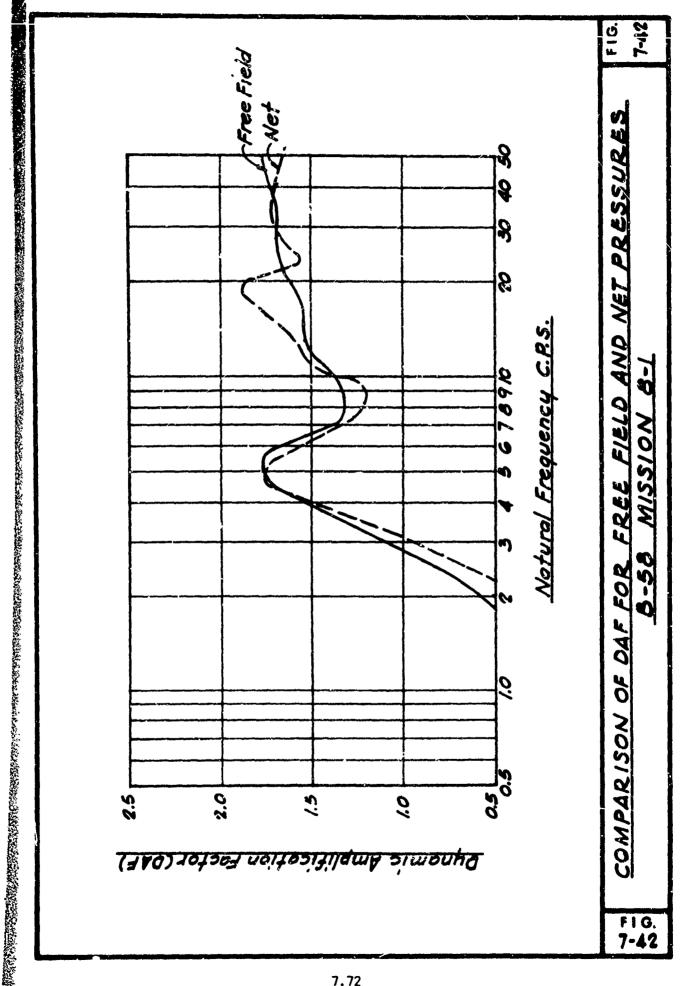
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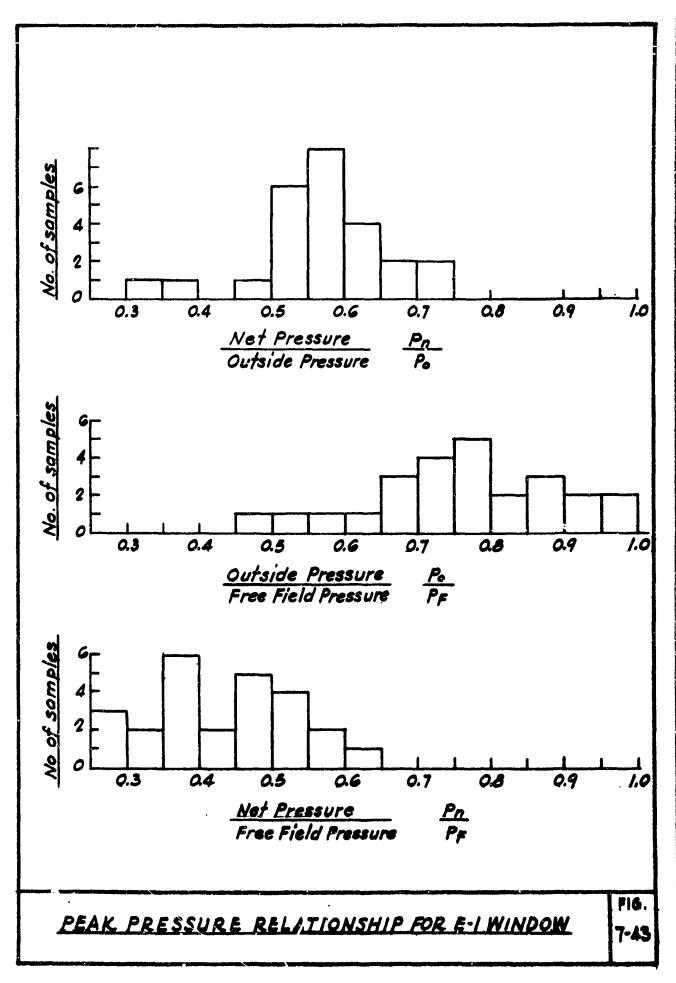
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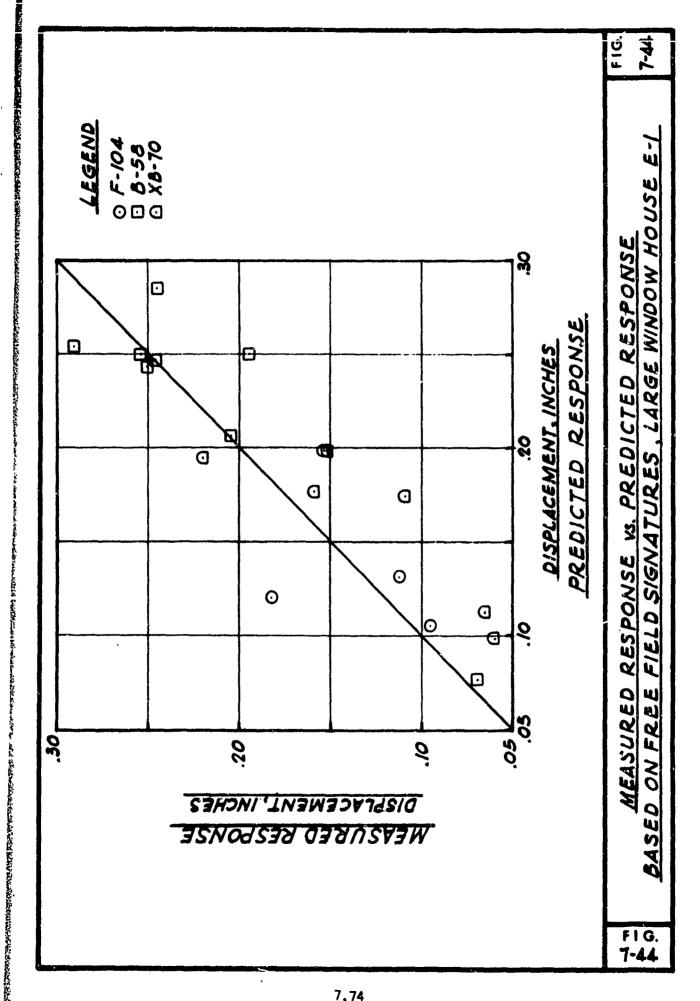


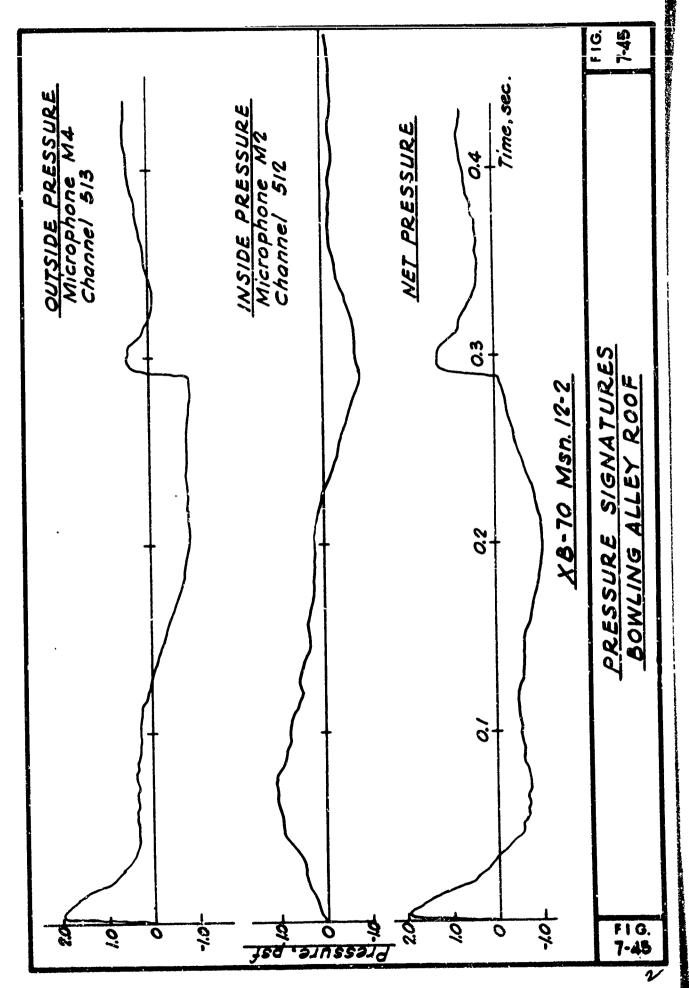


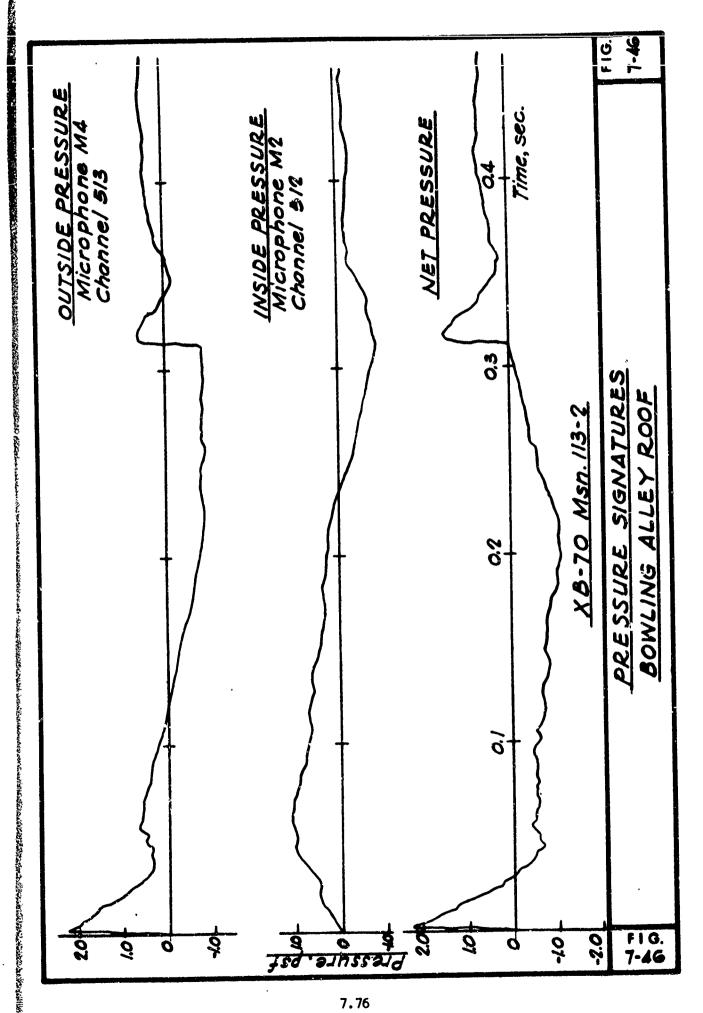


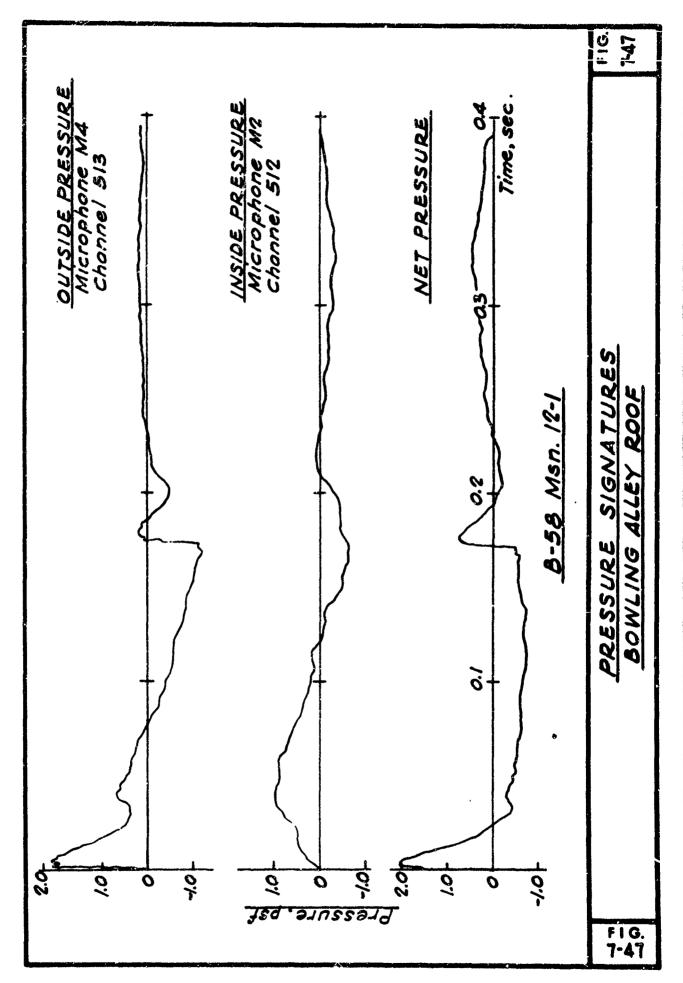


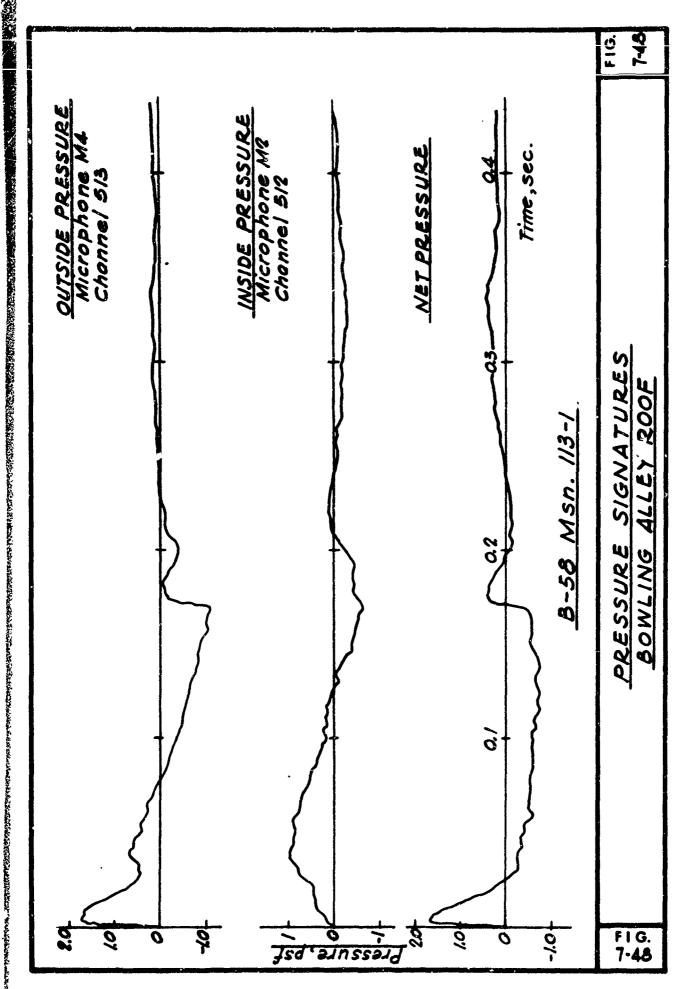


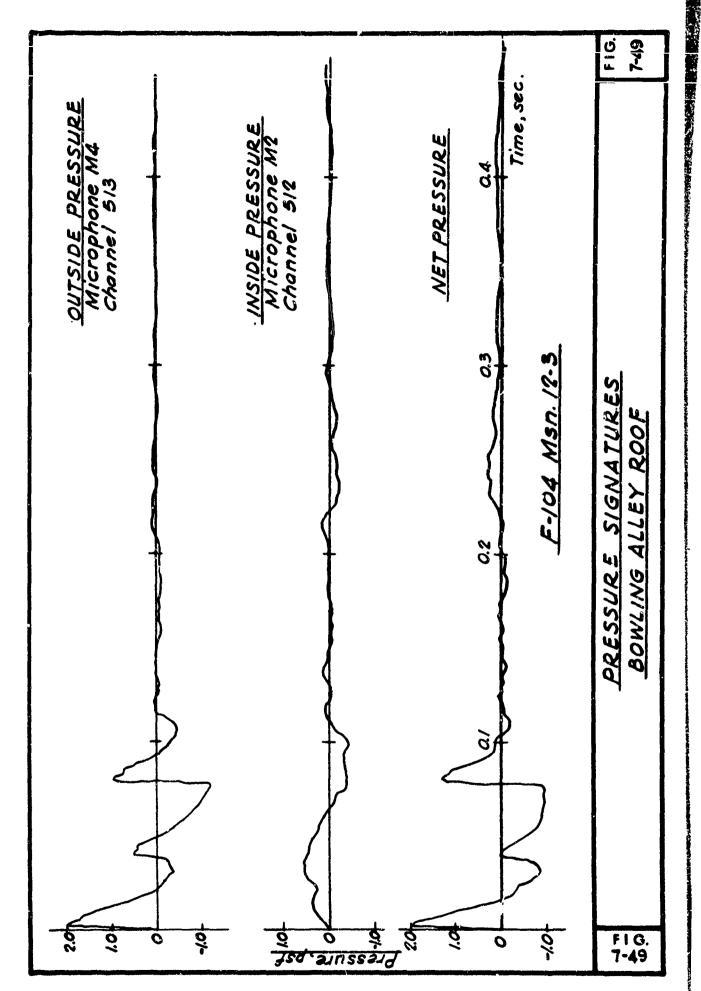


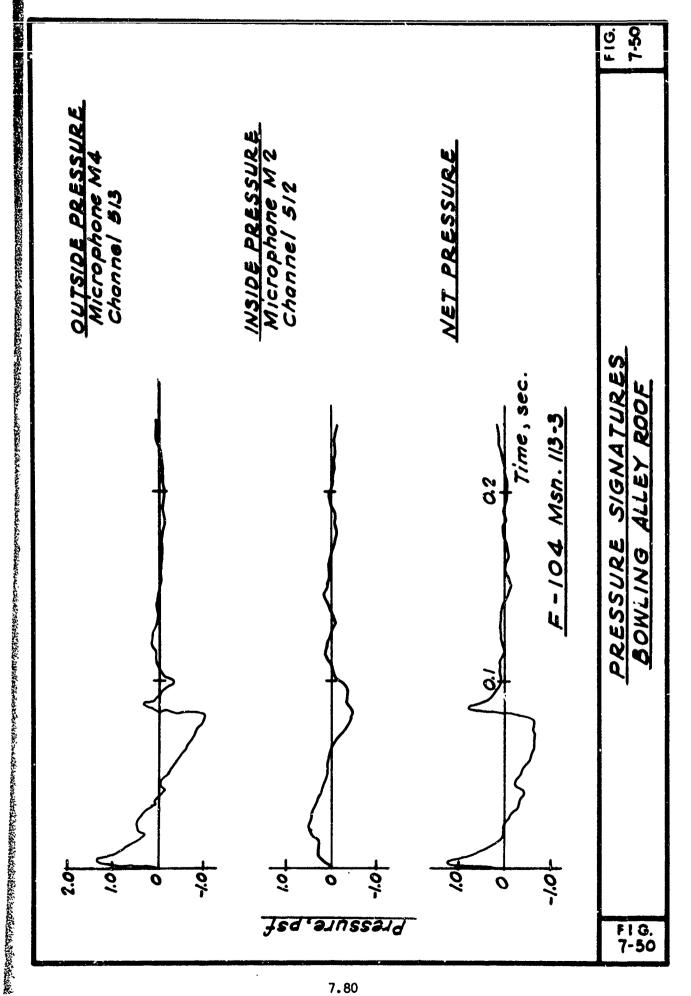


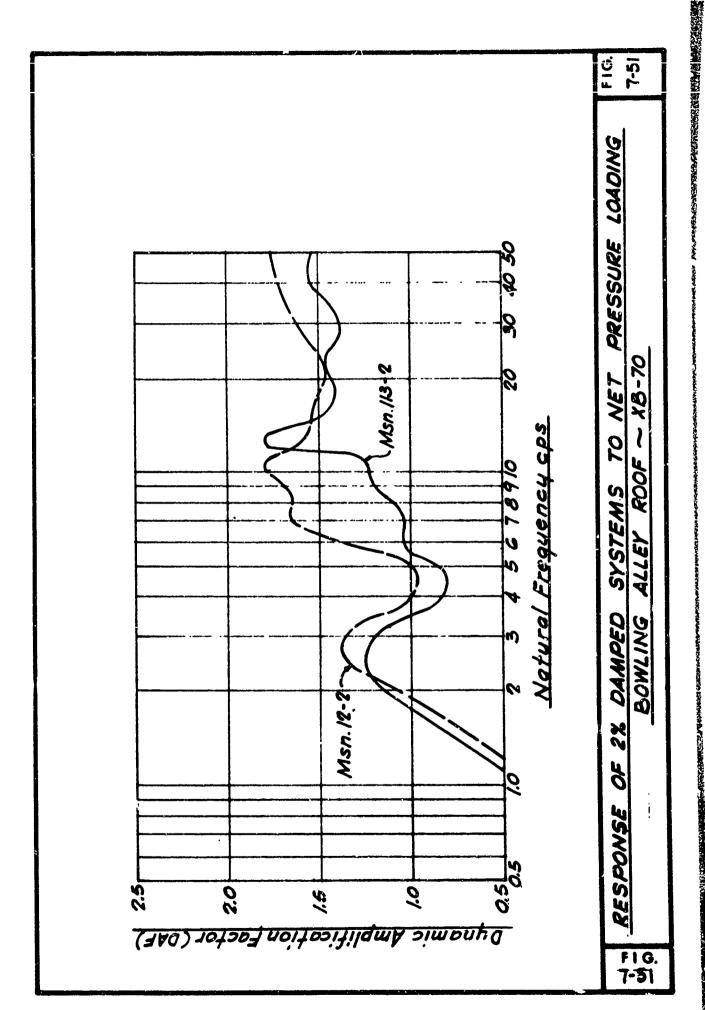


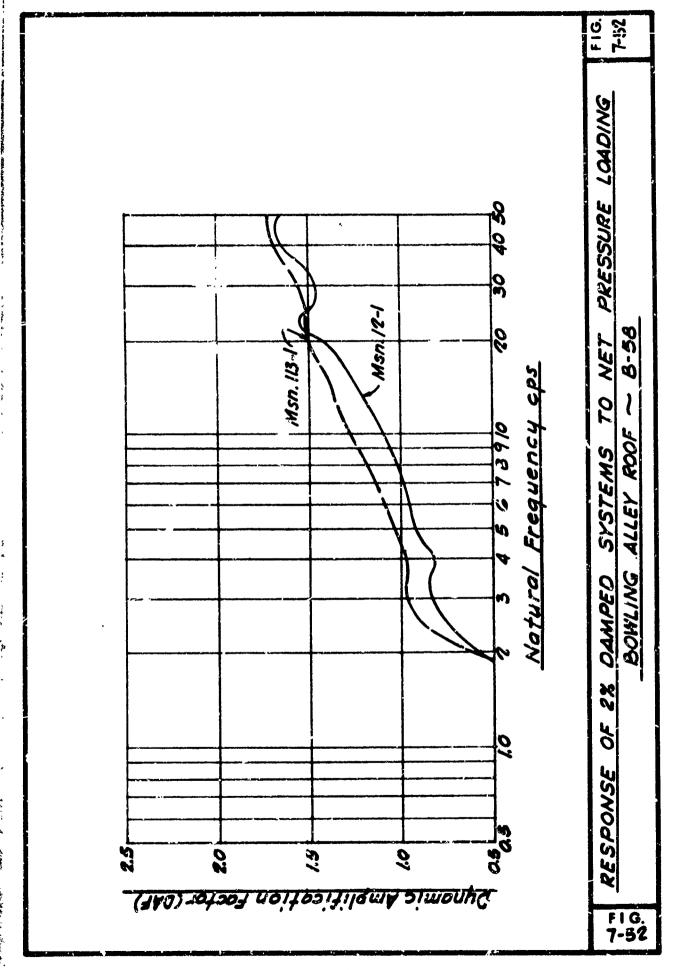




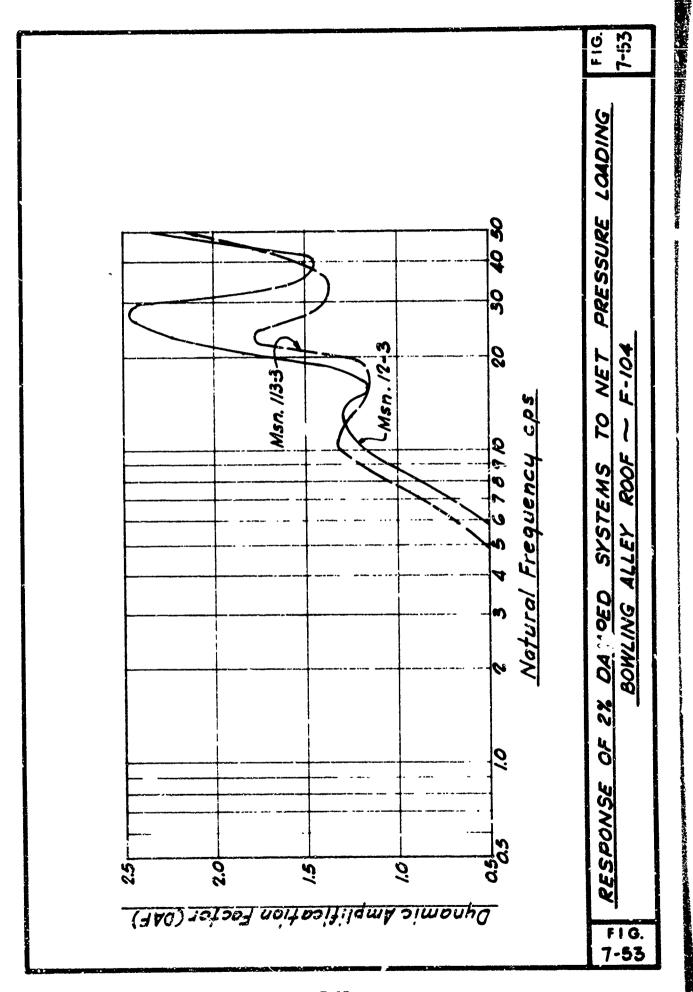








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## VIII. ANALYSIS OF STRUCTURAL RESPONSE DATA - RACKING RESPONSE

The previous chapter discussed the analysis of plate response data. Plate response was the lateral deformation of individual structure elements and was primarily of a bending mode. Racking response, the deformation of the structure as a whole and primarily of a shearing mode, is discussed in this chapter. The analysis of the racking response data was divided into two sections: A) Test Houses E-I and E-2, and B) Bowling Alley E-3.

## A. TEST HOUSES E-I AND E-2

This section presents the results of the analysis of the racking response of Test Houses E-I and E-2. Predicted response based on free field peak overpressures and DAF computed from free field signatures was compared with measured response for the north-south and east-west racking motions of E-I. The reasons and justification for using free field data have been discussed in Chapter II. The resking response of House E-I was analysed in detail and the racking response of E-2 was investigated in lesser detail and compared with E-I. The aircraft flight track was oriented such that it was possible to study racking response due to aircraft vectors that were nearly head-on (east-west racking) and side-on (north-south racking). In addition, the results of the Phase II tests were compared with the results from the Phase I tests.

#### INSTRUMENTATION

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Accelerometers were installed at the northeast corners of Test Houses E-I and E-2 to determine the racking response. These accelerometers were installed at the roof lines of Houses E-I and E-2 and at the second story floor line of House E-2. The locations of these instruments are shown in Figure 8-I. In addition, pressure microphones were installed in and around the two houses so that the actual and net pressure loading on the houses could be determined. The locations of the pressure microphones are shown in Appendix 8.

Note: All figures and tables are placed at the end of this chapter. For the altitudes, Mach number, offset, etc., of aircraft flight missions used in this chapter, see Tables 5-1, 5-2, and 5-3.

## TEST RESULTS - MEASURED DISPLACEMENTS

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Peak racking accelerations were of !ow magnitude (on the order of 0.1g). Table 8-1 lists peak accelerations at the roof lines of the northeast corners of Houses E-1 and E-2 for typical overhead missions of XB-70, B-58 and F-104 aircraft. These peak accelerations were of the same order of magnitude as those obtained in the Phase I tests <sup>19,20</sup>. Figures 8-2, 8-3 and 8-4 show acceleration-time records for east-west racking of the northeast corner of House E-1 for typical overhead XB-70, B-58 and F-104 missions.

The analog magnetic tape recordings of the accelerometer data were converted to digital form as discussed in Chapter IV. The digitized records were then numerically integrated twice to obtain displacements. Peak displacements obtained from the numerical integration process for typical overhead XB-70, B-58 and F-104 missions are listed in Table 8-2. The displacement-time records shown in Figures 8-5, 8-6 and 8-7 were typical for east-west racking of House E-1 for overhead XB-70/B-58/F-104 missions during Phase II. North-south racking displacements for the roof line of E-2 for XB-70, B-58 and F-104 missions during Phase I are shown in Figure 8-8. Note that these displacements were of the same order of magnitude (less than 0.005 in.) as the Phase II data. North-south and east-west displacements for the roof line of House E-2 were combined and are shown in Figure 8-9. In this figure, one half cycle (inside peak to outside peak) represents a time interval of about 0.07 seconds.

A comparison of the racking displacements of E-I and E-2 listed in Table 8-2 and the racking displacements obtained during the White Sands tests indicated that Phase II and White Sands racking displacements were of similar magnitude for structures of similar construction and for similar overpressures.

Note that the racking displacements due to F-104 and B-58 missions were generally larger than those due to the XB-70 and that the displacements due to F-104 missions were usually larger than those caused by B-58 missions, Table 8-2. Several factors caused this trend in response: signature duration, Mach number, and building length, all of which affect the net pressure signatures on the houses. Pressure signatures for the east wall and west wall and net pressure on the structure for typical east to west overhead XB-70/B-58/F-104 missions are shown in Figures 8-10, 8-11, and 8-12. For the missions shown, the time lags between the start of the boom on the east wall and on the west wall (building length divided by the speed of the aircraft) were 0.027, 0.031 and 0.033 seconds for the XB-70, B-58 and F-104 respectively.

Examination of the net pressure pulses indicated why the response was greater for the B-58 and F-104 missions. For these two aircraft, the net pressure signature was a distorted N-wave. However, the XB-70 net pressure signature was greatly changed from an N-wave and was reduced to two very short pulses separated by approximately 0.24 sec. This net pressure signature produced smaller displacements, as would be expected.

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It is therefore reasonable to expect that the future SST, with a faster speed and pressure signature of longer duration, will produce racking displacements of a typical house that will be of similar order of magnitude, or probably smaller, than those produced by the XB-70. However, the magnitude of displacements was very small for all aircraft for the low overpressure levels encountered and were far below the minimum required to cause damage.

#### PREDICTED DISPLACEMENTS

Predicted racking displacements were computed for the north-south and east-west directions of Test House E-I using methods explained in Append. A and Equation (A-3):

$$\Delta = \frac{P}{K} \times DAF$$

where  $\Lambda = \text{Peak}$ 

 $\Delta$  = Peak dynamic racking displacement

P = Total racking load on the house

K = Structure stiffness

DAF = Dynamic amplification factor as determined from free field signatures

The following is a brief summary of methods used to determine P. K. and DAF.

The total racking load, P, was taken as the average free field peak over-pressure from the five cruciform microphones times the effective building surface area. The effective surface area was taken as the area of the vertical surfaces (walls) normal to the direction of racking. The effective areas for racking in the N-S and E-W directions were 159 sq. ft. and 103 sq. ft. respectively. The total load, P, for a free field overpressure of one psf was then  $P_{ns} = 159 \text{ lb}$  and  $P_{ew} = 103 \text{ lb}$ .

For the small displacements involved, the stiffness K was difficult to accurately calculate utilizing normal values of material properties. Therefore, the stiffness was calculated by approximate methods. The following is an out-line of two different methods used to determine the stiffness of House E-I:

The first method used, which depended in part on recorded test data, was to determine the stiffness, K, from  $^8$ :

$$T = 2\pi \sqrt{\frac{W}{Kg}}$$
 (8-1)

which can be written as:

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$$K = \frac{4\pi^2 W}{T^2 a} \tag{8-2}$$

where W = Weight of roof, calling and upper one third of all walls = 18,900 lb., and T = Natural period as determined from integrated accelerometer records.

$$(T_{ns} = 0.062 \text{ sec.}, T_{ew} = 0.083 \text{ sec.})$$

Substituting these values:

$$K_{ns} = 5.0 \times 10^5$$
 lb/in  
 $K_{ns} = 2.8 \times 10^5$  lb/in

The second method of determining the racking stiffness of House E-I was the conventional approach which utilized normal values of material properties. The equation for the deformation of a shear wall due to a shearing load applied at the top of the wall is 24:

$$\Delta = \frac{1.2 \text{ hV}}{\text{AG}} \tag{8-3}$$

where

 $\Delta$  = Displacement at top of wall

h = Height of wall = 7.5 ft

V = Shear force applied at top of wall

A = Shear area = wall length times thickness

G = Shearing modulus = 0.4 E = 0.4 (44,200 lb/in
$$^2$$
)  
= 17,600 lb/in $^2$ . (Ref.2)

This equation was rewritten as:

$$V = \frac{AG\Delta}{1.2h} \tag{8-4}$$

For  $\Delta = 1$ , V = K = Stiffness, and

$$K = \frac{AG}{1.2h} \tag{8-5}$$

For Test House E-I, it was determined that there were 229 and 178 lineal feet (gross length minus width of openings) in the N-S and E-W directions respectively, of wood stud wall with 0.5" thick gypsum board on one face. There-

fore 
$$A_{ns} = (229 \text{ ft}) (0.5 \text{ in}) (12 \text{ in/ft}) - 1,374 \text{ in}^2$$

$$A_{ew} = (178 \text{ ft}) (0.5 \text{ in}) (12 \text{ in/f1}) = 1,068 \text{ in}^2$$

Substituting the actual values of A, G, and h, the N-S and E-W racking stiffnesses as determined by this second approach were:

$$K_{ns} = 2.23 \times 10^5 \text{ lb/in}$$
  
 $K_{ns} = 1.72 \times 10^5 \text{ lb/in}$ 

Note that these values were lower than those computed by the first approach. Similar results (where computed displacements, which are inversely proportional to stiffnesses, were greater than those experimentally determined for small deformations) have been observed by others  $^{21}$ . For this reason, and the fact that the results from the first method depended in part on recorded test data (natural period) and that the weight, W, of the structure could be accurately estimated, the results from the first method were used as being more accurate. Therefore, the values determined by the first method ( $K_{ns} = 5.0 \times 10^5$  lb/in and  $K_{ew} = 2.8 \times 10^5$  lb/in) were used in the computation of the predicted displacements.

In order to facilitate the computation of predicted displacements, the load, P, and stiffness, K, were combined to determine the unit racking displacement (P/K) for the northeast corner of House E-I for a sonic boom overpressure of one psf. The unit racking displacements were:

$$\Delta_{ns} = P_{ns}/K_{ns} = 0.00032 \text{ in/psf}$$
 $A_{ns} = P_{ns}/K_{ns} = 0.00036 \text{ in/psf}$ 

Predicted displacements were determined by multiplying these unit racking displacements by the average free field peak overpressure and corresponding DAF from spectra determined from the free field signatures. Predicted racking displacements for House E-I are summarized in Table 8-3 for typical overhead XB-70, B-58, and F-104 missions.

## COMPARISON OF PREDICTED AND MEASURED DISPLACEMENTS

The predicted displacements were plotted versus measured displacements in Figure 8-13. In this figure the 45° diagonal line indicated a one to one ratio of predicted to measured response. The predicted displacements were in good agreement with the measured displacements even though there was some scatter in the measured displacements. For both the east-west and north-south racking of House E-1, it was determined by the use of a statistical t-test that the average

ratio of predicted displacements (using free field data) to the measured displacements was equal to 1.0 at the 95 percent confidence level. The degree of precision of results and the probability that the results have rhis degree of precision were summarized in the table on page 5.5.

It was concluded from the comparison of the predicted and measured racking response that the free field peak overpressures and DAF spectra calculated from the free field signatures provide a good method for approximating the actual loading conditions and therefore racking response can be predicted using free field data. This conclusion applies to both "head-on" (aircraft flight track perpendicular to wall surface) and "side-on" (aircraft flight track parallel to wall surface) vectors, since for all practical purposes, the east-west racking is the result of a head-on vector and the north-south racking of a side-on vector.

#### SUMMARY OF FINDINGS

This section summarized the analysis of the racking response of test houses E-I and E-2. Predicted response using free field peak overpressures and DAF spectra computed from free field signatures was compared with measured racking response in the north-south and east-west directions of E-I. The aircraft flight track was oriented such that it was possible to study racking response due to aircraft vectors that were nearly head-on (east-west racking) and side-on (north-south racking). The results of the Phase II tests were also compared with those from Phase I <sup>19</sup>, 20 and White Sands <sup>2</sup> tests. The following findings resulted from these analyses:

- 1. Racking displacements at the roof line of the northeast corners of test houses E-I and E-2 were extremely small (less than 0.0018'' for E-I and less than 0.005'' for E-?) for sonic booms on the order of 2 psf overpressure.
- 2. Racking displacements of E-1 and E-2 recorded during Phases 1 and 11 were of similar magnitudes for similar overpressures.
- 3. The racking displacements of E-I and E-2 recorded during Phase II were of magnitude similar to displacements obtained at White Sands<sup>2</sup> for structures of similar construction and for similar overpressures.
- 4. Racking displacements predicted from free field peak overpressures and DAF spectra calculated from free field pressure signatures were in good agreement with measured displacements. For both the east-west and north-south

racking of house E-I, the average ratio of the predicted to measured displacement was equal to 1.0 at the 95 percent confidence level for comparable XB-70, B-58, and F-104 missions. These findings applied to both head-on and side-on vectors.

- 5. Racking response could be adequately predicted by using peak overpressure and DAF spectra calculated from free field signatures.
- 6. The future SST, for peak overpressures of about 2 psf, should produce racking displacements of typical houses that will be of similar magnitude, or possibly smaller, than those caused by the XB-70 missions. These racking displacements should be negligible and far below those required for damage.

The implications of these findings as related to structure element damage are discussed in Chapter IX. The following section discusses the racking response of the Bowling Alley.

### B. BOWLING ALLEY E-3

This section briefly summarizes the analysis of the racking response of the Bowling Alley E-3.

### INSTRUMENTATION

In order to study the racking response of the Bowling Alley, accelerometers were installed near the rops of the steel columns as shown in Appendix B.

#### TEST RESULTS - MEASURED DISPLACEMENTS

The analog magnetic tape recordings of the accelerometer data were converted to digital form as discussed in Chapter IV and then numerically integrated twice to obtain displacements. Peak displacements for typical missions of X8-70, B-58, and F-104 aircraft are listed in Table 8-4. The displacements were of extremely low magnitude (all less than 0.0041 inches). As free field signatures were not measured near the Bowling Aliey, a comparison could not be made of displacements predicted from free field versus measured displacements. The magnitude of the racking displacements were so small (le s than the maximum measured for E-2) that there was no damage.

TABLE 8-1

RACKING ACCELERATIONS AT ROOF LINE

NORTHEAST CORNER OF TEST STRUCTURES E-1 AND E-2

House	Racking Direction	Aircraft	Mission	Average Free Field <u>Pressure</u>	Feak Accel	eration <sup>()</sup>
				psf	ft/sec <sup>2</sup>	g <b>'</b> s
E-1	E-W	XB-70	13-2	2.00	~2.92	-0.091
			15-1	2.18	-2.51	-0.078
			15-2	2.29	-3.74	-0.116
		B-58	13-1	2,21	-3.80	-0.118
			15-2	2.34	+3.66	+0.114
			16-1	2.25	-3.82	-0.118
		F-104	13-3	2.01	-3.81	-0.118
			15-3	2.31	+4.74	-0.146
			16-3	2.02	-3.94	-0.122
E-1	N~S	×8-70	13-2	2.00	-4.26	-0.132
			15-1	2.18	-3.24	-0.100
			16-2	2.29	-4.21	-0.131
		B~58	13-1	2.21	-5.05	-0.157
			15-2	2.34	-4.02	-0.125
			16-1	2.25	-3.61	-0.!12
		F-104	13-3	2.01	-6.01	-0.186
			15-3	2.31	+5.02	+0.156
			16-3	2.02	-3.17	-0.095
E-2	E-W	XB-70	13-2	2.00	-2.71	-0.084
		B-58	13-1	2.21	-2.56	-0.079
		F-104	13-3	2.01	-8.67	-0.268
E-2	N-S	XB-70	13-2	2.00	-3.65	-0.113
		B-58	13-1	2.21	+2.91	+0.090
		F-104	13-3	2.01	+3.29	+0.102

Minus sign indicates acceleration toward house.
 Plus sign indicates acceleration away from house.

TABLE 8-2

RACKING DISPLACEMENTS AT ROOF LINE

NORTHEAST CORNER OF TEST STRUCTURES E-1 AND E-2

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House	Racking <u>Direction</u>	Aircraft	Mission	Average Free Field <u>Pressure</u> <u>psf</u>	Racking Displacement inch	Normalized Racking Displacement in/psf
E-1	E-W	XB-70	13-2	2.00	+0.00129	0.00065
- ,	<b>-</b>	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	15-1	2.18	+0.00112	0.00051
			16-2	2.29	+0.00112	0.00049
		B-53	13-1	2.21	-0.00156	0.00071
			15+2	2.34	-0.00120	0.00051
			16-1	2.25	-0.00157	0.00070
		F-104	13-3	2.01	+0.00177	0.00088
			15-3	2.31	+0.00156	0.00068
			16-3	2.02	+0.00168	0.00083
E-1	N-S	XB-70	13-2	2.00	-0.00142	0.00071
			15-1	2.18	-0.00094	0.00043
			16-2	2.29	-0.00077	0.00034
		B-58	13-1	2.21	+0.00148	0.00067
			15-2	2.34	+0.00099	0.00043
			16-1	2.25	+0.00077	0.00034
		F-104	:3-3	2.01	+0.00177	0.00088
			15-3	2.31	+0.00137	0.00059
			16-3	2.02	+0.00162	0.00080
₹-2	E-W	XB-70	13~2	2.00	-0.00309	0.00155
		B-58	13-1	2.21	+0.00421	0.00190
		F-104	!3-3	2.01	+0.00379	0.00188
E-2	N-S	XB-70	13-2	2.00	+0.00310	0.00155
		B-58	13-1	2.2	+0.00298	0.00!35
		F-104	13~3	2.01	-0.00491	0.00245

Minus sign indicates displacement toward house.
 Plus sign indicates displacement away from house.

TABLE 8-3
PREDICTED RACKING DISPLACEMENTS

# HOUSE E-1

Direction	Aircraft	Mission	Measured Displacement ΔM Inches	Free Field <u>Pressure</u> <u>psf</u>	Free Field DAF	Predicted Displacement <u>AP</u> <u>inches</u>	<u>ΔΡ</u> ΔΜ
E-W	XB-70 ·	13-2	0.00129	2.00	1.74	0.00125	0.97
		15-1	0.00112	2.18	1.68	0.00132	1.18
		16-2	0.00112	2.29	1.70	0.00140	1.24
	8-58	13~1	0.00156	2.21	1.54	0.00123	0.79
		15-2	0.00120	2.34	1.55	0.00130	1.08
		16-1	0.00157	2.25	1.54	0.00124	0.79
	F-104	13-3	0.00177	2.01	1.70	0.00123	0.70
		15-3	0.00156	2.31	1.76	0.00130	0.84
N-S	XB-70	13-2	0.00142	2.00	1.77	0.00113	0.80
		15-1	0.00094	2.18	1,70	0.00118	1.26
		16-2	0.00077	2.29	1.72	0.00126	1.64
	B-58	13-1	0.00148	2.21	1.62	0.00114	0.77
		15-2	0.00099	2.34	1.58	0.00118	1.19
		16-1	0.00077	2.25	1.58	0.00114	1.48
	F-104	13-3	0.00177	2.01	1.49	0.00096	0.54
		15.3	0.00137	2.31	1.65	0.00122	0.89

TABLE 8-4

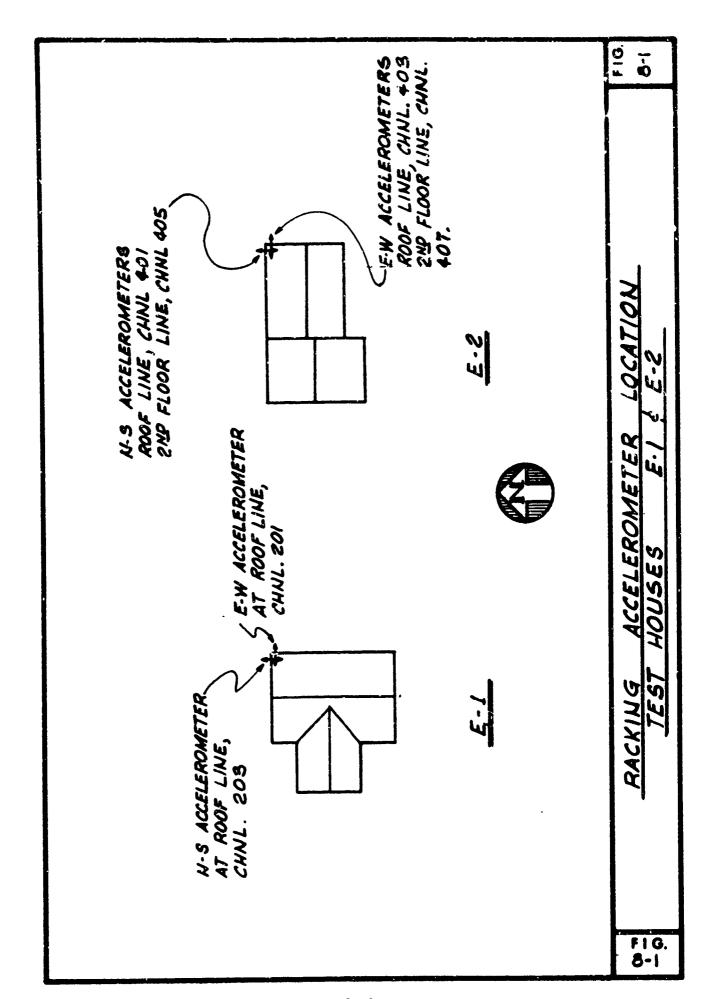
## BOWLING ALLEY RACKING DISPLACEMENTS

# AT TOP OF SOUTHEAST CORNER COLUMN

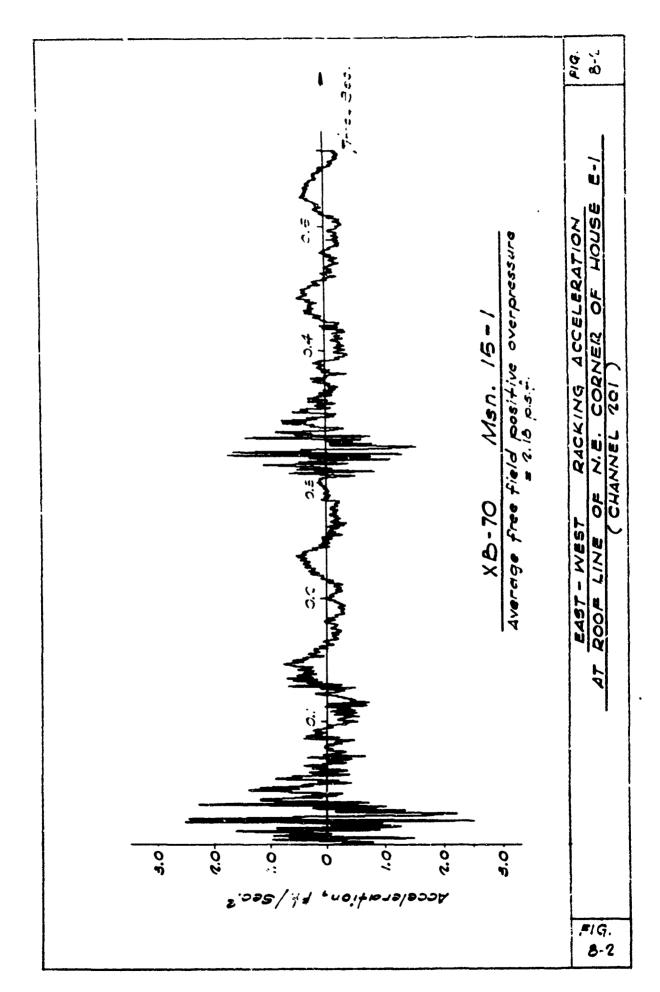
## IN EAST-WEST DIRECTION

Aircraft	Mission	Peak Acceleration 1)	Peak Displacement 1)
		tt/sec <sup>2</sup>	inches
XB-70	12-2	-0.631	-0.00308
	113-2	-0.604	0.00406
B~58	12-1	0.903	-0.00354
	113-1	-0.564	-0.00341
F-104	12-3	0.682	-0.00134
	113-3	-0.412	-0.00087

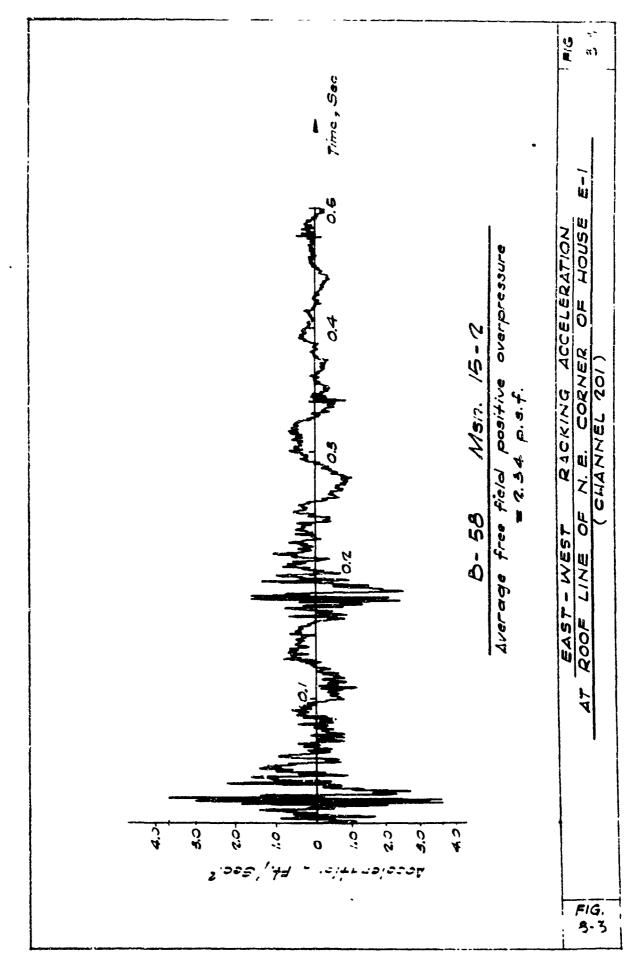
Minus sign indicates acceleration or displacement in eastern direction.



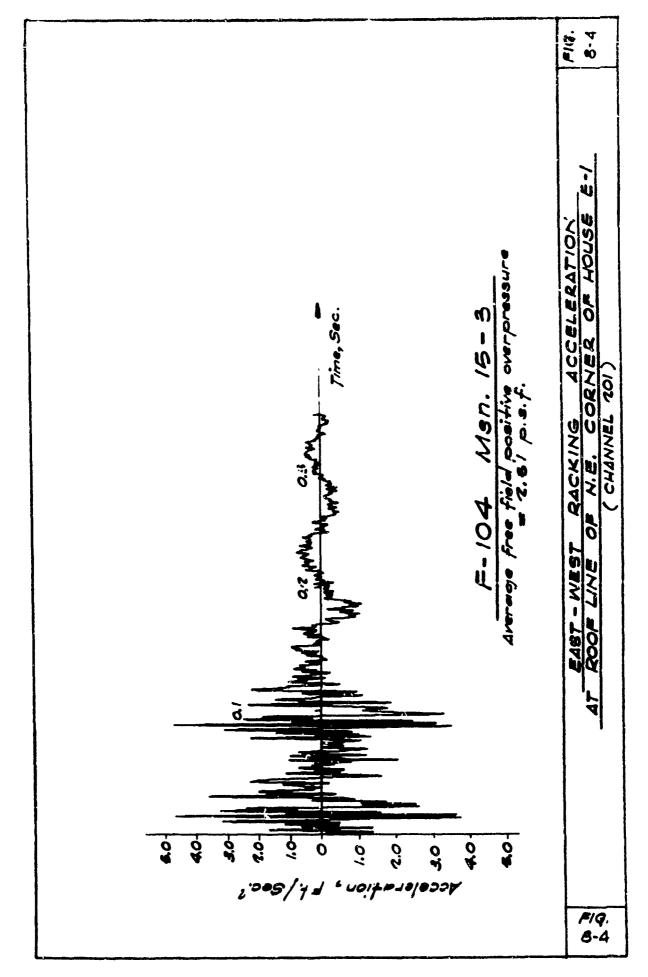
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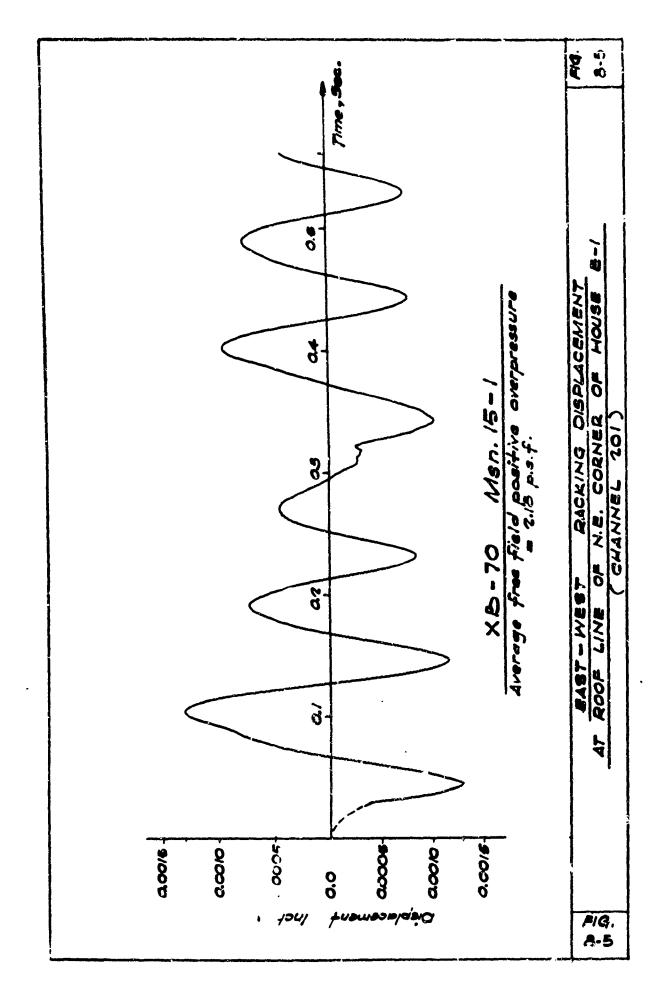
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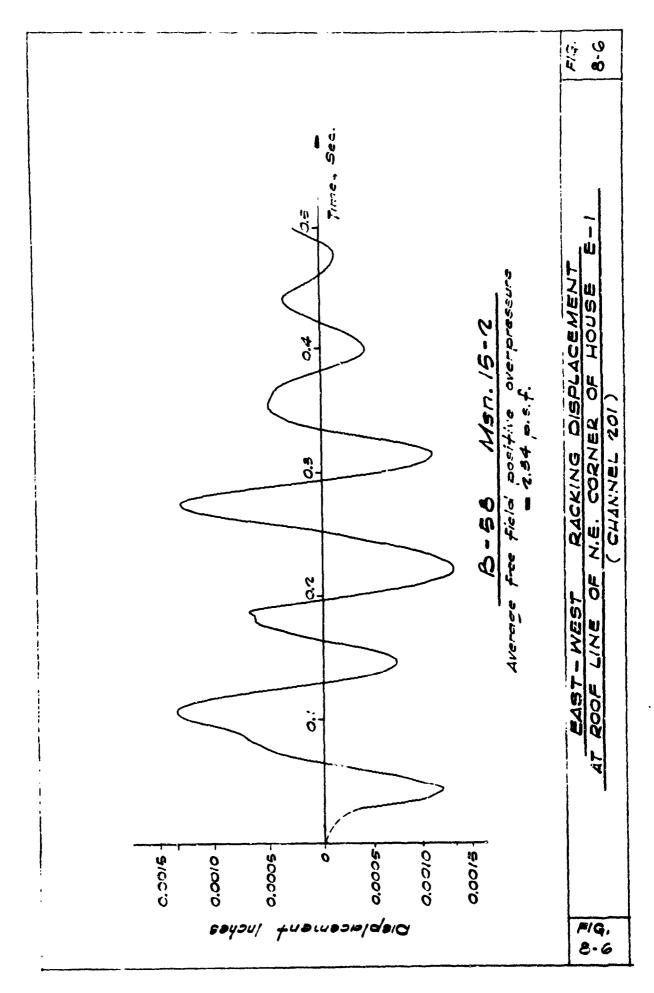


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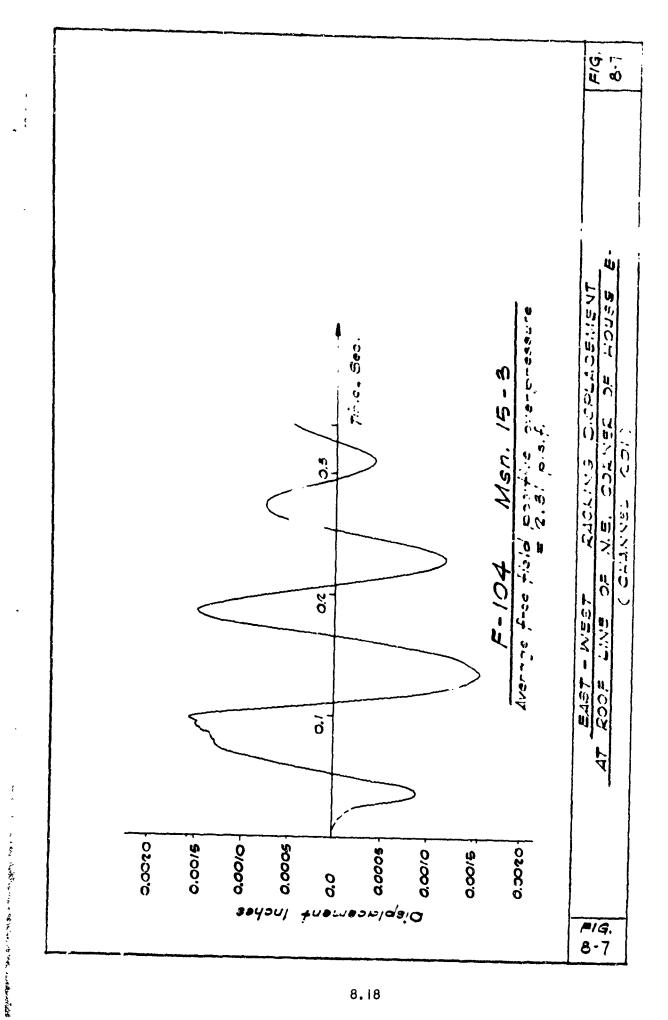


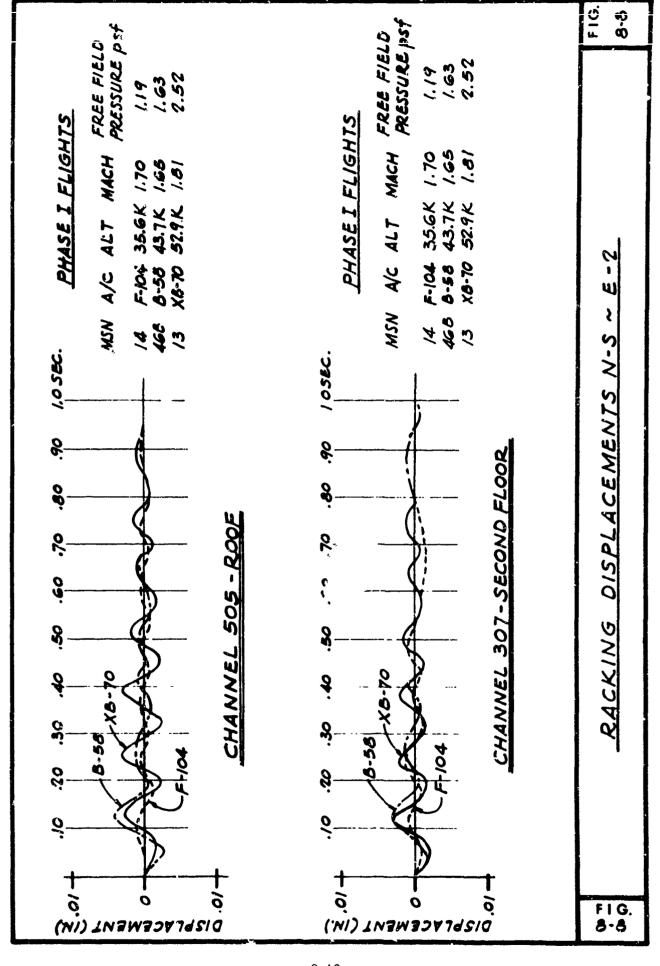
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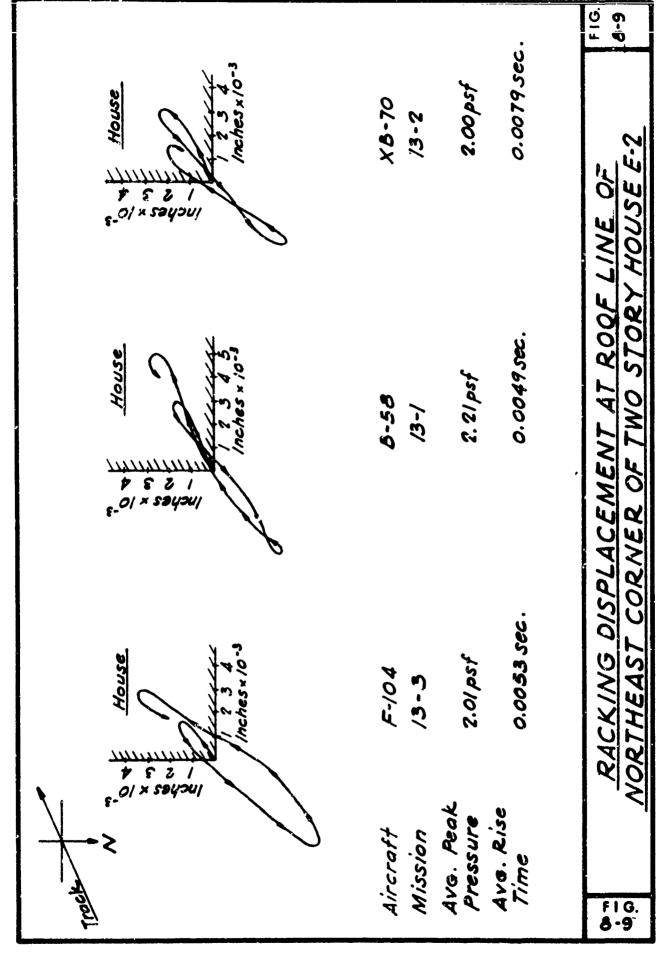
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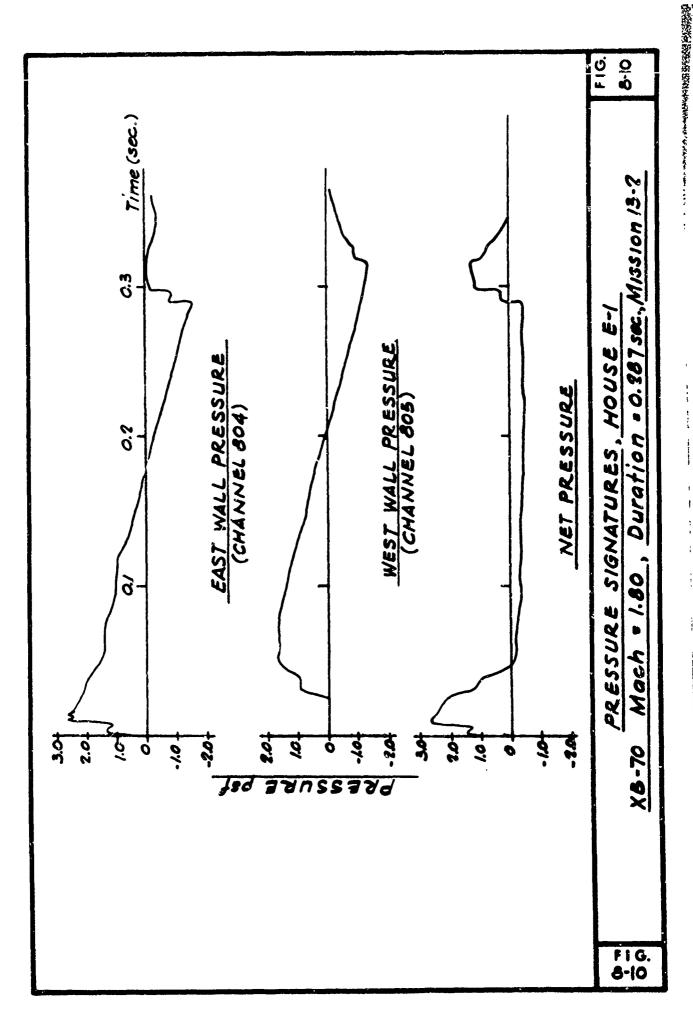
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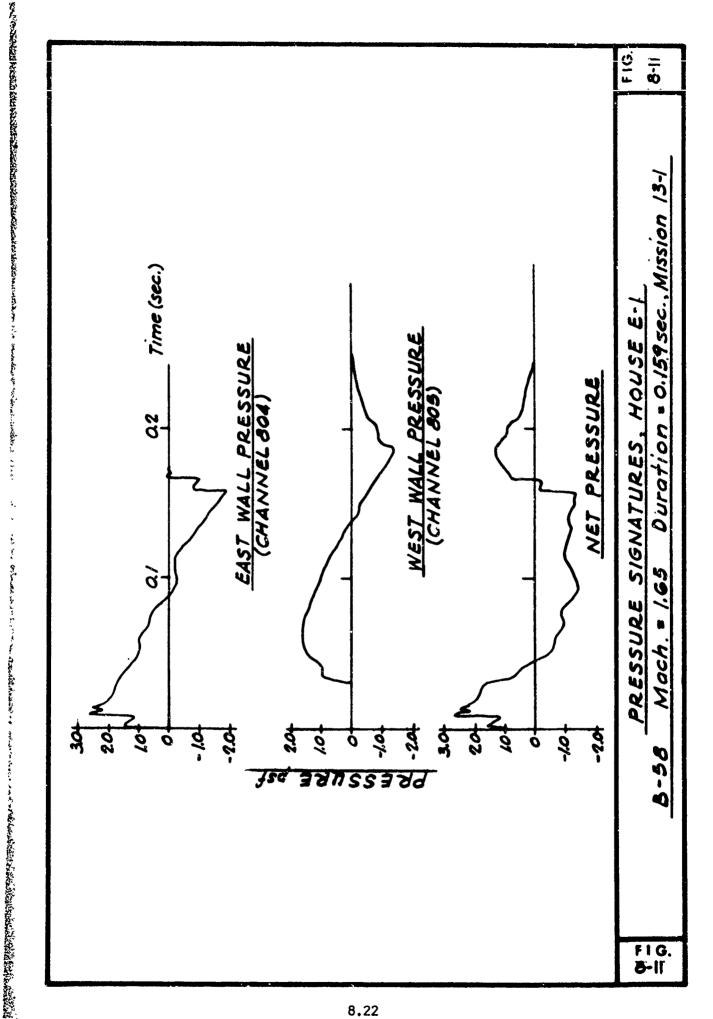
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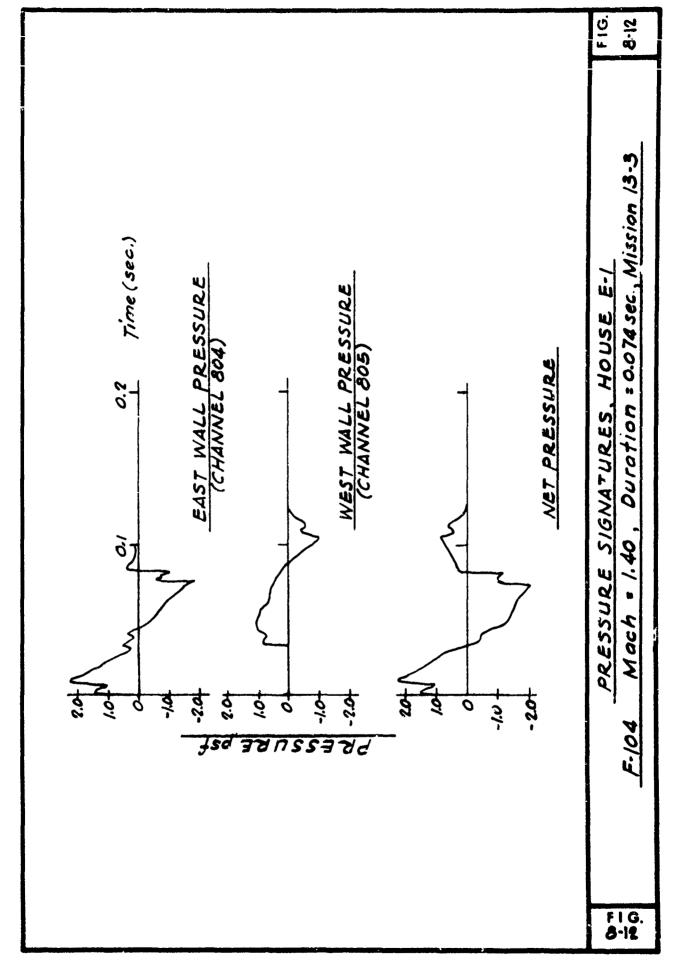
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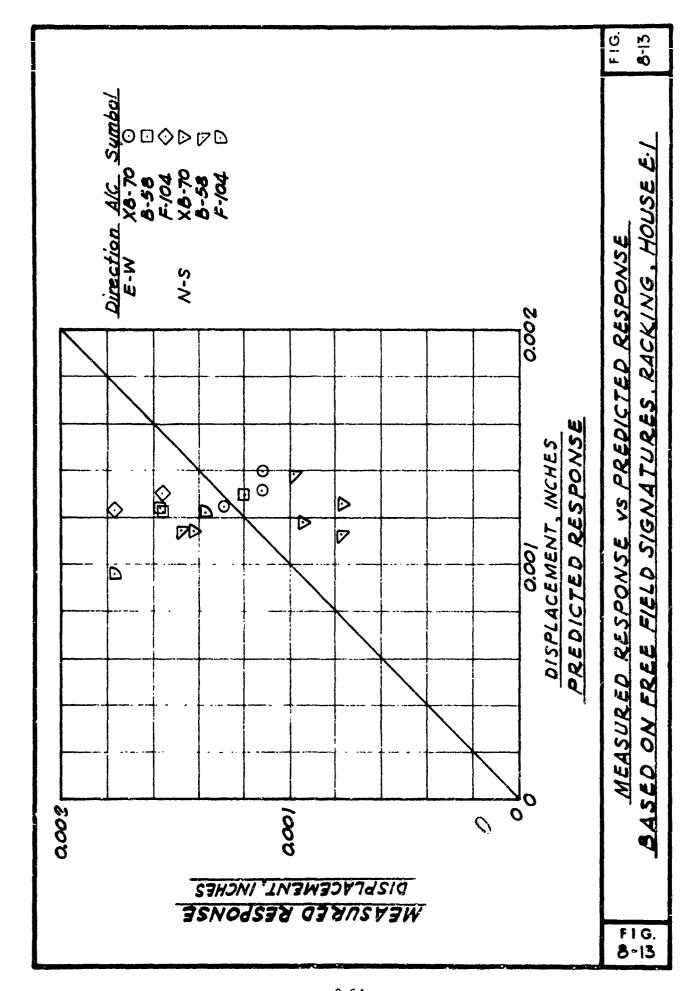


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#### IX. STRUCTURE DAMAGE

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The advent of supersonic aircraft and the accompanying sonic booms have produced undesirable side effects. Two of these effects are the irritation of people when subjected to a sonic boom and the damage to structures and structure elements caused by the sudden changes in overpressure produced by the sonic boom. The reactions of people are covered by reports by other participants in the Edwards Program. However, there would appear to be a link between a person's fear of structure damage when a boom occurs and his irritation about the boom. In this connection, education of the public with regard to what damage actually occurs versus feared damage from sonic booms would be beneficial.

In addition to helping reduce personal irritation with sonic booms, the process of adjudicating complaints and claims of sonic boom damage would be helped immeasurably if more knowledge and data were available regarding sonic boom damage. It is doubtful that sufficient data can be obtained to prove whether all damage claimed from sonic booms was or was not directly caused by a boom. Therefore, data and procedures should be developed to establish most likely or most probable causes of damage.

Since 1955 there have been over \$18 million claimed damages from sonic boom. During FY 1966 nearly 5,000 claims were filed. As time passes, more sonic booms will be generated as more supersonic flights are made and the number of damage complaints and claims will increase accordingly. The number of complaints could possibly be reduced if more were known about what actual damage does occur.

The ultimate objective of the studies of structure response to sonic booms is to understand the mechanism of failure under sonic boom loading to that damage claims can be properly evaluated. Failure implies that the loading has exceeded the ultimate strength of the structure element in question. Therefore, the objective could be divided into two parts: description of loading mechanism and prediction of magnitude of loading, and description of the strength of a structure element and prediction of its ultimate strength.

The knowledge gained through those analyses was sufficient to properly describe the loading mechanism. Mathematical and statistical models, some derived expressly for the Edwards study, can be used in predicting the load on a structure element, thus eliminating extensive instrumentation. The response of a structure or structure element can now be predicted with a good degree of accuracy. NASA has developed reliable methods for predicting the magnitude of overpressure. The analyses of Edwards free field signature data indicated that the Dynamic Amplification Factor (DAF) is influenced by the ratio of the peak negative overpressure (absolute value) to the peak positive overpressure. The studies also indicated that DAF is affected by the ratio of rise time,  $T_{ij}$ , boom duration,  $\tau$ . This ratio appears to increase with increase in lateral offset of the flight track.

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Review of the location of damage complaints at Milwaukee, Chicago, Pittsburgh, St. Louis and Oklahoma City indicated that often there are more complaints at some distance either side of the flight track than there are directly under the track. In all cases the complaints are spread over a fairly wide area. Figure 9-1 illustrates one possible cause for this lateral spread of complaints. Figure 9-1(a) shows the decay of overpressure with lateral offset. Figure 9-I(b) indicates how the ratio  $P_2/P_1$  may decrease with offset (as shown in Chapter V values of maximum DAF similarly decrease). Figure 9-1(c) illustrates the trend of the ratio  $T_{\parallel}/\tau$  as indicated by the analysis of Edwards data. Figure 9-1(d) indicates the possible combined effect of the two ratios on DAF. The amount of data recorded near the two test house structures at Edwards AFB was not sufficient to clearly define the trend of the variations in  $T_1$  and  $P_2/P_1$ . Additional free field signatures of comparable missions of XB-70 and B-58 aircraft were recorded by NASA for varying offsets of the aircraft during the Edwards testing. It is recommended that these signatures be analysed to determine if trends in the two ratios  $P_2/P_1$  and  $T_1/\tau$  can be established and hence determine the result  $\cdot$ ing effects on DAF values. These analyses could be an extension of the studies in Chapter VI. The results would also help establish the effective width of the "boom swath" laid down by supersonic aircraft and hence the number of structures exposed to potential damaging conditions. These data together with boom propagation and signature prediction methods developed by NASA should result in a method for predicting load magnitude for level flights with a good degree of accuracy.

The sonic boom tests at Edwards AFB were not designed to study the damage of test structures due to sonic booms, and no damage was observed in the test houses. However, it may be inferred from a study of the magnitudes of the deformations obtained during the test flights that damage of a properly designed and constructed house due to low magnitude booms (on the order of 2 psf) is extremely unlikely. The reasoning behind this statement is given in the following discussion of the two types of structural response; plate (lateral displacement) and racking (in-plane displacement).

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The maximum magnitudes of the plate displacements for both the walls and the large galss window in Test House E-I were considerably below the levels required to cause damage. The maximum displacement was measured for the DR, E-2 wall and was 0.034 inches (Chapter VII). This displacement corresponded to a calculated stress in the gypsum board on the interior of the wall that was less than 2 percent of the failure stress of gypsum board. The maximum stress as determined from the strain records in the E-I garage window was 790 psi, whereas the average flexural strength of regular plate glass can be taken as 6000 psi 33.

The racking displacements for both of the test houses were also less than the levels necessary to cause damage to either plaster or glass. The peak racking displacements at the roof lines of the northeast corners of the Test Houses E-1 and E-2 were less than 0.002" and 0.005" respectively. That these magnitudes were less than those required to produce damage to plaster or gypsum board walls has been substantiated by several series of previous tests. Sonic boom tests at White Sands<sup>2</sup> indicated that sonic boom overpressures of from 7 to 10 psf were required before any damage was observed in similar type structures. (It should be noted that the damage criteria was a crack that could be detected with a magnifying glass.) In addition, a series of laboratory racking tests. 22 of 8' by 8' stud wall panels with gypsum board wall covering (with and without openings) demonstrated that a minimum deflection of 0.16" could be obtained before any cracks were observed. It was estimated that the racking displacements of the test houses caused by the sonic booms could have been increased by at least five times before any noticeable damage would have occurred.

The extremely low racking displacements obtained at Edwards AFB and White Sands for nominal 2 psf sonic booms also indicated that glass damage due to a racking mode of failure is extremely unlikely and, in fact, should not occur. Previous laboratory tests<sup>23</sup> indicated that it is conservative to take 1/16" per foot of height (1/4" for a 4' high window) as the allowable racking deflection for a conventionally mounted window. In these tests, it was found that for nominal as-installed clearances of from 1/4" to 1/2", the total racking displacement before failure of the glass for a square window 4' by 4' varied from 1/2" to 2". The magnitude of deformation at failure obtained in these tests was compared with the results of the previously mentioned wall racking tests and the White Sands tests. It was concluded that considerable plaster damage should occur before any glass damage occurs due to racking deformation of the structure.

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Based on the reasoning in the previous discussion, it was concluded that damage in either a plate or racking mode of failure to a properly designed and constructed house due to low magnitude sonic booms (on the order of 2 psf) is extremely unlikely. However, more than this conclusion is needed to fully understand the problem of damage due to sonic booms. The mechanism of the loading has been well established by the EAFB tests and others, but little data is available on the strength of in-place materials and modes of failure of elements. The following questions therefore remain unanswered:

I. For complaints of sonic boom damage, what is the condition and environment of the structure in which the damage occurred? What is its age, strength, and level of maintenance? The statistical description of the strength of a structure element and prediction of its ultimate strength depend on many factors. The strength of a material can be obtained from laboratory tests. However, it is reasonable to assume that the failure strength of the material, once it is a part of a structure, may be different from its strength in the laboratory. There are many environmental conditions of a structure that could affect the failure strength of material in place as a structure element. Age and history of loading, differential shrinkage and/or settlement of the structure, large temperature differentials, weathering effects, humidity changes, wind loading, installation techniques, and other factors can have major effects on the failure point of an element.

3. What is the relationship of damage caused by sonic boom to damage caused by natural phenomena such as wind, earthquake, and weathering? Was the element exposed to possible damage by wind loading or other phenomena? Why do windows and walls withstand high velocity winds and yet apparently break under low overpressure booms? What were the overpressures at the time of damage? Does sonic boom damage occur with higher or lower frequency in elements (e.g., ceilings) that are not normally subjected to loading by natural forces.

The answers to the above questions must be obtained before the following can be determined:

- I. What is the mode of failure of structure elements due to sonic boom loading?
- 2. What damage is actually caused by sonic booms? If damage is not caused by sonic boom, what does cause it? What is the most likely cause of damage?
- 3. At what overpressure level will there be a minimum expenditure on damage claims?

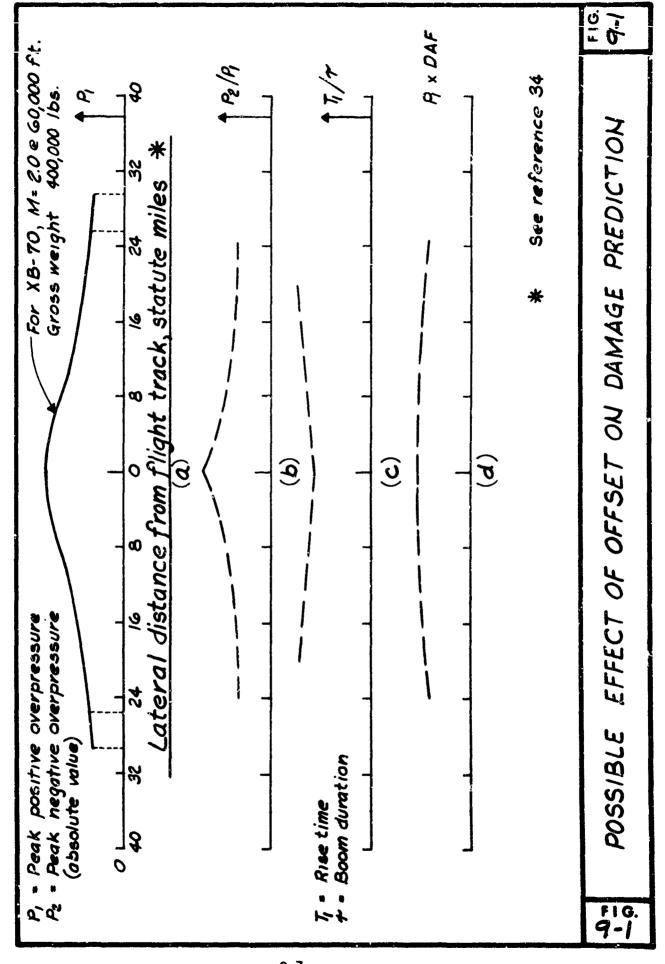
Thus, it is evident that more knowledge is required about the environmental strength and characteristics of structure elements - especially those that have been claimed as failures due to boom loading. The criterion for evaluating damage claims should be based on an evaluation of the most probable causes of failure knowing the environmental conditions of the element and the sonic boom loading on the element.

#### SUMMARY OF FINDINGS

This chapter presented a review of the results of the tests at Edwards and tests by others as applied to damage from sonic boom of low magnitudes (in the order of 2 psf). The following findings were derived:

- 1. Damage to properly designed and constructed houses from low magnitude sonic booms is extremely unlikely.
- 2. Damage should not occur to structure elements such as glass windows from racking motions caused by low magnitude sonic booms.
- 3. Other causes may result in many complaints or claims even at low levels.

4. Further data is needed on modes of failure of structure elements in order to evaluate damage and determine the most likely cause of damage.



### X. GENERALIZED DAF SPECTRUM

In the preceding chapters it was found that the response of a structure element could be computed by using free field overpressures and the DAF computed from the free field signatures. It was also found that the DAF spectra calculated by using a wave model described by free field signature parameters  $P_1, P_2, T_1$  and  $T_2$  (Figure 6-1) were equal to the DAF spectra obtained from digitized free field signatures. The purpose of the study discussed in this chapter was to derive a generalized DAF spectrum to use in predicting the response of a structure element when only the nominal free field overpressure and the type of aircraft were known.

To fulfill this purpose different non-linear regression models, see Appendix A, were fitted through DAF spectra computed from digitized free field signatures for YE-70, B-58 and F-104 missions. A total of 630 data points were used for the XB-70 missions, 540 for the B-58, and 350 for the F-104. The asymptotic behavior of these regression models was studied with the following results. For the XB-70 missions the asymptotes converged to a value of 2, for the B-58 to a value of 2 and for the F-104 to a value of 2.15. The upper frequency for which these models were computed was 50 cps. An effective cutoff in the lower frequencies was determined at the DAF value of 1. Using the signature wave model, lower frequency intercepts were calculated for free field signatures with durations equal to 0.4 and 0.5 seconds.

It was assumed in deriving this generalized DAF spectrum that 1) the free field signatures recorded in the E-2 cruciform microphone array could be considered as representative of future supersonic missions and, 2) the wave model derived for XB-70, B-58 and F-104 signatures could be extended to signatures with durations longer than 0.3 seconds.

The plotted values of the asymptotes represented the DAF spectrum. Since the modeling was done using statistical methods, the standard deviations of the asymptotes were readily determined and used in the calculation of the confidence interval. The dashed lines above and below the spectrum in Figure 10-1 represented a 95% confidence interval. This meant that there was a 95% probability that the peak values of the DAF spectrum were observed within the limits of the interval and that there was a 97.5% probability that all values of the DAF spectrum would be below the top line of the interval.

The generalized DAF spectrum could be used to obtain a prediction of the response of a known structure element. For example, knowing that an F-104 will fly at supersonic speed and will generate a sonic boom with a nominal overpressure of 2 psf, the response of a structure element can be predicted by multiplying the overpressure of 2 psf by a DAF of 2.15 and then dividing this product by the stiffness of the structure element. It should be emphasized that the DAF of 2.15 for F-104 applies only when the natural frequency of the element is greater than 4.4 cps and smaller than 50 cps. If a more detailed answer is desired and if the free field signature parameters  $(P_1, P_2, T_1, T_2)$  have been measured, a DAF spectrum can be computed from the wave model and the response then obtained from the corresponding DAF and the measured overpressure.

### SUMMARY OF FINDINGS

The purpose of the study discussed in this chapter was to derive a generalized DAF spectrum for use in predicting the response of a structure element when only the nominal overpressure and the type of aircraft were known. The data used in the analyses were measured by five microphones arranged in a cruciform array located near structure E-2 for the comparable missions of XB-70/B-58/F-104 aircraft (flights flown within a few minutes of each other). The following findings resulted from the study:

- I. A generalized DAF spectrum was obtained by studying the asymptotic behavior of DAF spectra computed from digitized free field signature data.
- 2. When the nominal pressure signature of a sonic boom is known, the generalized DAF spectrum can be used to predict the nominal response of a known structure element:
- 3. The magnitudes of the generalized DAF spectrum for different durations (Figure 10-1) were:

Duration of Bcom in Seconds	Range of Natural Frequencies in cps	Generalized DAF Magnitude
0.5	0.8 to 50	2.0
0.4	1.1 to 50	2.0
0.3	1.5 to 50	2.0
0.2	2.1 to 50	2.0
0.1	4.4 to 50	2.15

4. If a DAF spectrum is desired that is more detailed than the generalized DAF spectrum, and if the free field signature parameters ( $P_1$ ,  $P_2$ ,  $T_1$ ,  $T_2$ ) have been measured, a DAF spectrum can be computed from the wave model as described in Chapter VI.

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### XI. DAMAGE COMPLAINT INVESTIGATIONS

During the planning phases of the Edwards Test Program it became evident that many of the supersonic missions would subject a large number of buildings and structures at Edwards AFB and in communities near Edwards to sonic booms. Based on past experience, some damage was expected to occur. Therefore, to provide a fairly reliable basis for determining the extent of glass damage caused by the test program, a survey was made of all glass windows and doors in buildings and structures at Edwards. Provisions were also made to have an engineering-investigator inspect each complaint received from Edwards and the adjacent communities. The results of the glass survey are presented in the following text. There were many more complaints due to Phase I than Phase II missions as the supersonic flight paths were quite different during the two phases. Therefore, the complaints received and investigated are discussed in two parts, Phase I and Phase II. The results of the investigations of the on-site (Edwards AFB) and off-site complaints are then analysed. The chapter ends with a summary of findings and conclusions.

### SURVEY OF GLASS WINDOWS AT EDWARDS AFB

Prior to the test program, a survey was conducted of all glass panes in structures located at Edwards AFB. Survey forms were sent to occupants of the 2,226 residential units on the Base. Of these, 567 or about 25 percent returned completed forms showing a total of 101 cracked glass panes. Based on these returns, it was estimated then that there were about 404 cracked panes out of the total of 49,730 window panes (including glass doors). The total number of glass panes in the residential units was determined from drawings of the structures which were made available by Base Civil Engineering. In addition to the residential units, all buildings and facilities used for Base Operations were surveyed. Survey forms were sent to the custodians of the 2,912 buildings located on the Base. All forms were returned and reported a total of 60,660 panes of glass. 269 cracked panes and 25 broken or missing panes were reported. Table 11-1 lists the number of housing and building units, the total number of glass panes, and the number of broken and missing panes reported in the survey. The number of panes per person and size

All figures and tables are placed at the end of this chapter.

group differ from those found in San Antonio<sup>30</sup> where 56 percent of the glass panes were 0 to 2 square feet, 43 percent were 2 to 9 square feet, 0.8 percent were 9 to 40 square feet, and 0.2 percent were over 40 square feet. Based on an average of about four persons per residential unit plus occupants in bachelor quarters, a resident population of about 10,000 people was calculated for Edwards AFB. Using this population and all buildings on the Base, there is an average of 11 window panes per person, or, using residential housing only, there is an average of five panes per person. In San Antonio there was an overall average of 19 panes per person.

### COMPLAINTS FROM PHASE I MISSIONS

### Description of Mission Flight Paths

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Two different headings were flown by most of the aircraft during the three weeks of Phase I test missions. From 3 June through 12 June, missions were flown from east to west on a straight course heading of 2450 magnetic from a point several miles east of the test structures E-! and E-2 to a point just past the structures. Missions from 13 June through 23 June were flown east to west at 233° magnetic. The supersonic "racetrack" course shown in Figure II-I was flown by B-58 aircraft from 3 June through 12 June. The B-58 aircraft maintained essentially constant speed throughout the racetrack pattern. Radar plots indicated that all aircraft did not follow the precise radius of turn indicated. In addition, the flight tracks of many missions were not plotted after the aircraft started the turn to the north. Therefore, for many damage complaints, aircraft mission and location were not available. During this first period, the closest distance from flight track to the Lancaster test structure, L-2, was about 13 miles. The distances to most of Lancaster, Quartz Hill and Palmdale were greater. A total of 47 B-58 missions at Mach 1.5 to 1.65 were flown over this racetrack course. The F-104 and F-106 aircraft slowed to subsonic speed shortly after passing over the test structures. Table II-2 lists the number of supersonic missions for each aircraft for the 3 June to 12 June period.

Figure 11-2 shows the scheduled supersonic racetrack course flown by B-58 aircraft during the period 13 June to 23 June. The distance from the flight track to the Lancaster test structure for the 233° magnetic track was reduced to about 8 miles. The distances to the rest of Lancaster, Quartz Hill and Palmdale were similarly reduced. A total of 47 B-58 missions at speeds of

Mach 1.5 to 1.65 were flown over this course. The number of supersonic missions for each aircraft for the 13 June through 23 June period are listed in Table 11-2.

### Location and Types of Damage

Complaints from people other than those living at Edwards AFB were received by the Base Claims Office and daily summaries of the complaints were furnished to Blume personnel during the test flight period. Base Civil Engineering received complaints from personnel occupying residential housing on the Base, and two complaints of boom noise were received at the Air Force Plant, Palmdale. The total number of complaints received and initially attributed to Phase I of the Edwards Test Program were as follows:

Office Receiving Complaint	Number of Complaints-Phase I
Edwards AFB - Claims Office	51
Edwards AFB - Civil Engineering	8
Air Force Plant 42, Palmdale	_2
	61

Complaints were classified as to type of damage as shown in Table II-3. Table iI-4 lists the location of all Phase I complaints arranged chronologically by date of occurrence of damage. Complaints were investigated by an engineering investigator with AFLC Forms 666, 669, and 670 (see Figures II-3, II-4, and II-5) used for recording the results of the investigations. The orientation of the damage incurred in each structure was also noted.

### Comparison of Damage with Aircraft Mission

The engineer's investigation reports were analysed together with the mission log and the radar plots to determine if the type and speed of aircraft and location of flight path could be correlated with the damage. The missions on a given day were flown with only a short time interval between them and therefore in most cases it was r 'possible to pinpoint a specific boom as the cause of damage at a particular location. The major problem was that a person filing a complaint could usually give only an estimate of the time of occurrence of the boom which caused the damage. This time estimate often spanned an hour and occasionally a whole morning. In addition, many of the radar plots did not show the entire supersonic track of each aircraft.

A few of the plots were started before Barstow while many were stopped at the turn point of the racetrack course. The analysis of the damage complaints and aircraft missions was first divided into two sections that corresponded to the different flight track headings: 3 June through 12 June - 245° magnetic, and 13 June through 23 June - 233° magnetic.

### 3 June through 12 June - 2450 Magnetic (Figure II-I)

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Table II-4 lists all complaints received during Phase 1. Sixteen complaints were received that were attributable to the 3 June through 12 June period, for an average of 0.31 complaints per mission. Figure II-1 shows the locations and types of complaints.

In two instances during the 3 to 12 June period, specific booms were related to damage:

Barstow - 7 June - A large window was reported broken at about 0930. The radar plot showed a B-58 aircraft maneuvering to get on the correct track and heading at about the time of the reported damage. On the radar plot Barstow was less than five miles south of the track of this aircraft.

Edwards AFB Housing - 8 June bric-a-brac complaint was received from the Base housing area ming damage to a figurine that fell off a shelf at 0908. Mission log data showed a boom from a B-58 at 0908 at Radar Control which is a short distance from the housing area. The flight was displaced five miles north from the flight track over the test structures or almost directly over the Base housing area. A free field overpressure of 3.17 psf was recorded near test house E-2 on the Base.

### 13 June through 23 June - 2330 Magnetic (Figure 11-2)

The number of complaints increased from sixteen for the 3 to 12 June period to 45 for the 13 to 23 June period. Table 11-4 lists all complaints received and Figure 11-2 shows the locations and types of complaints for this period. For this period there was an average of 0.56 complaints per mission as compared to 0.31 for the 3 to 12 June period.

During the 13 to 23 June period, over half of the complaints occurred on two days. Twenty-four complaints were reported on 20 and 21 June. All of the complaints from the Quartz Hill area, one from Lake Isabella, four from Lancaster and six from Tehachapi were reported for these two days. All missions on these two days were flown by B-58 aircraft with a number of missions

having nominal 3 psf design overpressures. Average overpressures recorded at Edwards AFB showed three booms over 3 psf, eight over 2.5 psf, four over 2.0 psf, and all other missions except four over 1.5 psf. Average overpressures recorded near the test structure L-2 (Lancaster) exceeded 2 psf for two missions (2.04 and 2.35 psf), were between ! and 2 psf for eight missions, and were less than I psf for eight missions. The radar plots indicated a B-58 aircraft descending before reaching Rosamond at 0935, 20 June. Complaints were received from Quartz Hill and Lancaster that claimed damage before and after this time. The radar plots indicated several other B-58 aircraft on 20 and 2! June descending while in the vicinity of Tehachapi. These aircraft could not be identified as to mission numbers.

Three complaints were received for damage due to test program booms on 14 and 15 June. No damage complaints were received for 16 June. Only F-104 supersonic missions were flown on these three days. As noted previously, F-104 missions normally slowed to subsonic speed shortly after passing over the test houses at the Base. Therefore, the number of complaints would be expected to be less as fewer buildings were exposed to booms. The maximum average overpressure recorded near test structure E-2 at Edwards for these three days was 3.96 psf at 0915 on 15 June 1966. The maximum overpressure recorded at L-2 was 1.21 psf on 14 June 1966.

For the missions flown on 233° magnetic, two specific missions could be related to specific damage:

<u>Tehachapi - 20 June</u> - The Postmistress happened to be looking at a clock opposite her desk at the time a boom broke a window in the U. S. Post Office. At the same time, a window was broken in a department store located in the same building. The time was noted as 1043. The radar plot indicated a B-58 aircraft at this time had just turned to the east a short distance beyond Tehachapi.

Lake Isabella - 20 June - A window was reported broken at approximately 0915. The radar plot showed a B-58 aircraft in a supersonic turn in the vicinity of Lake Isabella at 0900. This was approximately 30 miles north of the scheduled return leg of the track.

3 June through 23 June - both flight tracks.

The complaints for both periods and all aircraft headings were tabulated in Tables II-5, II-6, and II-7. Table II-5 compared the type of damage and period during which damage occurred. There were 83 incidents of damage of all types reported during Phase I. Table II-6 compared the number of damage inci-

dents with geographical location. Aircraft missions, date of occurrence of alleged damage, number of valid complaints, and the maximum average overpressure measured near E-2 and L-2 were compared in Table II-7. Of the forty incidents of valid glass damage, all but four were attributed to B-58 missions. The term incident denotes one damaged pane of glass or one piece of bric-a-brac. Of the four incidents possibly attributable to F-104 missions, two occurred on days when only F-104 aircraft flew test missions and the other two damage incidents occurred at Edwards AFB on a day when both B-58 and F-104 missions were flown. For the incidents of valid damage other than glass, one of bric-a-brac damage occurred on a day of F-104 missions and in a location probably overflown by these missions. No incidents of damage could be directly attributed to XB-70 missions.

in summary, 90 percent of the incidents of valid glass damage (engineering investigator found damage could have been caused by sonic boom) and 89.5 percent of all incidents of valid damage were attributed to B-58 missions. Of these valid damage incidents possibly caused by B-58 missions, approximately 60 percent occurred on days when the maximum average positive overpressure exceeded 3 psf. One important unknown factor was the actual overpressure adjacent to the damaged elements. This could have a major bearing on the cause of damage as the B-58 aircraft were turning and in some cases descending at supersonic speeds over or near the areas where the damage occurred; "superboom" overpressures greatly exceeding those measured near E-2 and L-2 could have been produced.

For Phase 1, there was a total of 61 complaints involving 83 incidents of damage. Of the total, 48 incidents of damage, or 58 percent, appeared to be valid after investigation (damage possibly caused by a sonic boom). For 25 of the 52 glass damage incidents, repairs had been made or the glass removed prior to the arrival of the engineer-investigator. The cause of the cracks in the glass therefore could not be definitely established to be from causes other than sonic boom. Seventy percent of the valid complaints were made by owners of the structures involved. Eighteen claims have been filed with the Edwards AFB Claims Office. Sixteen of these claims have been paid for a total of \$2,199.93. One claim is still pending.

### COMPLAINTS FROM PHASE II MISSIONS

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All supersonic missions during Phase II of the test program were flown from east to west on a  $245^{\circ}$  magnetic heading. Missions except those by the

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XB-70 were scheduled to slow to subscnic speeds shortly after passing over the test structures on the Base. The XB-70 missions normally made gentle turns to the north after passing over the test structures E-1 and E-2. As a result, a much smaller number of buildings were subjected to test program sonic booms and few areas were overflown by aircraft making turns or other maneuvers at supersonic speeds.

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Table II-8 lists by date of occurrence and location of damage the eleven complaints that could be attributed to flights during the 31 October 1966 through 17 January 1967 period (Phase II). Five glass damage complaints were recorded. Three of the eleven complaints were for damage that occurred on days when no test program flights were flown. Damage incidents by location and by type were compared in Tables II-5 and II-6.

After investigation, seven of the incidents of damage were recommended for payment if claims were filed; five could be assigned to test program flights and two were apparently caused by SR-71 flights on I December. Four of the five damage incidents attributable to program flights were for glass damage and one was for bric-a-brac. XB-70 missions appeared to have been the cause for two of the glass damage incidents while B-58 missions apparently caused the other two. The bric-a-brac incident was traced to an XB-70 flight. As of June 22, 1967, three claims have been filed and \$55.27 has been paid for two approved claims. One claim is still unsettled (awaiting information from claimant).

A tabulation was made in Table II-9 of the complaints received, aircraft missions flown, and the range of maximum average overpressures measured near E-2. Two (50%) of the incidents of valid glass damage were attributed to B-58 missions making a supersonic turn over Mojave. Average overpressures recorded at Edwards AFB showed four booms over 3 psf, five over 2.5 psf, three over 2 psf, and all other missions except two over 1.5 psf. The percent of total glass damage incidents attributed to B-58 missions during Phase II was nearly half (55%) of the percentage found during Phase I. This difference was largely due to the change in the supersonic flight plan for the B-58 aircraft. In Phase I all B-58 aircraft flew on a supersonic racetrack course, whereas during Phase II the aircraft slowed to subsonic speed shortly after passing E-I and E-2. Therefore, fewer buildings were subjected to B-58 sonic booms and the booms were not normally produced by aircraft maneuvering at supersonic speeds. The number of incidents of damage attributed to XB-70 missions is not surprising as there were seventeen supersonic missions flown during Phase II versus only three during Phase I.

### EVALUATION OF DAMAGE COMPLAINTS FROM PHASES ! AND !!

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There were 72 complaints involving 96 incidents of damage received during Phases I and II of the test program. After investigation, 55 of the incidents appeared to be valid (damage that could have been caused by sonic boom test program). An incident of damage denotes one cracked or broken glass pane, or one category of other type of damage (cracked plaster or cracked stucco, etc., Table II-3). Thirty-seven, or 72.5 percent, of the valid incidents of damage were reported by owners of the property involved. The remainder of the complaintants were renters. Eighty percent of the valid complaints were for glass damage, 5.5 percent for plaster or stucco, and I4.5 percent for damage to bric-a-brac and other fallen objects. Glass damage was by far the major source of complaints. Therefore, additional analyses of the glass complaints were made.

The combined population of Palmdale, Lancaster, Rosamond, Quartz Hill and Tehachapi is about 55,000. Assuming 19 window panes per person 50. a total of about 1,045,000 panes were subjected to sonic boom. Assuming II panes per person (based on the total number of window panes at Edwards AFB) a total of 605,000 panes of all sizes in these communities were subjected to sonic booms. The latter total figure was used for the analyses of damage incidents rather than the former for three reasons. First, the distribution of glass sizes was felt to be more similar to those at Edwards AFB because the type of residential construction at Edwards is similar to the surrounding area. Secondly, the proportion of glass sizes 0 to 2 square feet was much less and 9 to 40 square feet was much greater than those found at San Antonio $^{30}$ . Many residential units in the areas surrounding Edwards have large windows  $(2.5' \times 6' \text{ to } 4' \times 6')$  and sliding glass doors. As a result there are more 9 to 40 square feet panes and fewer 0 to 2 square feet. The third reason for using the lower number of panes per person was that complaints and incidents of valid damage per million boom-pane exposures would be based on a more conservative figure for glass population. A boom-pane exposure is one pane subjected to one boom.

Table II-IO lists the total number of glass panes for communities adjacent to Edwards, the number of complaints of damage received, and number of panes in each size damaged by sonic boom. The number of damaged panes per million total boom-pane experience was also calculated for each glass size group. II4 sonic booms were used in this calculation, based on three XB-70 and 94 B-58 missions during Phase I and I7 XB-70 supersonic missions during Phase II. The

Century series fighter missions were not included as most of these aircraft slowed to subsonic speeds soon after passing the test structures. The B-58 missions during Phase II similarly reduced speeds. Information regarding the headings for the YF-12 and SR-71 missions was not released and therefore it was not known what areas were overflown outside of Edwards. They were therefore excluded.

Table II-II presents a tabulation of the glass panes and complaints of glass damage at Edwards AFB. 357 sonic booms were used to calculate the boom-pane exposure as all supersonic test missions measured and recorded near E-2 during Phases I and II passed near most of the structures on the Base.

The glass panes at Edwards AFB were surveyed prior to start of test missions while those in adjacent communities were not. The types of construction, particularly the residential units, for both areas are very similar. The size distribution of glass panes in residential units therefore should be similar for both areas. The communities adjacent to Edwards had a total of 69.0 million boom-pane exposures. The Base had 39.4 million boompane exposures during the test program plus millions of boom-pane exposures from other than test program missions. The complaints of glass damage of all sizes at Edwards totalled 0.203 panes per million exposures while from the adjacent communities they totalled 0.566 panes per million exposures; a rate 2.8 times greater. The number of panes possibly damaged by test program booms were 0.127 per million boom-pane exposures at Edwards versus 0.464 damaged panes per million boom-pane exposures in the adjacent communities; or 3.7 times the Edwards rate. The Edwards rate exceeded that found in the adjacent communities; in one glass size group (0 to 2 square feet), 0.076 to 0.015 damaged panes per million boom-pane exposures.

Certain conclusions could be drawn from these results. The glass panes at Edwards were surveyed prior to the test missions and their condition determined. This survey was made to eliminate as far as possible any confusion that might arise in recognizing boom damage versus damage cased by differential shrinkage or differential settlement of a structure. Some of the complaints received from EAFB housing were for damage obviously not caused by test program booms. Therefore, i. was concluded that the remaining glass damage at EAFB that occurred during the test program was most likely caused by a boom.

A second possible conclusion was that the glass panes at Edwards have been subjected to so many sonic booms over the years that the "weak sisters"

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were broken prior to the test program. A more convincing explanation for the difference in rates of damaged panes was that the missions apparently causing nearly 90 percent of the glass damage were B-58 aircraft maneuvering at supersonic speed. All aircraft were on straight courses while passing over the Base and therefore the Edwards structures were not subject to possible super booms created by maneuvering aircraft. It was therefore concluded that the rates of glass damage per million boom-pane exposures at Edwards should be used for predicting future glass damage from level supersonic flights with similar overpressure levels rather than the rates determined from investigation of glass damage complaints in the communities adjacent to Edwards.

The valid incidents of glass damage were also examined on the basis of type of frames, previous condition and prientation, Tables 11-12. Eleven of the damaged panes had been installed in wooden frames and thirty-one in aluminum frames. There was no correlation between type of frame and damage. Based primarily on statements of the complainants and partly on visual inspection (if the glass was still in place) all glass panes were in good condition prior to the time of boom damage. No relationship of orientation of damaged panes to aircraft producing the boom was possible as there was not sufficient information available to definitely determine location of the boom-producing aircraft.

It is of interest to note that no damage from sonic booms occurred in either test structure E-I or E-2. This can be explained by the fact that the construction of the structures was completed just prior to the start of test missions. The structures were less than one year old at the end of the test program. Therefore the window mounting details were in good condition; there was little time for differential settlement to occur in the structures, and the structures had not been subjected to repeated weathering effects.

### SUMMARY OF FINDINGS

The preceding text has discussed the results of a survey of all glass panes in structures at Edwards AFB and the results of investigations and analyses of damage complaints received during Phases I and II. Overpressure measurements were made by instruments located about two miles from most of the structures on the Base. However, no overpressure measurements were available for the communities adjacent to the Base where the largest number of damage complaints were reported; therefore direct comparisons of damage and

overpressure were not possible. Based on detailed analyses of the avialable data, the following findings are presented:

- I. The rate of valid glass damage in Edwards AFB Buildings, all of which had been condition surveyed prior to the test program, was 0.127 panes damaged per million boom-pane exposures or 27 percent of the rate for buildings in communities adjacent to Edwards which were not condition surveyed prior to test missions.
- 2. During Phase I, the IIO,390 glass panes in structures at Edwards were subjected to more booms from test missions than were the 605,000 glass panes in the adjacent communities; however, the aircraft while over Edwards were flying straight courses and then made turns at supersonic speeds over adjacent communities. Some focusing of the boom overpressure (or super booms) may therefore have been produced with peak overpressures greatly exceeding those produced on the Base.
- 3. During Phase I, 90 percent of the incidents of valid glass damage (engineering investigator determined damage could have been caused by sonic boom) were attributable to B-58 missions. The remaining 10 percent were apparently due to F-104 missions.
- 4. The valid glass damage rate per mission during Phase I was 8.8 times the rate during Phase II when aircraft generally flew straight courses while at supersonic speeds.
- 5. The num in of complaints received decreased from 61 during Phase I to III during Phase II. This large decrease in number of complaints can be attributed to two factors: a) the 8-58 aircraft made turns and other maneuvers at supersonic speeds over several communities adjacent to Edwards AFB during Phase I, and b) during Phase II the XB-70 flew supersonically on a relatively straight course over a few of the cities adjacent to Edwards.
- 6. For all incidents of damage recorded during Phases I and II, 60.5 percent were for glass damage.
- 7. Fifty-eight percent of all incidents of damage received during Phases I and II were listed as valid. Of these valid incidents, 80 percent were for glass, 5.5 percent for plaster or stucco, 0.0 percent for structural, and 14.5 percent for bric-a-brac or other fallen object damage.

- 8. Glass damage was repaired or the broken glass removed for 55 percent of the glass damage incidents before the engineering investigator could investigate the alleged damage and hence, the validity of all glass damage could not be definitely established.
- 9. Damaged glass panes ranged in size from 1.3 square feet to 82.5 square feet. 2.7, 43.2, 43.2 and 10.9 percent of the incidents of damage occurred in the 0-2, 2-9, 9-40 and over 40 square feet size groups respectively.
- 10. No sonic boom damage was observed in the test structures prior to or after the test flights. There were minor shrinkage cracks in the test structures prior to start of test flights. However, no discernible extension or widening of these cracks was observed although observations were made and recorded daily.
- II. As the condition of the glass panes at Edwards AFB was determined prior to the Test Program, the number of damaged panes caused by booms from test missions should be a reliable indicator of  $v_{\rm cl}$  iid glass damage to be expected from future level supersonic flights generating sonic boom peak overpressures of 2 to 3 psf. The rate was one damaged pane per 7.9 million boom-pane exposures.
- 12. A large percentage (from 57 to 84 percent) of future valid incidents of damage from sonic boom should be for glass.

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TABULATION OF GLASS PANE SURVEY
OF BUILDINGS AND STRUCTURES AT EDWARDS AFB

	Number of	Size of G	Number of Size of Glass Panes in Square Feet	in Squar	e Feet		Cracked Panes	Broken or Missing
Structures	Units	0-2	2-9	9-40	9-40 Over 40 Total	Total	Reported	Panes Reported
g <sub>r</sub>	2,226 <sup>a)</sup>	3,500	19,720	26,510	0	49,730	404 <sup>b)</sup>	0
Base Operation	2,912	21,647	29,696	6,773	2,544	099*09	269	- S2 - L2
Total Percent of Total	5,138	25,147 22.8	49,416	33,283 30.2	2,544	110,390	673	25

Based on 101 cracked panes reported on the 25% of forms returned.

TABLE 11-2

SUMMARY-PHASE I SUPERSONIC MISSIONS

Determined from drawings of Base Housing.

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Aircraft	Dates	No. of Missions	Primary Heading	Comments
B-58	3-12 June	•	245 <sup>0</sup> Mag	Racetrack Course
22 2 X8-70	3-12 June	ю	245° Meg (16262°M)	Straight Course
F-104	7-12 June		ı	Straight Course
B-58	13-23 June	, 47	233 <sup>0</sup> Mag	Racetrack Course
F-104	13-23 June	34	233 <sup>0</sup> Mag	Straight Course

a) from Mission Log, Edward, Phase I, Appendix D.

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### TABLE 11-3 CLASSIFICATION OF TYPES OF COMPLAINTS

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Notation	Type of Complaint
1.	Glass Damage - Window and/or Door
2.	Glass Damage - Miscellaneous,(Auto, Mirror)
3.	Plaster cr Stucco Damage - Cracks
4.	Plaster or Stucco Damage - Fallen
5.	Structural Damage
6.	Fallen Object Damage - Bric-a-brac
7.	Fallen Object Damage - Miscellaneous
	(Fixtures, lamps, mirrors, etc.)
8.	Miscellaneo Damage
	(TV, Bathroom fixtures, etc.)
9.	Noise Complaint - No Damage
10.	Information Call - No Damage

SUMMARY OF COMPLAINTS ATTRIBUTED TO PHASE 1<sup>a)</sup>
(All dates are 1966)

Time of Occurrence	rrence	Date	Date Complete Complete Location	401+6001		True of Claimb	ا ا	ğ	Results of Investigation
Date Oate	Time	Received	Number		į	Explanation	Rent	\$	A/D Remarks
1965 NA		VA	56	EAFB	_	Glass- Window(!)	Ş	۵	Damage occurred prior to program
Prior to Program	gram	9 Jun	59	Barstow	_	Glass- Window(1)	0	۵	Demage occurred prior to program
6 Jun 1000-1030	030	l Aug	19	Lancaster 7	7	Fallen Mirror	0	Ş	No Claim filed
6 Jun 100C-2000	000	6 Jun		Tehachapi		Glass - Window(1)	0	۵	Previous damage
6 Jun 1000-2000	000	e Jun	m	Lancaster	_	Glass - Window (2)	0	≪	Will not file claim
					7	Fallen Picture		⋖	
6 Jun 0900-1100	<u>8</u>	9 Jun	ø	Rosamond	m	Plaster and Stucco	0	۵	Probably shrinkage and settlement
6 Jun NA		e-10 Jun	57	EAFB	_	Glass - Window (1)	œ	۵	Broken by B-B gun
6 Jun AM		9 Jun	7	Barstow	М	Stucco	0	۵	Probably shrinkage and settlement
6 Jun NA		e-10 Jun	55	EAFB		Glass-Window (3)	œ		Insufficient data available
6 Jun AM		20 Jun	22	Tehachapi	_	Glass - Window (!)	œ	•	Claim filed and paid
						and door (1)			

## TABLE 11-4 (Continued)

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- BE -	Time of Occurrence	Date	Date Complaint Location	Location		Type of Claim	- LEWO	İ	Results of Investigation
Date Time		Received	Number		<u>Ş</u>	Explanation	Rent	1	A/D Remarks
6 Jun NA	٧	6-10 Jun	52	EAFB		Glass-Window (1)	œ	⋖	Possibly caused by program
7 Jun	0930-1030	7 Jun	2	Barstow		Glass-Window (1)	œ	<	Claim filed
7 Jun	0011-0060	o Jun	9	Rosamond	See	e June			
7 Jun	Æ	o Jun	7	Barstow	See	e june			
7 Jun	AM	20 Jun	22	Tehachapi	_	Extended crack in door- See 6 June	oor- Se	9	lune
8 Jun	8060	8 Jun	4	EAFB	9	Figurine on shelf	œ	∢	Claim filed and paid
8 Jun	0011-0060	9 Jun	9	Rosamond	See	e June			
8 Jun	₹	9 Jun	7	Barstow	See	6 June			
8 Jun	0830	27 Jun	44	Barstow	m	Stucco	ပ	۵	Previous cracks, probably shrinkage
unf 6	0011-0060	9 Jun	9	Rosamond	See	6 June			
9 Jun	₹	o Jun	7	Barstow	See	6 June			

TABLE 11-4 (Continued)

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	Results of Investigation Own-	Rent A/D Remarks		O A Claim filed and act of Investigate	۵ ۵	program flights	Owner just warned	0	• •	0	0	beams 0 D		∢ 0	0 Damage occured prior to program	0	
	Type of Claim	Explanation	· · · · · · · · · · · · · · · · · · ·	Plaster	Shattered porch light R	giche	No damage	TV (Protective glass	On picture tube)	Glass-Window (1)	Glass-Window (1)	Exposed ceiling beams	twisted	Dishes on shelf	Plaster	Glass-Window (2)	Clock on wall
	Location	No.	Lancaster 7	Tehachapi 3			Lancaster 10	Lancaster 8	Palmdale	Lancaster 1	Tehachap! !	Lancaster 5		Rosamond 6	4	Tehachapi i	Lancaster 7
	Complaint Location	. Number	ω	12	13		Ŋ	32	6	01	14	15		_		8	31
Date	Complaint	, scelved	10 Jun	13 Jun	9 June		21 Jun	22 Jun	13 Jun	13 Jun	14 Jun	20 Jun		13 Jun		17 Jun	21 Jun
Time of Occurrence	of Damage	Date Time	9 Jun AM	9 Jun 0930	9 Jun 1400		6-11 Jun -	11-17 Jun NA	13 Jun AM	13 Jun 0953	13 Jun AM	13 Jun 1000-2000		13 Jun NA		13-17 Jun NA	14-15 Jun 0915

A TOTAL STATE OF THE STATE OF T

[ABLE 11-4 (Continued)

Date	of Damage	Complaint	Complaint Complaint Location	Location	Type of Claim	0		Results of investigation
	Time	Received	Number	ું	"	Rent	₽	Rent A/D Remarks
0021 un 1200	200	onl. 92	Ş	lancaster 7	Light fixture-glass	<b>₹</b>	× z	¥.
14 .lun 1	14 Jun 1660-1615	15 Jun	: R	EAFB	Glass-Window (2)	œ		
20 Jun 1	20 Jun 1030-1100	20 Jun	91	Tehachapi i	Glass-Window (1)	0	∢	
20 Jun 1022	022	21 Jun	61	Tehachapi 1	Glass-Window (1)	0	•	Claim filed and paid
20 Jun 1043	043	20 Jun	21	Tehachap! !	Glass-Window (1)	0	•	Claim filed and paid
20 Jun 1044	044	20 Jun	22	Tehachapi I	Glass-Window (1)	œ	<	Claim filed and paid
20 Jun 1000	000	20 Jun	22	Lancaster	Glass-Window (!)	œ	⋖	Approve 75% payment
								previous cracks.
20 Jun MA	\$	14 Jul	92	Lancaster !	Glass-Window (2)	0	<	Negotiate settlement
20 Jun AM	3	21 Jun	27	Quartz Hill 7	Light fixture	0	۵	
20 Jun 1045	1045	20 Jun	28	Quartz Hill 1	Glass-Door (1)	0	<	
20 Jun 1015	510	22 Jun	17	Tehachape (	Glass-Window (1)	0	⋖	Claim filed and paid

TABLE 11-4 (Continued)

Time of Occurrence of Damage	Date Complaint	Date Complaint Complaint Location	Location	·	Type of Claim	- Law		Results of investigation
Date Time	Received	Number		9	Explanation	Rent		A/D Remarks
20 Jun NA	6 July	45 T	Tehachapi 5		Brick Pillar, 27" high supporting wood fence rails	0	0	Will not file claim
20 Jun 0910	20 Jun	29 0	Quartz Hill	_	Glass-Window (2)	0	∢	
20 Jun 0910	20 Jun	33 L	Lancaster		Glass-Window (2)	0	∢	Claim filed and paid
20 Jun 4M	24 Jun	37 0	Quartz HIII 1	_	Glass-Window (1)	0	⋖	
20 Jun 0915	20 Jun	38 L	Lake Isabella		Glass-Window (!) Glass-Window (!)	0	<b>4</b> 0	Damage occurred prior to program
20-21 Jun AM	21 Jun	42 0	Quartz Hill I 7		Glass-Window (2) Medical type office scale fell	0	<b>«</b> 1	No claim
20 Jun	22 Jun	34 L	Lancaster 3	۰,	Stucco	0	۵	Previous cracks, probably shrinkage
21 Jun AM	2; Jun	Z0 T	Tehachapi <sup>1</sup>		Glass-Window (I)	0	<b>⋖</b>	Claim filed and paid

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THE PROPERTY OF THE PROPERTY O

Time of Occurrence	}					,		Results of Investigation
Date Time	Received		N. then	3	Type of Claim		1 5	
	200			2	EXPIGNATION	Kent	2	A/U Kemarks
21 Jun 1315	21 Jun	8	Lancaster	wr	Plaster-ceiling Light fixture fell from celling	0	< <	Will not file claim
21 Jun NA	23 Jun	40	Lancaster	m	Plaste:-celling	0	<b>⋖</b>	Partially approve - 50% (Previous water damage) Claim filed and pald
21 Jun 0905	22 Jun	14	Lancaster	2	Attic access cover fell - molding supports loose	0	1	Information call, will not file claim
21 Jun 0910	21 Jun	46 T	Tehachapi	_	Glass-Window (2)	0	<	
21 Jun NA	21 Jun	8,4	Quartz Hill	0	Notse	A A	•	(Complaint through AF Plant 42)
21 Jun NA	21 Jun	54 E	EAFB	_	Glass-Vinde (12)	or	۵	Insufficient data available
21 Jun 0905-0945	21 Jun	49 L	Lake Hughes	6	Noise	≨	ŧ	(Complaint through AF Plant 42)
22 Jun NA	24 Jun		EAFB	_	Glass-Window (2)	œ	ĸ	

TABLE 11-4 (Continued)

Time of Occurrence of Demage	Date Complaint	Complaint	Date Complaint Complaint Location		Type of Claim	Own-		Results of Investigation
Date Tim≪	Received	Number		9	Explanation	Rent	Ş Ş	Rent A/D Remarks
23 Jun NA	23 Jun	53	EAFB	_	Glass-Window (1?)	œ	٥	Insufficient data available
23 Jun 0845	23 Jun	24	ЕАҒВ	2	Glass-Auto Windshield	0	ı	Complaint withdrawn
23 Jun 0955	24 Jun	58	Tehachapi	_	Glass-Window (1)	<u>«</u>	<b>«</b>	
23 Jun 0855	23 Jun	23	Tehachapi	_	Glass-Window (1)	œ	⋖	Claim filed and paid
23 Jun 0912-1256	23 Jun	35	Paimdale		Glass-Window (1)	œ	<	Partial payment, previous crack
Jun NA	22 Jun	36	Lancaster	m	Stucco and Plaster	0	0	Previous cracks and patching, evidence of settlement. Will not file claim
1965-66 NA	22 Jun	ñ	Quartz Hill	<b>د</b>	Concrete irrigation pipe, splitting in longitudinal seam at beginning of irriga- tion period	0	1	information call, will not file claim

stigation	Poorly done project, old cement used for slab and mortar. Some fill under front end of reservoir washed out. Will not file claim	
Own-Results of Investigation Rent A/D Remarks	Poorly done proused for slab arundor front end out. Will not	
Own-	0	0
Type of Claim Explanation	Small concrete block reservoir- cracks	Glass-Window (3) Cracked mortar around tile
غ ا	۳v	<b>- ∿</b>
Location	Palmdale	ancaster
Complaint	43 Pa	47 61
Date Complaint Complaint Location Received Number	27 Jun	J Jul
urrence	<b>Y</b>	×
Time of Occ of Dama Date Time	Z V Z	<b>Z</b>

### a) Notation

- NA information not available
- Claim approved. Engineering investigator found damage could have been caused by sonic boom from test program flights.
  - Claim denied. Engineering investigator judged that damage was not caused by sonic boom from test program flights. **C** 
    - Owner reporting claim
- R Renter reporting claim
- Quantity in parentheses indicates number of damaged panes. b) Refer to Table 11-3 for classification of types of claims.

TABLE 11-5

DAMAGE INCIDENTS BY TYPE AND TEST PROGRAM PERIOD

(A complaint may involve several incidents and types of damage)

	Complaint Classification		PHASE	SE I						PHASE	)E 11
		June 66 -	3 June 66 - 12 June 66	13 June 66	13 June 66 - 23 June 66					31 Oct	31 Oct 66-17 Jan
		Track 0245° Mag.	15° Mag.	Track @	Track @ 233 <sup>0</sup> Mag.	Date Not	Date Not Available	Total		Track 6245°	8245° Mag
2	≨уре	Number	Percent	Number	Percent	Number	Percent	Number Percent Number	Percent	Number	Percent
	Glass-Window and/or Door	=	52.3	36	67.8	īU	55.6	52	62.8	ø	46.1
7	Glass - Miscellaneous	0	0	-	6.1	0	0		1.2	0	ō
M	; Plaster or Stucco-Cracks	ধ	0.61	m	5.7	<b>Carlon</b>		ω	9.6	2	15.4
4	Plaster or Stucco-Fallen	0	0	_	6.1	0	0	•••	1.2	0	0
2	Structural	0	0	M	5.7	m	33.3	9	7.2	0	Ċ
9	Fallen Object-Bric-A-Brac		4.8	_	6.1	0	0	2	2.4	<u></u>	7.7
7	Fallen Object-Miscellaneous	ĸ	14.3	r2	9.6	0	0	ω	9.6	4	30.8
00	Miscellaneous	-	4.8	-	6.1	0	O	2	2.4	Ø	0
6	Noise Complaint-No Damage	0	0	7	ສ.ຄ	0	0	7	2.4	0	O
2	10 Information Call-No Damage	-	4.8	0	0	0	0	-	1.2	င	0
	TOTAL	21	100.0	53	100.0	Ø	100.0	83	100.0	5	0.001
Ì											

DAMAGE INCIDENTS BY LOCATION AND TEST PROGRAM PERIOD

(A complaint may involve several incidents and types of damage)

		Æ	PHASE 1						FHASE 11	
Location	3 June 66 Track 6	3 June 66 - 12 June 66 13 June Track <b>6</b> 245 <sup>0</sup> Mag Trac	~ ~	66 - 23 June 66 Date Not Available e 233 <sup>9</sup> Mag	Dete Not	Available		Total	31 Oct 66 Track (12	31 Oct 66 - 17 Jan 67 Track (245° Mag.
	Number	Percent	Number	Percent	Number	Percent	Number	Percent Number	Number	Percent
Barstow	m	14.3	0	0	_	=	•	4.8	0	0
EAFB	7	33.3	7	13.2	_	=	2	18.0	m	23.1
Lake Hughes	0	0	-	6.1	0	0	-	1.2	0	0
Lake Isabella	0	0	٠٥	3.8	0	0	2	2.4	0	0
Lamont	0	0	0	0	0	0	0	0		7.7
Lancaster	9	28.6	15	28.2	Ŋ	55.6	56	31.4	4	30.7
Movaje	0	0	0	0	0	0	O	0	2	15.4
Palmdale	0	0	ĸ	5.7	<b></b>	=	4	4.8	0	0
Rosamond		4.8	7	ω κ)	0	0	m	3.6	٣	23.1
Quartz HIII	0	0	, o	17.0	_	=	<b>9</b>	12.0	0	0
Tehachapi	4	19.0	4	26.4	0	0	81	21.8	0	0
TOTALS	21	100.0	53	100.0	o,	0.001	83	100.0	13	100.0

TABULATION OF DAMAGE INCIDENTS VERSUS MISSIONS
AND MEASURED PEAK OVERPRESSURES
FOR PHASE I

the content of the second of the second seco

								1													1			
	Missi	Missions Flown	own a)		E S	Number of	of	č	ci de	ınts	of	incidents of b, c)	<b>.</b> .		<b>6</b>	P	2	: de	Valid Incidents of d)	oto	2		Range of Max	Range of Maximum Average
Date							ā	mag(	Damage by Type	, ,	90.					Ö	TRA CE	yd ,	Damage by type	3)			Positive Ov	Positive Overpressures
	XB-70	8-58	F-104	-	7	m	4	2	9	1	80	6	10 T	·	2	2	4	2	9	7	8	<u> </u>	@L-2(psf)	@E-2(psf)
4 June	_		-										°									0	NA <sup>e)</sup>	1.19-2.53
eunf 9	**			<u>o</u>		7				7			4	rU						_		9	0.57-0.94	1.64-3.45
7 June		2												_								_	0.64-1.47	0.98-3.42
8 June	-	2											7									_	0.11-1.13	1.65-3.35
9 June		4				_				_	_		M			-						_	0.39-1.66	1.55-4.04
13 June		œ	7	4				_					L-	4								2	0.97-1.50	1.83-2.97
14 June			9	7									M	7								7	0.42-1.21	1.33-2.84
15 June			හ										0									0	0.45	1.19-3.96
16 June			M										0									0	<b>V</b> Z	1.43-1.79
20 June		2		91									6	<u>.</u>								5	0.53-1.56	1.38-3.13
21 June		<u></u>		4		7				_	-	7	0	<u>_</u>	, -	7				_		9	0.48-1.54	1.17-3.23
22 June		o	80	N									7	N								7	0.14-2.00	1.00-3.76
23 June		7	7	4	-								ľΩ	M								M	0.30-2.13	1.26-5.81
Unknown <sup>†</sup>	<b>~</b>			6		-	ļ	m		2	_		1 17	5						-		9		
Torals	3	94	35	52	-	ω	-	9	2	ω	2	2	1 83	64	0	2	0	0	7	M	0 48	<u>@</u>		

## TABLE 11-7 (Continued)

a) From Mission Log, Edwards Phase I, Appendix D

b) See Table 11-3 for classification of types of complaints

Some compiaints were for more than one type of damage and/or more than one pane of glass. ô

Engineering investigator found damage could have been caused by sonic boom. <del>Q</del>

e) Not available.

f) Complaint not assignable to any one day.

SUMMARY OF COMPLAINTS ATTRIBUTED TO PHASE 11<sup>8)</sup>
(Dates are 1966 unless noted)

Time of Occurrence	Date							
of Damage	Complaint	Complaint Location	Location		Type of Claim <sup>b)</sup>	Own-		Results of Investigation
Date Time	Received	Number		<u>۸</u>	Explanation	Rent	<b>1</b>	Rent A/D Remerks
10 Nov NA	10 Nov	62 L	Lancaster	-	Glass-Window (!)	0	<	XB-70 8 miles south of designed track
16 Nov 1150	16 Nov	63	Mojave		Glass-Window (i)	œ	~	8-58 turning over Moisva
23 Nov 1035	25 Nov	64 L	Lancaster	7	Clock fell from wall	0	<	XB-70 1.3 miles north of residence
23 Nov 1004 & 1150	28 Nov	7 69	Lancaster	۳	Plaster-Ceiling	œ	۵	Not boom damage
l Dec 1040	- Dec	65 E	EAFB	~ ~	Clock-Kitchen Clock-Dining Room	œ	<b>∢</b> □	Not caused by program flights Not boom damage
1 Dec 0130-1515	- Dec	<b>8</b>	EAFB	9	Plate hung on wall	œ	<	Not caused by program flights
8 Dec 1230	8 Dec	67 R	Rosamond	-	Glass-Window (2)	0	۵	Extremely poor conditions 8-58 over Rosamond
8 Dec 1239	8 Dec	88	Rosamond	7	Suitcases feil off of shelf, striking washing machine and bird cage	0	۵	Not caused by program flight

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## TABLE 11-8 (Continued)

Results of Investigation	Rent A/D Remarks	A B-58 over Mojave	\ XB-70 turning over Lamont (Approx. 7 mi. south of Bakersfield)	Not boom damage. Water stains evident at cracks. Damage not related to any particulator boom
O.O.	ent		_	
Ó	æ	0	0	0
Type of Ciaim		(1) Wohulmessel8	Glass-Window (!)	Plaster
	<u>9</u>	-	_	n
Location		Mojave	Lamont	Lancaster
Date Complaint Complaint Location	Number	70	12 1	1 12
Date	Rece I ved	15 Dec	17 Jan 1967	3 Jan 1967
Time of Occurrence of Damage	Date Time	8 Dec 1200	17 Jan 1015-1020 1967	YN YN

# a) Notation NA information not available

- Claim approved. Engineering investigator found damage could have been caused by sonic boom from test program flights.
  - Claim denied. Engineering investigator judged that damage was not caused by sonic boom from test program flights.
- Owner reporting claim
- Renter reporting claim
- Refer to Table 11-3 for classification of types of claims. Quantity in parentheses indicates number of damaged panes. <u>م</u>

TABULATION OF DAMAGE INCIDENTS VERSUS MISSIONS
AND MEASURED PEAK OVERPRESSURES
FOR PHASE II

		i	(q			1	Nimbor Of	•	1 2	1 5	Incidents of c), d)	0,	ਉ			Valid incidents of e)	- P	10,0	lent	es.	fe)		Range of Maximum Average
(e)+00	MI SS I	Missions riown	i MO		5	5	5 <b>&amp;</b>	_	Damage by Type	-	/pe					۵	Damage by Type	ge t	۲ ×	уре	<b>-</b>		Positive Overpressures
<u>.</u>	XB-70	B-58	XB-70 B-58 F-104	1	2 3	m	1		9	7	8	6	2	5 6 7 8 9 10 T	-	12345678T	ы	<u>,</u>	ايّا	[ ]	0		@E-2 (psf)
	,	,	C	-										-	-							-	1.54-3.51
) )	7	۹ 5	ŧ	. <b>-</b>										_	_							_	2.22-370
NON G	r	2 °		-		-								~								_	2.41-3.27
(† £ 5.		4	•			•			-	8				M								2	1
) - 3		-		'n					•					4	-							-	1.36-3.19
2 c	c	<u>,</u> ,	•	– ۱										_	_							-	1.39-2.39
Unknown	4	1		•																		0	1
								I					-										
Totals	(y	28	4	Ò	0	6 0 2 0	0	1	-	4	0	0	0	<u>n</u>	0 1 4 0 0 0 13 4 0 0 0 0 1 2 0 7	0	0	。		_	[	_	

) Lays on which damage occurred.

) From Mission Log, Edwards Phase 1, Appendix D

c) See Table 11-3 for classification of types of complaints.

Some compleints were for more than one type of damage and/or more than one pane of glass. Ŧ

Engineering investigator found damage could have been caused by sonic boom. ê

f) No program filghts this day.

TABLE II-IO

SIZE DISTRIBUTION OF DAMAGED GLASS
IN STRUCTURES IN COMMUNITIES ADJACENT TO

EDWARDS AIR FORCE BASE<sup>a)</sup>

		Distri	bution		Total
Glass size in sq. ft.	0-2	2-9	9-40	Over 40	
Number of panes <sup>b)</sup>	-	<b>co</b>	-	-	605,000
Number of exposures, millions <sup>C)</sup>	-	-	-	-	69.0
Panes claimed damaged	1	19	15	4	39
Panes claimed damaged per million exposures	0.015	0.276	0.217	0.058	0.566
Panes possibly damaged by sonic boomsd)	1	15	12	4	32
Panes possibly damaged by sonic booms per million exposures <sup>d</sup> )	0.015	0.217	0.174	0.058	0.464

### Notes

- a) Lancaster, Palmdaie, Quartz Hill, Rosamond and Tehachapi
- b) 55,000 population times !! panes per person or total of 605,000 panes.
- c) Based on 114 supersonic missions (3 XB-70 and 94 B-58 during Phase 1 and 17 XB-70 during Phase 11).
- d) Engineering investigator found damage could have been caused by sonic boom.

TABLE 11-11

SIZE DISTRIBUTION OF DAMAGED GLASS
IN STRUCTURES AT EDWARDS AFB

	+ <del>7</del>	Dist	ribution		Total
Glass size in sq. ft.	0-2	2-9	9-40	Over 40	
Number of Panes from Surveys					
Base Housing	3,500	19,720	26.510	0	49,730
Base Operations Buildings	21,647	29,696	6,773	2,544	60,660
Total	25,147	49,416	33,283	2,544	110,390
Number of Exposures, Millions <sup>a)</sup>	-	-	-	-	39.4
Panes claimed damaged	3	i	·4	0	8
Panes claimed damaged per million exposures	0.076	0.025	0.102	0	0.203
Panes possibly damaged by sonic booms	0	1	4	0	5
Panes possibly damaged by sonic booms per million exposures <sup>b</sup>	0	0.025	0.102	0	0.127

### Notes

- a) Based on 357 supersonic missions (Phase I: YF-12, 2; SR-71, 3; XB-70, 3; B-58, 94, F-104, 35; F-106, 18; for a total of 155. Phase II: XB-70, 17; B-58, 69; F-104, 85; SR-71, 31; for a total of 202).
- b) Engineering investigator found damage could have been caused by sonic boom.

TABLE 11-12
COMPLAINTS OF GLASS DAWAGE THAT OCCURRED DURING PHASES 1 & 11

Complaint Number	Location	Size Width ft.	± ±	Årea Sq.ft.	G) ass	Frame	Orien- tz.íon	See 1+1on	instal- lation	Approval
FHASE 1										
_	<b>Teha</b> chapi	3.8	6.2	23.5	a.	ć	S	Cracked	Hor.Sliding	۵
8	Barstow	8.5	7.6	82.5	۵	¥	SE	boo <sub>0</sub>	Fixed	<b>≪</b>
M)	Lancaster	2.1	£.:	2.7	3 3	4 \ Z	4 ×	A/N	<b>4</b> /2	N/N/N/N/N/N/N/N/N/N/N/N/N/N/N/N/N/N/N/
•		2.1		2.7	*	ζ χ	ž	V / N	V /2	
<b>о</b>	Palmdale	 5.	4.0	6.0	3	<b>¥</b>	ဟ	Cracked	Fixed	≪
**		<u>-</u> r.	4.0	6.0	3	Α	υì	Good	Crankout	∢
0	Lancaster	3.0	3.3	6.6	**	¥	w	good Good	Hor.Silding	⋖
4	Tehachapí	3.6	4.5	16.2	*	poom	3	Good	Fixed	≪
16	Tehachapi	3.0	3.6	10.8	3	₹	z	Good	Fixed	æ
1.7	Tahachapi	2.5	2.5	6.3	*	Mood	ш	9009	Vert.Sliding	<b>⋖</b>
8	Tehachapi	۲.	2.5	3.2	3	pocy	ш	Cracked?	Fixed	۵
•		1.3	2.5	3.2	3	Mood	m	Cracked?	Fixed	٥
61	Tehachapi	3.0	3.0	0.6	3	F	3	Good	Fixed	⋖
2	Tehachapi	3.0	3.0	0.6	3	¥	3	роод	Fixed	⋖
21	Tehachap i	7.5	5.6	42.0	α.	¥	ш	poog	Fixed	⋖
22	Tehachani	9.2	8,3	62.5	۵	¥	ш	Good	Fixed	⋖
:		9.2	8.3	21.1	۵	ΑI	ш	goog	Fixed	K
		3.0	6.8	20.3	۵	¥	ш	boo <sub>0</sub>	Door	<b>⋖</b>
23	Tenachapi	3.8	6.3	23.9	۵.	Ā	S	Good	Fixed	∢

TABLE 11-12 (Continued)

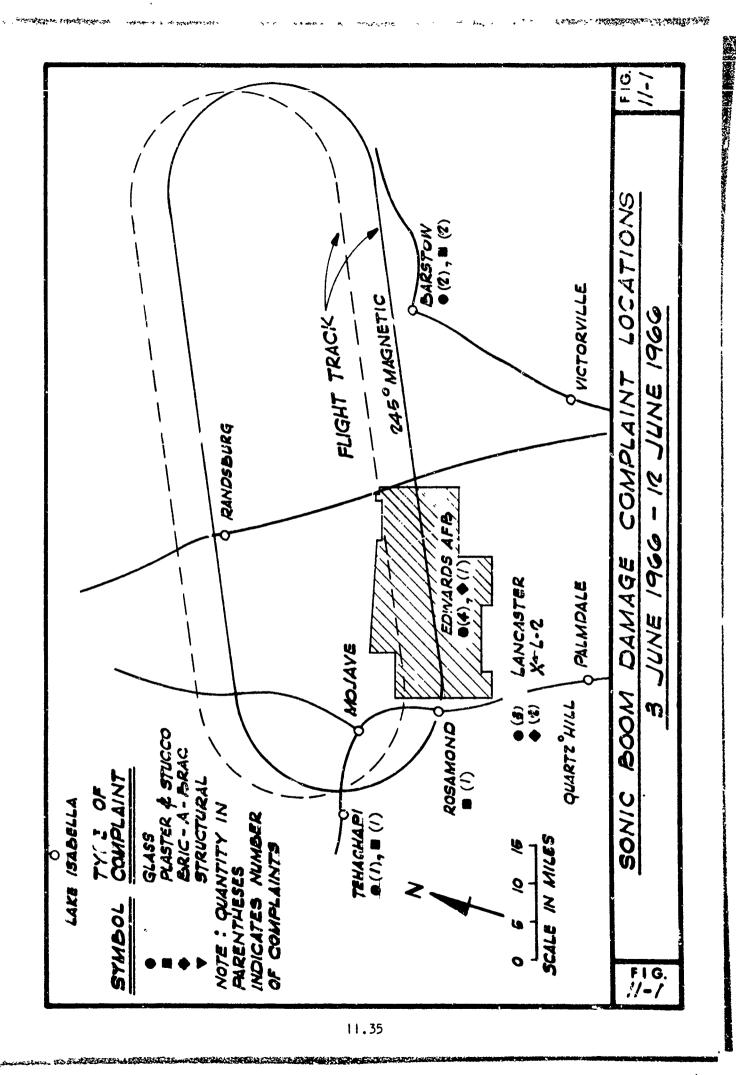
ALLEGE CONTRACTOR

•		Size								
Compleint Number	t Location	Width ft.	± ±	Area Sq. ff.	Glass	Frame	Orien- tation	Previous Condition	Instal- lation	Approval
PHASE !	PHASE (Continued)									
25	Lancaster	4.5	0.9	27.0	۵	ΑI	3	Cracked	Hor.Sliding	A, 758
56	Lancaster	<b>.</b>	8.0	32.0	Thred sayer	poo <sub>M</sub>	3	600d	Fixed	∢
		3.0	8.0	24.0		poo <del>x</del>	ш	Good	Fixed	∢
<b>%</b>	Quartz Hill	2.0	2.5	5.0	3	¥	3	роод	Hor.Sliding	∢
&	Quartz Kill	8.0	2.0	2.0	33	M W W	<b>2</b> 2	9009 9009	Vert.Sliding Fixed	∢ ∢
33	Lancaster	<u></u>	3.0	0.4 0.3.	<b>3</b> %	<b>4 4</b>	s s	роо <u></u>	Crankout Crankout	< <
35	Paimdale	9.0	7.0	63.0	۵.	¥	z	Cracked	Fixed	⋖
37	Quertz HIII	3.0	3.0	9.0	3	¥	ш	Good	Fixed	∢
58	Lake Isabella	2.0	3.8	7.6	3	Ä	ш	Good	Hor.Sliding	∢
42	Quartz 41111	2.0	2.2	4.4	* *	Wo sol	шω	900 900 900	Vert.Sliding Fixed	∢ ≪
94	Tehachap i	1.3	2.5	₩. 4	* *	poo <sub>M</sub>	ш ш	9009 9009	Vert.Sliding Vert.Sliding	<b>⋖</b> ⋖
47	Lancaster	2 2	4 W W	8 4 4 O N N	***	<b>Z X X</b>	ഗഗഗ	Good (?)	Crankout Crankout Crankout	< < <
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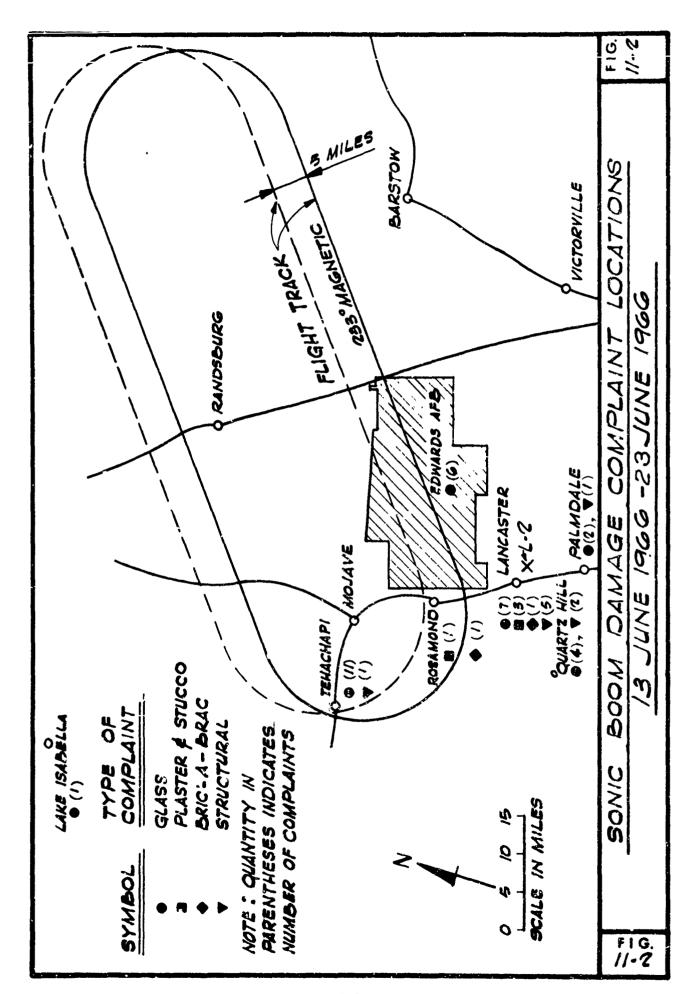
TABLE !1-12 (Continued)

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		Size								
Comp. sint	Location	Wldtt.	ŧŧ	Area Sq.ft.	Gl ass	Frame	Orien- tation	Previous Condition	Instal- lation	Approval
PHASE 1 (Continued)	ontinued)									
50	EAFB	2.9	ພ ພ ໝ ໝ	5.3	33		3 Z	X X X X X X X X X X X X X X X X X X X	F1xed N/A	< <
) 	EAFB	2.9	5. E.	0.0	* *	¥ ¥	m 3s	4 <b>4</b> /2 /2	Fixed Hor.Silding	< ≪
52	EAFB	2.9	3.8	0.1.	<b>3</b>	A	ш	N/A	Fixed	∢
55	ЕЛГВ	000	<u></u>	üüü	***	Steel Steel Steel	zzz	X X X 4 X X	Crankout Crankout Crankout	Z Z Z
58	Tehachapi	6.7	6.1	40.8	۵.	Mood	W	Good	Fixed	∢
PHASE 11										
62	Lancasiter	8.	3.8	6.9	*	¥	ш	роод	Fixed	∢
63	Mojave	8.	3.9	7.0	*	Ā	ш	Good	Crankout	∢
67	Rosamond	w w & &	6.3	23.9 23.9	۵.	<b>~ ~</b>	ဟ ဟ	Poor Poor	Hor.šliding Fixed	۵۵
02	Mojave	10.1	4.7	47.5	۵	¥	3	Good	F1xed	∢
72	-amont	8.	3.8	6,9	3	<u> </u>	S	<b>b</b> 005	Hor.sliding	∢



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# FIGURE 11-3

INVESTIGATOR'S SONIC BOOM DAMAGE REPORT (Write legibly or type)						
1. Name of claimant(s)	2. Address	3. Claim or complaint number				
		4. Date property inspected				
5. Contact		6. Location of damaged property:				
following phone number:						
c. Speed of aircraft:	boom complaints?	if so, give details:				
d. If claimant is a corporation, has the claim form been properly executed and does the file contain an authority to file claims (if SF 95 not signed by an officer of the corporation)?  e. If claim is in excess of \$1,000 and involves realty, does file contain a statement of title showing that the Gaimant owns the property?  f. Is this damage covered by insurance?						
i. In glass damage cases, does the estimate or bill take into consideration salvage value and discount sates?						
b. If multiple booms, are any identified	by time and date?	stach statements:				
d. When was damage first discovered?  e. Were windows and doors open or clos f. Did the person who experienced the sffected? How?  g. List names and addresses of anyone e	boom report that mobile object					

AFLC FORM 666

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# FIGURE 11-3 (Continued)

10. NAMES AND ADDRESSES OF PERSONS INTERVIEWED: (attach statements)				
	······································			
18. Hes dessage been repaired?	ng and how determined:			
13. Type of property damaged and general description of the damage:				
14. DETAILS PERTAINING TO SURROUNDING COMMUNITY:  8. Type of terrain:				
b. Type of surrounding community:				
**************************************	***************************************			
<u> </u>				
15. DATA PERTAINING TO EXTERIOR OF PROPERTY:				
a. Type of construction and size of building:	***************************************			
h. Type of recting and condition:	***************************************			
c. Type of foundation and conditions:	•			
d. Condition of sidewalks:	·			
14 DATA BETAINING TO INTEDIOR OF PROPERTY.				
16. DATA PERTAINING TO INTERIOR OF PROPERTY:  a. Was say settlement noted?				
b. Type of construction of walls and ceilings (wood lath and pleater, wallhoard, tile or or	ber):			
c. Condition of plaster, wallboard, or tile: (Is damage old or recent?)	***************************************			
d Are walls and ceilings papered, painted or tiled?				
e. Location and type of cracks (identify on photos and draw diagram of damaged area on	, 			
<del></del>	***************************************			
	***************************************			
f. When was dusage area last decorated and what was the extent of redecoration?	**************************************			
	**************************************			

ting in the company of the company o

# FIGURE 11-3 (Continued)

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17 Describe sense	al condition of the property:
Dement Better	
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***************************************	
18. Type of glass	damage, if any (identify on photos and draw diagrams on AFLC Form 669). Describe type of glass, dimensions
	kness, extent and type of preexisting damage:
mending the	thesis, extent and type of preexisting damage:
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10 PINIDINGS A	ND RECOMMENDATIONS:
	Deny:
(In your discu	ssion be specific as to when the damage occurred in relation to the time when the sonic boom occurred. Set out
your opinion	ssion be specific as to when the damage occurred in relation to the time when the sonic boom occurred. Set out as to the cause of the damage. If the damage was partially caused by sonic boom set forth percentage you believe sonic booms and your rationale for arriving at this conclusion. Use a continuation sheet if necessary):
attributable to	sonic booms and your rationale for arriving at this conclusion. Use a continuation sheet if necessary):
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20. Any other co	mmens:
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Date	Typed Name, Rank, Title, and Organization of Inspectors: Signature
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# FIGURE 11-4

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# SONIC BOOM - GLASS DIAGRAMS

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# SONIC BOOM - INTERIOR DIAGRAMS

ROOM				
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CEILING PLAN 3	1	1	S WALL	<b>)</b>
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CEILING PLAN S	<b>E</b>	Ε		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\

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GLOSSARY OF TERMS

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## GLOSSARY OF TERMS

- A peak deflection
- A: digitized acceleration values
- b height of plate (window)
- C. coefficients obtained from Simpson's rule
- C coefficient of variation
- c damping coefficient
- D plate stiffness =  $\frac{Et^3}{1-v^2}$
- DAF dynamic amplification factor
- E elastic modulus
- F,f forcing function
- | moment of inertia
- K stiffness
- L length of beam or plate
- M,m mass
- m subscript referring to m<sup>th</sup> normal mode
- n sample size
- n subscript referring to n<sup>th</sup> normal mode
- P total load . an element
- P() probability
- P atmospheric pressure
- P<sub>i</sub> inside pressure
- $P_n$  net pressure  $(P_n = P_0 P_1)$
- P outside pressure
- P peak positive overpressure
- Pa peak negative pressure
- PF participation factor

 $p - 1\omega^2 - \beta^2$ 

q - distributed load on an element

r - subscript referring to rth mass

S - sample standard deviation

s<sup>2</sup> - sample variance

T - natural period

 $T_1$  - rise time (time from start of boom to pea \_ositive overpressure)

T<sub>2</sub> - time from start of boom to peak negative pressure

t - plate thickness

t - time

V - enclosed volume

w - displacement (perpendicular to surface of window plate), see also  $\Delta$ 

x - coordinate (of point on window plate)

x - displacement (see also  $\Delta$ )

\* - velocity

🛱 - acceleration

x; - the i<sup>th</sup> sample

ス - sample mean

β - percent of critical damping

Δ - displacement

ε - strain

v - Poisson's ratio

τ - time from start to end of boom

→ deflected shape

w - natural circular frequency

# APPENDIX A

STRUCTURAL AND STATISTICAL PRINCIPLES

#### APPEND:X A

#### STRUCTURAL AND STATISTICAL PRINCIPLES

The structural and statistical principles used in the analyses of structural response data and free field signature data in this report are reviewed in this Appendix. Equations are derived where deemed desirable. References are given as required.

#### STRUCTURAL PRINCIPLES

The primary purpose of the structural analyses presented in this report was to compare measured response with predicted response. Measured response was determined from Instrumented test structures and predicted response was calculated using structural principles presented herein. To evaluate the predicted response of a structure or structure element to a sonic boom dynamic loading, the properties of idealized structures were represented by mathematical models. In the formulation of these models, it was assumed that the structure remained elastic and that the damping losses were proportional to the relative velocity between masses or to the velocity of each mass relative to the ground. The idealized structure, which was a multidegree of freedom system, was then modeled by independent single-degree of freedom systems. Each normal mode of vibration of the multi-degree of freedom system was treated as an i. ...pendent single degree of freedom system. The single-degree of freedom system was related to the multi-degree of freedom system by the modal participation factor. There were as many natural frequencies as there were elements of the multi-degree of freedom system and each mode of vibration was excited by the disturbing force to a degree determined by the element's modal participation factor.

The following is a brief derivation of the equations for a single-degree of freedom system and an explanation of how the single-degree of freedom system is related to the multi-degree of freedom system.

#### Single Degree of Freedom System

The equilibrium equation of a single degree of freedom damped system subjected to an arbitrary forcing function, f(t), is:

$$m\ddot{x} + c\dot{x} + kx = f(t) \qquad (A-1)$$

where:

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m = mass

c = damping coefficient

k = stiffness

 $\ddot{x}$  = acceleration

\$ = velocity

x = displacement

Dividing Equation (A-I) by m, letting  $\frac{c}{m}=2\beta$ , and  $\frac{k}{m}=\omega^2$ , Equation (A-I) becomes:

$$\ddot{x} + 2\beta \dot{x} + \omega^2 x = \frac{f(+)}{m}$$
 (A-2)

It should be noted that when  $\omega=\beta$ , the motion is critically damped; that is, the motion loses its vibratory character. The value of the damping coefficient, c, is then  $\frac{c}{2m}=\sqrt{\frac{k}{m}}$ . Thus  $c=2\sqrt{km}$ .

When a system is 2% critically damped the damping coefficient, c, is equal to:

$$c = 0.04 \sqrt{km}$$

If it is assumed in the solution of Equatic 'A-2) that P is the peak value of the disturbing force, the term  $P/m\omega^2$  or note  $m\omega^2 = k$  the term P/k can be factored out of the solution to obtain:

$$x(+) = \frac{P}{k} \cdot DAF \qquad (A-3)$$

where DAF is the Dynamic Amplification Factor and P/k is the static deflection of a structure element subjected to a load P. The DAF therefore represents the influence of a dynamic load or the change of the magnitude of the load with time.

The magnitude of the DAF for a given structural element also depends upon the element's natural frequency, stiffness, and damping. Natural frequency is the number of vibrations per second resulting when an element is displaced from its at rest position and released; stiffness is an element's resistance to displacement or rotation; damping is the measure of a system's capacity, to absorb energy and cause vibrations produced by a disturbance such as a sonic boom to diminish as the time after the disturbance increases.

An example of a DAF spectrum has been plotted in Figure A-I for a typical N-type pressure wave. In this illustration, the DAF has been plotted on its

vertical scale and the natural frequencies, in cycles per second, of structure elements have been plotted on the horizontal scale. A graph of this type can be used in the following manner. Suppose that this graph represented the DAF spectrum for a supersonic aircraft mission and it is desired to compute the maximum deflection of a wall panel for a sonic boom peak dynamic pressure of, say 2 psf. The natural frequency of the wall panel could be determined by calculation or measurement. Assume that the value obtained was 20 cps. Then the value of the DAF can be scaled from the graph and is equal to 1.75. The stiffness, k, of the element could also be determined by calculation or measurement, and assume that the value obtained was k = 200 psf/in (which includes a correction to account for the fact that the wall is a distributed mass). Then the dynamic displacement would be:

$$\Delta$$
 dynamic =  $\frac{P}{k} \times DAF = \frac{(2 \text{ psf})}{(200 \text{ psf/in})} \times 1.75$ 

 $\Delta$  dynamic = 0.0175 in.

# Multi-Degree of Freedom System

To obtain the response of a multi-degree of freedom system, where each normal mode is treated as an independent one-degree of freedom damped system, the modal equation is derived  $^{4,5}$  by energy methods:

$$\ddot{x}_{n} + 2\beta_{n}\dot{x}_{n} + \omega_{n}^{2} x_{n} = \frac{\Sigma F_{r} \phi_{rn}}{\Sigma M_{r} \phi_{rn}^{2}}$$
 (A-4)

where the subscript r represents a lumped mass, subscript n a -mal mode,  $F_r$  a forcing function on mass r,  $M_r$  a mass, and  $\phi_{rn}$  the def -shape of mass r in the nth mode. Solving Equation (A-4) we obtain:

$$x_{rn}(t) = \frac{P(max)}{2} \cdot (DAF)_{n} \cdot (PF)_{rn}$$
 (A-5)

where  $(PF)_{rn}$  is the participation factor of mass r in the nth mode. Equation (2-6) can be rewritten thus:

$$x_{rn}(t) = \frac{P(max)}{m_r \omega_{rn}} (DAF)_n$$
 (A-6)

where:

$$\omega_{rn}^2 = \frac{\omega_n^2}{(PF)_{rn}}$$

The concept of DAF is widely used and whether the forcing function is a

load, an acceleration, a velocity or a displacement, it has been called Dynamic Load Factor  $^{6,5}$ , Load Response Spectrum  $^{7}$ , Magnification Factor  $^{8}$ , Dynamic Magnification Factor  $^{9}$ , acceleration, velocity and displacement Response Spectrum  $^{10,4}$  and Shock Spectrum  $^{11,12}$ .

When the forcing function is given in digital form, the response of a single-degree of freedom damped system is  $^{13}$ :

$$x(t) = \frac{P}{k} \cdot p \cdot \frac{\Delta t}{3} \cdot \frac{i}{5} \cdot \frac{C}{J = 0} A_{j} e^{-\beta p \cdot (1-J)\Delta t} sinp(1-J)\Delta t \qquad (A-7)$$

where:

$$P = \sqrt{\omega^2 - \beta^2}$$

$$A_j = \text{digi'ized input value, normalized with respect to P}$$

$$C_j = \text{coefficients obtained from Simpson's Rule.}$$

#### STATISTICAL PRINCIPLES

Statistical methods were used to estimate the true value of observations resulting from the same phenomena, to compare various parameters, and to provide a degree of confidence in the conclusions made from analyses of data.

In order to estimate the true value of observations resulting from the same phenomena, the mean, the variance, the standard deviation and the coefficient of variation were computed.

The sample mean (average)  $\bar{x}$ , is a measure of central tendency corresponding to the center of gravity in a mechanical system. It is mathematically expressed as:

$$\overline{x} = \frac{\Sigma x_1}{n} \tag{A-8}$$

where  $x_i$  is the  $i^{th}$  observation in a sample of size n.

The sample variance, s<sup>2</sup>, is a measure of the spread or dispersion of the observations about the mean and corresponds to the moment of inertia in a mechanical system. The mathematical expression of the sample variance is:

$$S^2 = \frac{\Sigma(x_1 - \overline{x})^2}{n - 1}$$
 (A-9)

The sample standard deviation is defined as the square root of the variance and corresponds to the radius of gyration measured from the center of gravity in a mechanical system. The sample mean and the sample standard deviation

ation have the same units as the observations.

Another measure of the spread of the data is the sample coefficient of variation. This dimensionless quantity is defined as the ratio of the sample standard deviation to the sample mean:

$$C_{V} = \frac{s}{x} \tag{A-10}$$

Statistical tests were used to verify each hypothesis which could be rejected or accepted. If it was rejected an alternate hypothesis was accepted. There are, however, two types of errors in hypothesis testing: the rejection of a true hypothesis, type I error, and the acceptance of a false hypothesis, type II error. The probability of committing an error of type I is denoted by  $\alpha$  and is called the level of significance of the test; the probability of committing an error of type II is denoted by  $\beta$  and is equal to one minus the power of the test. The probability of accepting a true hypothesis, (I- $\alpha$ ), can be called the confidence level of the test. More generally, the confidence level can be defined as the probability that the conclusion is correct.

In order to test the hypothesis that the mean of a normally distributed random variable was equal to a specified value,  $\mu_{0}$ , against the alternate hypothesis that the mean was greater than  $\mu_{0}$ , the following t statistic was used <sup>14</sup>,15:

$$t = \frac{(\bar{x} - \mu_0)\sqrt{n}}{S}$$
 (A-11)

If  $\dot{} \leq \dot{}_{\alpha;\,n-1}$ , the hypothesis of equality was accepted. If  $\dot{} > \dot{}_{\alpha;\,n-1}$ , the hypothesis of equality was rejected and the alternate hypothesis,  $\ddot{x} > \mu_{_{\scriptsize O}}$ , accepted.

In order to test the hypothesis that the means of two normally distributed random variables were equal against the alternate hypothesis that one of the means was greater than the other, the following t statistic was used:

$$t = \frac{\bar{x}_1 - \bar{x}_2}{\sqrt{\frac{s_1^2}{n_1} + \frac{s_2^2}{n_2}}}$$
 (A-12)

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If  $t \le t_{\alpha;\nu}$ , the hypothesis of equality was accepted. If  $t > t_{\alpha;\nu}$ , the hypothesis of equality was rejected and the alternative hypothesis,  $\bar{x}_1 > \bar{x}_2$ , was accepted.

It was often required to express the relationship between two variables in order to predict the outcome of one by knowing the other. The general equation expressing the relationship was given by a non-linear regression model <sup>17</sup>:

$$f(y) = a + b_1 (g_1(x) - \overline{g_1(x)}) + \dots + b_n (g_n(x) + \overline{g_n(x)})$$
 (A-13)

where a, b<sub>1</sub>, b<sub>2</sub>, . . . b<sub>n</sub> were constants calculated from observations and f(y) was any function of y such as y,  $\frac{1}{y}$ , log y, y<sup>n</sup>, g<sub>1</sub>(x), g<sub>2</sub>(x), g<sub>n</sub>(x) were any function of x such as  $\frac{1}{x}$ , log x, x<sup>n</sup>. The more basic model was the linear regression:

$$y = a + b_1 (x - \bar{x})$$
 (A-14)

Another model was the second order polynominal:

$$y = a + b_1 (x - \bar{x}) + b_2 (x^2 - \bar{x}^2)$$
 (A-15)

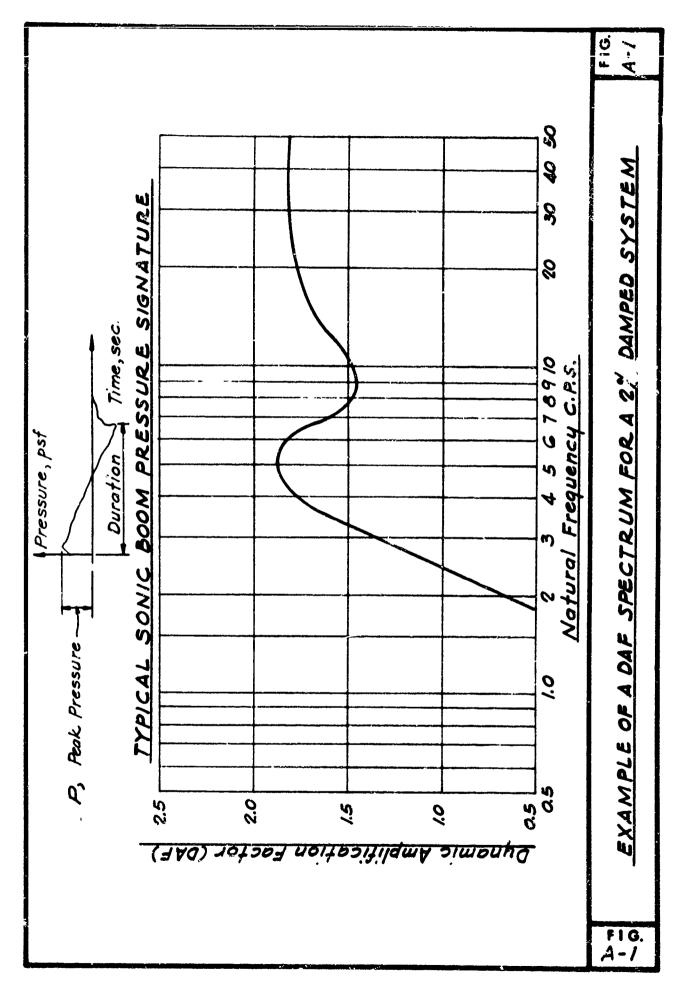
Once the model was fitted through the data it was possible to calculate the standard deviation of y for a given value of x. For the linear model:

$$S_y = \left[S^2 \left(\frac{1}{n} + \frac{(x - \bar{x})^2}{\Sigma(x_1 - \bar{x})^2}\right)^{\frac{1}{2}}\right]$$
 (A-16)

$$S^{2} = \frac{\sum (x_{1} - \overline{x})^{2} - \left[\sum (x_{1} - \overline{x})(y_{1} - \overline{y})\right]^{2}}{\sum (x_{1} - \overline{x})^{2}}$$

$$(A-17)$$

The predicted value was obtained from the regression model, as for the linear model given in Equation (A-14). A prediction was usually accompanied by a range. In the regression analysis the range was given as a confidence interval, or the probability that a future observation would lie within the upper and lower values given. A confidence interval of 95%, for example, meant that there was a 95% probability that any observation lies within the upper and lower value.



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# APPENDIX B

INSTRUMENTATION LOCATIONS, SYSTEMS, AND FREQUENCY RESPONSE

<u>FOR</u>

TEST STRUCTURES E-1, E-2 AND E-3 AND
FREE FIELD MICROPHONES (CRUCIFORM ARRAY)

# APPENDIX E

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# INSTRUMENTATION LOCATIONS, SYSTEMS, AND FREQUENCY RESPONSE FOR TEST STRUCTURES E-1, E-2, AND E-3 AND FREE FIELD MICROPHONES (CRUCIFORM ARRAY)

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# **LEGEND**

Symbol	<u>                                      </u>
MA	Acoustic Microphone (20 - 10,000 cps)
M	Pressure Microphone (0.1 - 10,000 cps)
ML	Pressure Microphone (0.1 - 10,000 cps)
MLC	Pressure Microphone (0.02 - 10,000 cps)
A	Low Frequency Accelerometer (dc - 500 cps)
A P	High Frequency Accelerometer (100 - 2000 cps)
SG	Strain Gage (2000 cps)
\$	Strain Gage (2000 cps)
D	Displacement (5 - 100 cps)
TR	Tape Recorder
BR	Bedroom
FR	Family Room
KIT	Kitchen
LR	Living Room

TABLE B-I

# INSTRUMENTATION LOCATION - STRUCTURE E-I

(See Fig. B-I & B-2)

Transducer	Tape Recorder	Channe !	Location
MAI	TR-1	101	In center of LR suspended 6 feet from floor.
MA2	TR-I	102	In center of FR-KIT area suspended 6 feet from floor
MA3	TR-I	103	Center BRI suspended 6 feet from floor.
MA4	TR-I	104	BRI movable.
MA5	TR-I	105	FR-KIT area, movable.
MA7	TR-I	113	Outside subject group
Al	TR-3	304	On concrete block in LR.
A2	TR-3	305	On concrete block FR-KIT area.
A3	TR-I	106	On concrete block BRI (vertical)
<b>A</b> 5	TR-2	201	At top plate on E wall at NE corner (East-West (acceleration).
<b>A</b> 6	TR-2	203	At top plate on N wall at NE corner (North-South (acceleration).
All	TR-2	202	BRI E wall, mid-height center stud (horizontal).
MLI	TR-8	803	Outside N wall above plate.
ML2	TR-8	804	Outside E wall.
ML3	TR-2	204	BRI next to All.
ML4	TR-2	205	Center ceiling attic side above FR-KIT area.
ML5	TR-8	805	Outside W wall of garage at plate line.
ML6	TR-8	806	Outside S wall above plate line, center.
SG3	TR-2	207	Center big window (garage) (axis horizontal).
MA8	TR-2	209	Trigger mike in field.

Refer to Legend, Page B.I, for explanation of notation and abbreviation.

TABLE B-2

# INSTRUMENTATION LOCATION - STRUCTURE E-2\*

(See Figs. B-3 through B-5)

	Таре		
Transducer	Recorder	<u>Channel</u>	Location
MAI	TR-I	107	Between LR and DR 6 feet above floor.
MA2	TR-I	108	Over center in KIT 6 feet above floor.
MA3	TR-1	109	Center of BRI 6 feet above floor
MA4	TR-I	110	Center of FR 6 feet up.
MA5	TR-I	111	Movable FR-KIT-DR.
MA6	TR-1	112	Movable FR-KIT-DR.
AIP	TR-3	306	On concrete block FR.
A2P	TR-3	307	Movable FR-KIT-DR area. (Dinette window 10/21)
A5P	TR-3	308	Movable FR-KIT-DR area. (Pantry louver door 10/31)
A6P	TR-3	<b>3</b> 09	Movable FR-KIT-DR area. (Cabinet door 10/31)
A9P	TR-3	310	On concrete block BRi. (N-S Direction) - Movable.
AIOP	TR-3	311	Movable FR-KIT-DR area. (Side of stove 10/31)
ALIP	TR-3	312	Movable FR-KIT-DR area. (Dining room window 10/31)
A12P	TR-3	313	On concrete block BRI. (E-W direction) - Movable.
Al	TR-3	<b>30</b> 1	On concrete block DR.
A2	TR-3	302	On concrete block FR.
A5	TR-4	401	On exterior at roof plate line on N side of NE corner.
<b>A6</b>	TR-4	403	On exterior at roof plate line on E side of NE corner.
A7	TR-4	405	On exterior at second floor p'ate line on N side of NE corr.
A8	TR-4	407	On exterior at second floor plate line on E side of NE cnr.
A9	TR-4	402	On bottom chord of roof truss approx. over center of BRI.
All	TR-4	404	On center stud at mid-height on E wall of DR.
A12	TR-4	406	On center stud at mid-height on N wall of BRI.
SG4 I	TR-2	206	Located on large plate glass window garage entrance.
SG42	TR-2	208	Located on large plate glass window garage entrance.
SG43	TR-2	210	Located on large plate glass window garage entrance.
SG44	TR-2	212	Located on large plate glass window garage entrance.
DI	TR-4	411	Adjacent to A5 with same axis.
D2	TR-4	412	Adjacent to A6 with same axis.
ML2	TR-4	408	Suspended between LR and DR adjacent to MAI.
ML3	TR-4	409	Located in attic above BRI.
ML4	TR-4	410	Suspended below ceiling center BRI.
ML i 1	TR-8	811	Outside E wall middle of second story.
ML12	TR-8	812	Outside E wall middle of first story, outside of DR.
ML13	TR-8	810	Outside on W wall above garage roof.
ML14	TR-8	809	Outside W garage wall above plate line.
ML15	rR-8	801	Center of roof N side.
ML16	TR-8	802	Center of high roof's side.
ML17	TR-8	807	Outside N wall middle of second story.
ML18	TR-8	808	Outside S wall mid-second story, midway between porch roof and eave line.

Refer to Legend, Page B.I, for explanation of notation and abbreviation.

TABLE B-3

INSTRUMENTATION LOCATION - BOWLING ALLEY E-3

(See Fig. B-6)

Transducer	Tape Recorder	<u>Channe I</u>	Location
Alh	TR-5	501	Top of stee! column (interior of building) East-West racking acceleration.
A2H	TR-5	502	Top of steel column (south side) East-West racking accel.
A3H	TR-5	503	Top of steel column (south side) North-South racking accel.
A4H	TR-5	504	Top of steel column (west side) North-South racking accel.
A5V	TR-5	505	Center of roof girder, vertical acceleration of girder.
M2	TR-5	512	Interior - 3 feet below roof.
M4	TR-5	513	Exterior - above roof.
SIL	TR-5	507	Strain gage on bottom flange of roof girder at centerline.
S2L	TR-5	508	Strain gage on bottom flange of roof girder at 1/4 point.
S3L	TR-5	509	Strain gage on bottom flange of purlin at centerline.

# TABLE B-4

# INSTRUMENTATION LOCATION - FREE FIELD MICROPHONES

# (CRUCIFORM ARRAY)\*

(See Fig. B-7)

Transducer	Tape Recorder	Channel	Location
MLCI	TR-6	601	East corner cruciform array.
MLC2	TR-6	603	North corner cruciform array.
MLC3	TR-6	605	West corner cruciform array.
MLC4	TR-6	607	South corner cruciform array.
MLC5	TR-6	609	Bottom of mast, center cruciform array.
MLC6	TR-6	611	Top of mast, center cruciform array.

<sup>\*</sup>Refer to Legend, Page B.I, for explanation of notation and abbreviation.

## TABLE B-5

#### INSTRUMENTATION SYSTEMS

The measuring systems involved the following equipment:

#### TAPE RECORDERS TR-1 through TR-5

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# MA-System (Acoustic Microphone)

Microphones - B & K Model 4134 Power Supplies - B & K Model 2801 Amplifiers - Burr-Brown Model 9860

### Low Frequency System

Accelerometers - Kistler Model 303MIO Control Panels - NASA built

## ML-System (Pressure Microphones)

Microphones - Photocon Model 464 Signal Conditioning - Photocon Dynagage Model DG-605 Amplifiers - Burr-Brown Model 9860

# Strain Gage System

Strain Gage - Micro-Systems Type PAI-16-350 Amplifier Box - NASA built, using Fairchild ADF-1 Amplifier

## High Frequency System

Accelerometers - Endevco Model 2219E Amplifiers - Glennite Model KA-1006

#### Velocity System (Displacement System)

Velocity Transducer - MB Model MB-124 Amplifier - CEC System D Power Supply - CEC Model 2-105A

Tape Recorders - CEC VR-3300 (30 ips)
Direct Write Oscillographs - CEC Model 5-124
Galvanometer Driver Amplifiers - CEC Model 163
Oscilloscopes - Hewlett-Packard Model 140A
Squelch circuits and selector modules as designed and fabricated by NASA Langley Research Center.

- AND STREET, 
# TABLE B-5 (Continued)

#### TAPE RECORDER TR-6

# ML-System

Microphones - Photocon PRP-464-15D (Modified by partly plugging vent hole to extend low frequency response)

Signal Conditioning - Photocon DG-605D Dynagage

Amplifier - Burr-Brown Model 9077A

Tape Recorder - CEC VR-3300 (30 ips)

# TAPE RECORDER TR-8

#### ML-System

Microphones - Altec 21BR-150 Signal Conditioning - Photocon 600D Dynagage Amplifier - Ampex 15770-1 Tape Recorder - Ampex CP100 Direct Write Oscillographs - CEC 5-124

TABLE 8-6

TRANSDUCER FREQUENCY RESPONSE AND ACCURACY

Transducer	Frequency Response	Accuracy
Acoustic Microphones (MA)	20-10,000 cps	<u>+</u> 2.1 db
Pressure Microphones (ML) (TR-2, 4, 5, and 8)	0.1-10,000 cps	<u>+</u> 2.1 db
Pressure Microphones (MLC) (TR-6)	0.02-10,000 cps	<u> </u>
Low Frequency Accelerometers (A)	dc-500 cps	<u>+</u> 5%
High Fr ruency Accelerometers (A P)	100-2000 cps	<u>÷</u> 12%
Strain Gages (SG, S)	2000 cps	<u>+</u> 2%
Displacemen; (D)	5-100 cps	<u>+</u> 2%

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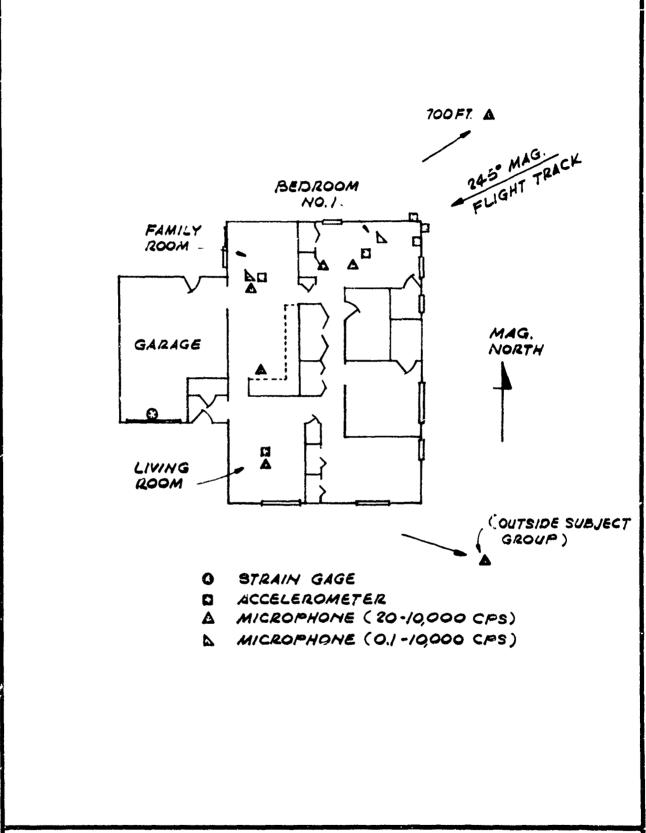
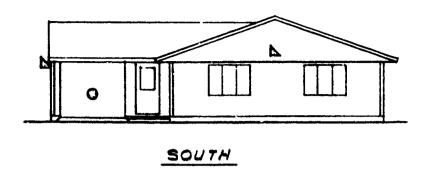
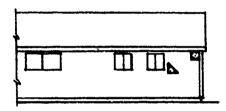


FIG. B-I INSTRUMENTATION LOCATION,
STRUCTURE E-I FLOOR PLAN







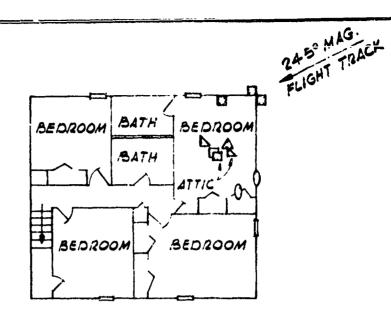
PART EAST

PART HORTH

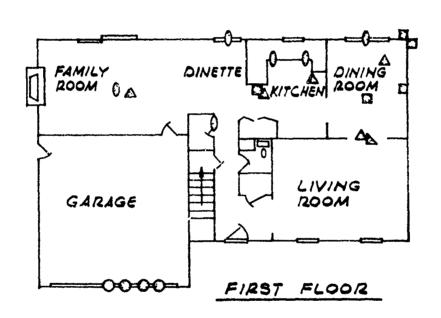
# ELEVATIONS

- O STRAIN GAGE
- ACCELERAOMETER
- ► MICROPHONE (0.1 10,000 CPS)

FIG. B-2 INSTRUMENTATION LOCATION,
STRUCTURE E-1 ELEVATION



# SECOND FLOOR



- O STRAIN GAGE
- D ACCELEROMETER
- A MICROPHONE (20-10,000 CPS)
- MICROPHONE (0.1-10,000 CPS)
- 1) HF ACCELEROMETER

MAG. NORTH

FIG. B-3 INSTRUMENTATION LOCATION,
STRUCTURE E-2 FLOOR PLAN

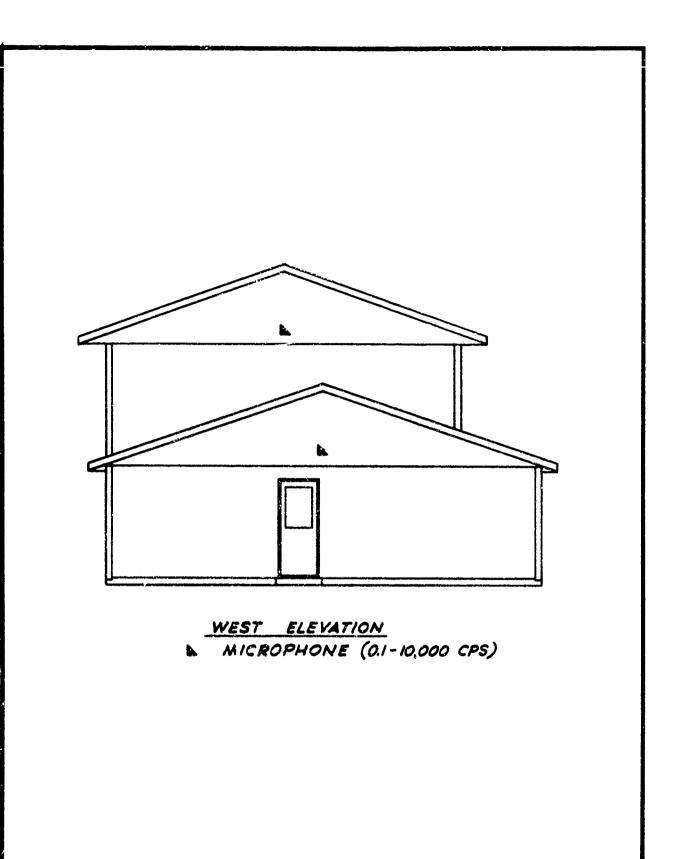
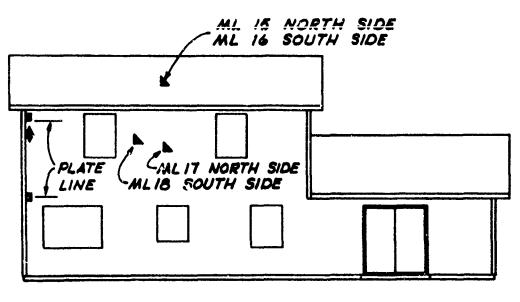
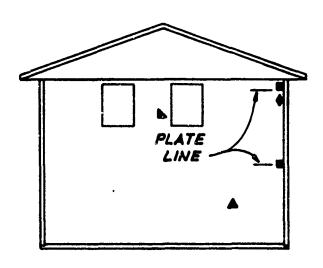


FIG. 8-4 INSTRUMENTATION LOCATION
STRUCTURE E-2 ELEVATION



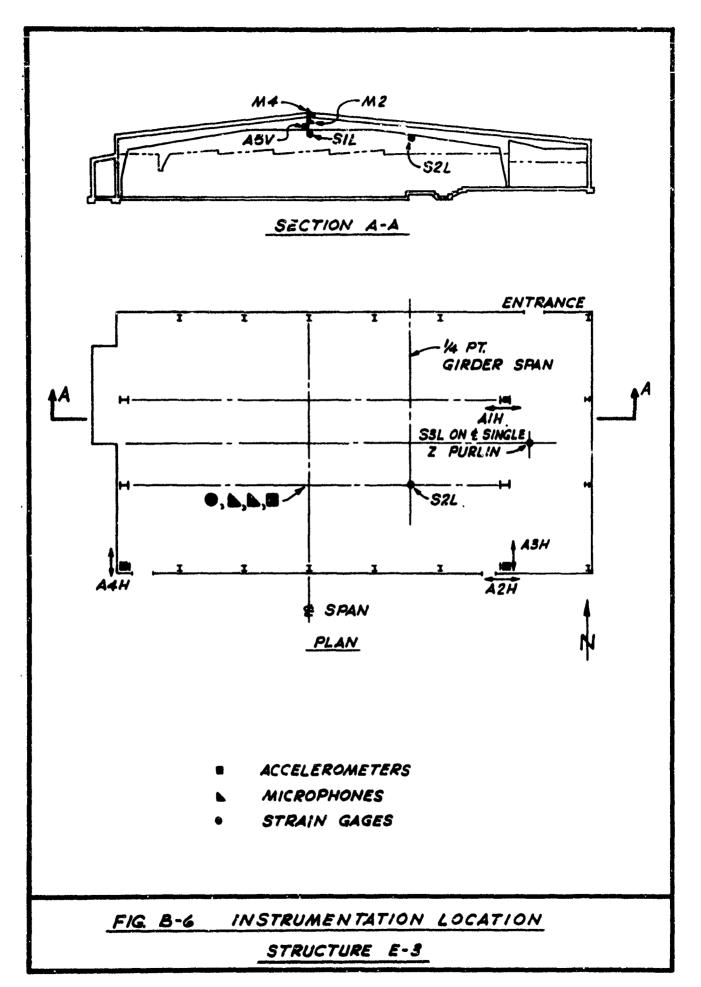
# NORTH ELEVATION



# EAST ELEVATION

- ACCELEROMETER
- ► MICROPHONE (0.1-10,000 CPS)
- DISPLACEMENT GAGE

FIG. 8-5 INSTRUMENTATION LOCATION
STRUCTURE E-2 ELEVATION



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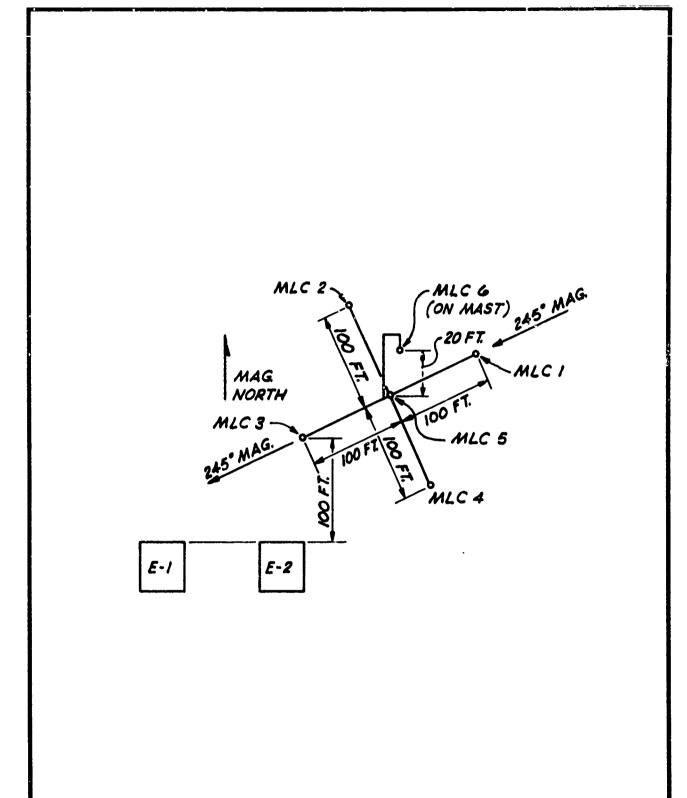


FIG. 8-7 INSTRUMENTATION LOCATION
FREE FIELD MICROPHONES
(CRUCIFORM ARRAY)

# APPENDIX C

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INSTRUMENT CALIBRATION PROCEDURES

## C. INSTRUMENT CALIBRATION PROCEDURES

The following general procedures were followed for calibrating instrumentation installed in E-1, E-2 and E-3:

1. All equipment was left in the "Power On" condition, except tape recorders which were turned off over weekends only.

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- 2. All instrumentation channels were calibrated prior to and immediately after each day's run. Calibration commenced at 0600 on run days.
- Use of voice annotations was held to a minimum to maintain IRIG timing on the tapes.
- 4. On each run day, personnel were informed, prior to calibrating, of values to set on the various channels. Variations in gain settings were recorded on the log sheet for the particular mission.
- 5. All pertinent data, including unusual conditions or events, were recorded on the appropriate data sheets.

## TAPE RECORDERS TR-1, 2, 3, 4, 5 AND 6

# Photocon Microphone Calibration

1. Tune Dynagage.

- 2. Set Dynagage at attenuation of "18".
- 3. Set Burr Brown Amplifier at 18 dB.
- 4. Balance Dynagage for "Zero Output".
- 5. Install the proper adaptor on the driver unit of the Model PC-125 calibrator.
- 6. Check the battery condition of the PC-125 by turning the function control to "Bat. Check". If the meter reads below the line marked "Bat. Check", recharge the batteries for a minimum of 12 hours. If the meter reads above the "Bat. Check" line, proceed as follows:
- 7. Set the "DB SPL" control to 120 dB, turn the function control to "Operate" and adjust the "SPL ADJ" control until the "SPL" meter reads 0 dB.
- 8. Adjust Burr Brown amplifier gain to obtain a "2vPP" signal at tape recorder input for SPL of 120 dB.
- 9. Alternately switch calibrator "on & off" and check balance and gain settings. The system is now ready to make the day's calibration and record on tape. "OTE: After system calibration is on tape, <u>DO NOT</u> retune Dynagage.
- 10. When flight settings are made, leave Dynagage at "18". Add or subtract as needed in Burr Brown amplifier. (Always stay I dB under the assigned level - If the difference is an odd number.)

- ii. Continually check the Dynagage tuner for do balance.
- 12. DO NOT rebalance system after the command "Recorders On" is given.
- 13. Only one (1) variable will be used to obtain the desired SPL, if possible.
- 14. A 2 V PP signal will be the equivalent of 120 dB SPL.

NOTE: If the tuning meter should read high throughout the entire tuning range, it indicates that the link circuit is open. If this happens, the transducer cable and its connectors should be inspected. If the meter stays near the middle of the scale during tuning, a short in the transducer cable or in the transducer itself is indicated.

### Accelerometer Calibration

TELEPERENCE THEFTON .

- 1. Set accelerometer voltage at "+ 28 volts dc".
- 2. Set accelerometer amplifier voltage at "+ 15 volts dc".
- 3. Check output voltage when switch is in "Amplifier" position.
- 4. Balance output to "Zero" with balance pot, adjust dc balance and check with digital voitmeter.
- 5. Run a current insertion calibration on the sensitivity range selected for the day's flight, using table below as a guide:

External Calibrate
8 micro amps
16 micro amps
20 micro amps
20 micro amps
20 micro amps

# Current Insertion Calibrating Procedure

- Insert the phone jack of the external insertion box into front of accelerometer control panel.
- 2. Record "Zero" voltage on data sheet.
- 3. With the calibrate switch of the external calibrate box in the "Positive" position, adjust the balance pot to give the required current level as listed in step 4 above. Record the voltage, then switch to the "Negative" calibrate position and record the voltage on your data sheet.
- 4. Record calibrate 0, +, and signals on tape recorder.

# Strain Gage Calibration

- 1. Check system for proper sensitivity range card. (Register Board.)
- 2. Check output voltage (amplifier balance) when switch is in "Dummy Gage"

· Analogic companies.

position. (Should be "Zero".)

- 4. If calibrate voltage varies more than 20-millivoits from original calibration, call to attention of project engineer.
- 5. Switch to "Active Gage" position and zero active bridge.
- 6. Check calibrate voltages with digital voltmeter. (Record on data sheet.) Record calibrate signal on tape recorder.

# Bruel & Kjaer Microphone Calibration

- 1. Set Burr Brown Amplifier (Model 9860) at 100 dB.
- Install the proper adapter on the driver unit of Model PC-125 callbrator. (Photocon unit.)
- 3. Check the battery condition of the PC-125 by turning the function control to "Bat. Check". If the meter reads below the line marked "Bat. Check", recharge the batteries for a minimum of 12 hours. If the meter reads above the "Bat. Check" line, proceed as follows:
- 4. Set the "dB SPL" control to 100 dB, turn the function control to "Operate" and adjust the "SPL ADJ" control until the "SPL" meter reads zero dB.
- 5. Verify that the two 100 dB settings produce a 1.5 volt p-p (± 10%) reading on the oscilloscope. (Note: If scope indicates greater than ± 10%, set unit's knob to produce 1.5 volts (± 10%) and then reset knob by means of a set screw, to zero).
- 6. Verify that oscillograph deflection is approximately 0.5 in. with the two 100 dB settings.
- 7. For data runs, set amplifier gain knobs in accordance with the published schedule for each individual mission. (Normally, these settings were determined by SRI and were different for each noise and each boom mission.) The dial settings then become the "calibration" for each mission. (Examples: If dials indicate 177 dB, the 1.5 volt p-p signal of step 5 above equals 117 dB. If dials indicate 83 dB, 1.5 volts p-p = 83 dB.

## High Frequency Accelerometer Calibration

- 1. Set oscillator to 1000 Hz (cps).
- 2. Plug oscillator into "Oscillator" terminal on Datacraft calibration panel
- 3. Plug scope into "Monitor" terminal on Datacraft calibration panel.
- 4. Set selector switch on Datacraft panel to proper channel, and set toggle switch to "Input".
- 2. Adjust amplitude control on oscillator until proper mv/g level is read on scope (400 mv/g accelerometers are being used). Correct input voltages will be assigned each day.
- 6. Reset toggle switch on calibration panel to "Output". Adjust gain control on that panel until output reads 2.0 volts p-p on the scope.

 Repeat for other channels, turning selector switch to proper channel each time.

#### TAPE RECORDER TR-8

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In order to ensure recordings of the Sonic Boom pressure within the linear range of the instrumentation system, and utilize the optimum dynamic range of the equipment, a proper system sensitivity must be established. Experience has shown that the actual measured pressure to nominal anitcipated pressure can vary in the order of 1.5. Therefore, a system sensitivity calibration should take into account this possible increased overpressure. Thus, given a 2 PSF nominal peak overpressure, the system should be set for 3 PSF full scale.

The following procedure gives a system sensitivity of 3 PSF peak.

- I. Set DC amplifiers gain controls on channels 1-6 to -20 dB position and adjust oscillograph galvanometers for 1" centers across the paper. Apply 40 cps cal tone at 0.5 volts rms to channels 1 through 6, one at a time. Adjust oscillograph sensitivity potentiometers for a 1" deflection peak-peak. The oscillograph is now adjusted for a 3 PSF peak value.
- 2. Reset DC amplifier gains (channels I-6) to 0 db position and switch transducer balance switches to position #2. Energize tape recorder and place in record mode. Apply 400 cps to channels 7-12, one at a time, and adjust FM reproduce amplifiers to 0.5 volts rms output. Read this output on channels I-6, channel I corresponding to channel 7, channel 2 to channel 8, etc.
- 3. Return channels I-6 DC amplifiers to +20 db level and switch transducer balance switches back to position #1.
- 4. A system end-to-end calibration is now obtained by applying the acoustic calibrator to each transducer. As each channel is checked, adjust the transducer sensitivity potentiometers for 0.234 volts rms (0.7 PSF peak) on VTVM. The tape recorder is now adjusted for a 3 PSF peak value. Record this signal, with appropriate annotations, on the tape recorder. The system is now ready for operation.
- 5. A system end-to-end post calibration is accomplished by repeating step 4, but no adjustments are made.
- Sensitivity adjustment: Should the booms continually exceed the 3 FSF peak overpressure range, a greater dynamic range can be adjusted accordingly.

When system sensitivity other than POSF peak are desired, system adjustments as illustrated below, can be made.

For a 4 PSF boom (rominal), let 1 volts rms = 6 PSF peak, then X volts rms = 0.7 PSF peak (calibrator), X = 0.117 volts rms.

With calibrator applied, adjust the transducer sensitivity potentiometers to obtain this reading for each channel. The 400 cps insert at 0.5 volts rms is now equivalent to 6 PSF peak to peak.

- Oscillograph: For 6 PSF peak value, now read out values using "60" scale. Using 400 cps insert, adjust for I" peak to peak deflection as previously.
- 3. An alternative method is shown below:

For a - 6 db attenuation
then: 6 db = 20 log X
log X = 0.3
or X = 2
Therefore, new system sensitivity is:
(2)·3 PSF = 6 PSF

The disadvantage of this method is that only - 3 db steps are available on the dynagages, thus for any value other than - 6 db the peak value becomes a fractional number and direct readout is difficult.
i.e.:

For = 3 db of attenuation then: 3 db = 20 log X $\log X = 0.15$ X = 1.41so new range becomes  $(1.41) \cdot (3) = 4.23 \text{ PSF peak.}$ 

# APPENDIX D

# MISSION LOG

Contents	Page
Legend	D. I
Table D-I Mission Log - Edwards Phase I	D. 2
Table D-2 Mission Log - Edwards Phase II	D.7

# LEGEND

The following is an explanation of notations and abbreviations used in Mission Log Tables D-1 and D-2.

1.	DATE	Day, Month, Year
2.	MSN	Mission Identification Number
3.	A/C	Aircraft
4.	ALT, KFT, MSL	Altitude, 1000 ft. above Mean Sea Level
5/	MACH OR SPD	Mach Number for supersonic aircraft or
		Speed in Knots per Hour for subsonic air- craft.
6.	EPR, TKFF,(LDG)	Engine Power Ratio, Takeoff. If ratio is for landing, it is shown in parenthesis.
7.	HDG	Heading, degrees measured clockwise from magnetic north.
8.	OFFSET	Offset from track, North or South.
		Table D-I, offset is given in miles.
		Table D-2, offset is given in 1,000 ft.
		Table D-2, L = left or south, R = right
		or north.
9.	BOOM TIME	Observed boom time or time overhead for
		subsonic aircraft at house E-2, ZULU. Local
		time is ZULU minus eight hours.

TABLE D-1
MISSION LOG - EDWARDS PHASE I\*

	DAT	ε	MSN	A/C	ALT	MACH	EPR	HDG	OFF-	BOOM TIME
DY	MO	YR			KFT	OR			SET	HR MON SC
				İ	MSL	SPD			N/S	ZULU*
4	JUN	66	14.	F-104	35,6	1.7				
4	JUN	66	13.	XB-70	52,9	1.81	1	243	2.5N	17 28 00
6	JUN	66	39	B-58	31.4	1,25	1 1	244	4.64N	16 00 00
8	JUN	66	39B	KC-135	10.3		1,6			
6	JUN	66	70	B-58	43.9	1.60		245	0.55N	16 08 51
6	JUN	66	70B	KC-135	5.4		1.5			
6	JUN	66	40	B-58	31.4	1.48	{	246	0.20N	16 18 40
6	JUN	66	40B	KC-135	5.4	_	1.5			
6	Jun	66	71	B-58	44.2	1.59		245	5.0GM	16 30 00
6	Jun	66	71B	KC-135	3.3		1.5			
6	JUN	66	41	B-58	31.3	1.45		247	0.17N	16 34 44
	JUN		41B	KC-135	3.3		1.5		'	
	JUN		72	B-58	43.9	1.55	<b>j</b> .	244	4.85N	16 43 55
	Jun		72B	KC-135	2.8		1.5			
	Jun		74	B-58	32.4	1.30		242	.728	17 01 52
	JUN		74B	KC-135	8.3		2.35			
	JUN			B-58	43.4	1.57		245	5.00N	17 11 00
	JUN			KC-135	8.3	_	2.35			
	JUN			B-58	31.8	1.46	[ _ [	248		17 17 00
	JUN			KC-135	3.3		2.35			
1	JUN			B-58	43.3	1.53		245		17 24 40
	JUN			KC~135	2.8		2.35			
	JUN			XB-70	72.0	2,83		262	4.10N	17 26 00
	JUN			B-58	31.9	1,43		247	0.25N	17 31 30
	JUN			KC-135	2.5		2.35			
	JUN			B-58	31.6	1.48		241	1.098	16 10 40
	JUN			KC-135	4.3	1	2.35	i i		ł
	JUN		45A 45B	KC-135	3.0	3.20	2.35		4 053	1
	JUN			B-58 KC-135	43.7	1.70	0 25	244	4.95N	16 23 50
	JUN		77B	B-58	31.7	1 83	2.35	044	0 100	16 33 12
	JUN		46A	KC-135	2.6	1.51	2.35	244	0.108	10 30 12
	JUN		46B	B-58	43.7	1.65		346	5.42N	16 40 05
	JUN			B-58	38.7	1,31		245	5.42N 5.23N	17 11 20
	JUN			KC-135	3.0	4,34	2.35	780	J. ZJR	1, 11 20
	JUN		79A	B-58	31.6	1.52	55	244	0.12N	17 22 20
	JUN		79B	KC-135	2.6	1.02	2.35		0.120	
	JUN		49A	B-58	43.3	1.43	35	252	4.68N	17 28 15
	JUN		49B	KC-135	4.3	••••	2.35		2,004	
	JUN		80A	B-58	31.6	1.53	55	244	0.25N	17 38 45
	JUN		80B	KC-135	3.0		2.35		-,=011	-, 40 30
	JUN		50A	B-58	43.3	1.43	}	245	5.00N	17 47 37
	JUN		50B	KC-135	8.3		2,35		· · ·	
	JUN			B-58	31.4	1.49		245	0.065	17 56 25
	JUN		81B	KC-135	4.3	-, 10	2.35		-,	
					<del></del>			L		

<sup>. \*</sup>Refer to Legend, Page D.1 for explanation of notations and abbreviations.

MISSION LOG - EDWARDS PHASE I (Continued)

	DATE	3	MSN	A/C	ALT	MACH	EPR	HDG	OFF-	BOOM TIME
DY	MO	YR		.,,	KFT	OR			SET	HR MIN SC
-					MSL	SPD			N/8	ZULU
	JUN		1	XB-70	31.8	1.38		246	5.028	15 19 00
1	JUN		43A	B-58	42.4	1.62		245	5.24N	16 00 22
	JUN			KC-135	14.3		2.35			
,	JUN			B~58	31.2	1.44		244	0.23N	16 06 45
1	JUN		75B	KC-135	8,3		2.35			
	JUN		42A	B-58	43.3	1.67		247	4.85N	16 14 50
	JUN		42B	KC-135	2,8		1.5			
	JUN		73A	B-58	31.2	1.50		245	0.10N	16 24 20
t .	JUN		73B	KC-135	2.5		1.5			
•	JUN		41A	B-58	43.2	1.60		246	5.32N	16 30 10
	JUN		418	KC-135	5.3		1.5			
	JUN		72A	B-58	31.2	1.49	, _	245	0.16N	16 38 45
	JUN		72B	KC-135	2.8		1.5			
1	JUN		57	KC-135	3.3		1.5		, , , , , ,	
	JUN		57B	B-58	37.6	1.66		248	5.90N	17 05 10
	JUN		80RA	KC-135	2.8		1.5	0.4	0 3 435	
	JUN		80RB	B-58	31.3	1.46	,	247	0.14N	17 12 30
8				KC~135	5.3		1.5	044		17 01 00
	JUN		56RB	B-58	43.0	1.64	, ,	244	5.14N	17 21 22
	JUN		87	KC-135	3.3		1.5	045	0.40	17 00 00
8			87	B-58	31.4	1.49	, ,	245	0.40N	17 28 30
	JUN		55RA	KC-135	10.3		1.5	044		17 20 10
	JUN			B-58	43.2	1.64	,	244	5,16N	17 36 10
	JUN		86RA	KC-135	5.3	, 40	1.5	229		.7 45 00
1	JUN		86RB	B-58	31.4	1.49	١, ,	249		7 45, 00
1 .	JUN		86SA	KC-135	5.3	1 50	1.5	040	O OEN	16 08 30
9			86SRB		31.0	1.50	, ,	246	0.25N	10 00 20
9			55SA	KC-135	10.3	1 60	1.5	244	5,17N	16·19 20
9			55SRB		35.7	1.69	ا , و ا	244	5.174	10 19 20
9	JUN		87SA	KC-135	3.3	T 50	1.5	244	0.085	10 25 50
9			87SRB	•	31.0	I.53	1.5	244	0,000	16 25 58
9			56SA	KC-135	5,3	1 72	1.5	243	4.70N	16 34 50
	JUN		56SRB		42,3	1.72	1.5	273	7,7011	. 10 23 20
	JUN JUN		80SA 80SRB	KC-135	2.8 31.0	1.53	1.5	245	0.06ir	16 41 40
	JUN		i i	KC-135	31.0	1.04	1.5	223	0,0011	70 47 40
	JUN	4		· .	43.1	1.70	1.5	244	5.23N	16 49 10
	JUN			B-58	42.9	1.52		240	4.87N	17 07 54
	JUN			KC-135	5.3	1.02,	1.5	270	7,0/1	A, 0, 04
•	JUN			B-58	31.7	1.50	1.5	243	0.498	17 16 15
	JUN			KC-135	2.5	1.30	1.5	230	· 705	., ., .,
	JUN			B-58	43.1	1.52	1.0	241	4.69N	17 23 54
1	JUN			KC-135	2.8		1.5		-,0011	
	JUN			B-58	31.7	1.55		246		17 31 23
	JUN			KC-135	8.3	1.55	2.35	~70	j	A, UA AU
	0 UM	-00	, 300	20 100		L				

TABLE D-1
MISSION LOG - EDWARDS PHASE I (Continued)

			2002011 20	<u> </u>	RUS PILAS				
į	DATE	MŚN	A/C	ALT	MACH	EPR	HDG [	OFF-	BOOM TIME
DY	MO YR			KFT	OR		ď	SET	HR MAN SC
				MSL	SPD		<u></u>	N/S	ZULU
									,,,,
9	JUN 66	4384	B-58	43.0	1,68		243	4.62N	17 39 00
9	JUN 66	43SE		14.3	·	2.35			
9	JUN 66	42SA		43.3	1.70		244	4.92N	17 57 00
9	JUN 66	42SE		2.8		1.5	i	1	
9	JUN 66	46SA		42.9	1.68		246	4.74N	18 11 10
9		46SE		3.3	-	2,35			
9	1	72SA		31.3	1,53		248	0.63N	18 22 10
	JUN 66			2.8		1.5			
	JUN 66	18A	B-58	37.7	1.64		231	0.098	16 46 43
•	JUN 66		B-58	49.6	1,66		234	0.36S	16 49 22
	JUN 66		B-58	37.8	1.69		236	0.218	17 00 16
	JUN 66		B-58	49.2	1.72		231	0.358	17 02 48
	JUN 66		F-104	21.2	1.40		231	0.08N	17 12 35
•	JUN 66	• '	F-104	29.7	1.60			0.648	17 13 45
	JUN 66		B-58	49.3	1.67		233	0.03N	18 06 25
	JUN 66		B-58	38,1	1.67		232	0.118	18 07 35
	JUN 66		B-58	49.8	1.64		235	0.53N	18 20 25
	JUN 66		B-58	38.0	1.67		233		18 21 10
	JUN 66		F-104						16 08 00
	JUN 66		F-104	29.9	1.54		238	.108	16 10 50
	JUN 66		F-104		Ì		ł		17 45 00
	JUN 66		F-104	29.7	1.52		233		17 45 45
	JUN 66		F-104	29.7	1.49		231		17 57 30
14	JUN 66	37B	F-104	21.1	1.39		231	0.028	17 58 40
15	JUN 66	1XA	F-104	14.1	1,21		236	0.47N	16 14 50
15	JUN 66	1XB	F-104	28.1	1.50		233	0.13N	16 16 40
15	JUN 66	2XA	F-104	29.7	1.32	1	237	0.66N	16 21 40
15	Jun 66	2XB	F-104	14.1	1.20		233	0.22N	16 22 10
15	JUN 66	ЗХА	F-104	29.1	1.58	l	234	0.17N	1
15	JUN 66	3XB	F-104	14.2	1.15	ł	235	0.18N	
	JUN 66	•	F-104	14.1	1.28	l	235	0.18N	
	JUN 66		F-104	29.9	1.62	]	233	0.445	16 48 20
	JUN 66		F-104	29.3	1.65	1	230	0.105	15 56 25
	JUN 66		F-104	20.5	1.40		228	0.265	15 57 50
	JUN 66		F-104	29.7	1.65	l	344	0.25E	16 04 25
•	JUN 66		B-58	41.3	1,55	[	232	2.20N	15 54 50
	JUN 66	4	KC-1.35	5.3		1.5	1		l
•	JUN 66		B-58	32,1	1.45		232	1.908	16 08 00
	JUN 66		KC-135	3.3	I _	1.5	1		
	JUN 66	1	B-58	42,7	1.59		232	5.00N	16 18 54
	JUN 66		KC-135	4.3		2.35		1	
	JUN 66	•	B-58	31.2	1.43		236		16 27 10
•	JUN 66		KC-135	3.0		2.30			
•	JUN 66		B-58	43.0	1,59	l .	230	4.87N	16 35 40
	JUN 66		KC-135	,		2.30			
20	JUN 66	59A	KC-135	12.0	<u></u>	2,35		<u> </u>	

TABLE D-1

MISSION LOG - EDWARDS PHASE I (Continued)

	DAT	F	MSN	A/C	ALT	MACH	EPR	HDG	OFF-	BOOM TIME
hv	MO	YR	MUSIN	1 20	KFT	MACA	EFR	nDG	SET	HR MN SC
۲,	J.C	111	•		MSL				N/S	ZULU
			<del> </del>							
20	JUN	66	59B	B-58	43.4	1,41		233	5.00N	17 10 00
20	JUN	66	98A	KC-135	6.0		2,35			
20	JUN	66	98B	B-58	31.3	1.50		233		17 15 45
20	JUN	66	60A	KC-135	6.0		2.35			
20	JUN	66	90A	KC-135	6.0		2.35			
20	JUN	66		B-58	31.8	1.55		230	0.178	. 17 32 00
20	JUN	66	85A	B-58	32.3	1.45		231.	4.35N	17 40 00
20	JUN	66	85B	KC-135	2.6		2.30			}
20	JUN	66	93A	KC-135	2.6		2.30			
	JUN			B-58	32.1	1.55		231	0.178	17 47 50
	JUN			KC-135	2.5	-	1.5			
21	JUN	66		B-58	31.8	1.46		232	0.12N	16 01 55
	JUN			KC-135	2.8		1.5			
	JUN			B-58	43.6	1.67		233	5.12N	16 11 02
	JUN			KC-135	4,3	. •	2.35			
	JUN			B-58	31.7	1.47		233	0.17N	16 17 05
	JUN			KC-135	2.8		1.5		.,	
	JUN		66B	B-58	39.9	1.59		233	5.00N	16 25 17
	JUN			KC-135	3.0		2,35		0,000	10 00 2.
			100B	B-58	31.8	1.46		232	0.148	16 30 23
	JUN			KC-135	8.3		2.35		0,2.0	10 00 10
	JUN			B-58	44.1	1.62	3,00	232	4.83N	16 39 19
,	JUN			B-58	39.4	1.39		233	5.00N	17 29 35
•	JUN			KC-135	4.3	2,00	2.35		0,000	2. 25 50
	JUN			B-58	43.1	1.60		232	5.00N	17 44 12
	JUN			KC-135	5.3	2.00	1.5		0,00	11 12 14
	JUN			B-58	43.8	1.65		235	5.40N	17 56 55 ·
	JUN			KC-135	5.3	2,00	1.5		0,10.	2, 00 00
	JUN			KC-135	8.3		2.35			
	JUN			B-58	43.9	1.64		233	5.16N	18 08 59
	JUN		61A	KC-135	4.3		2.35			
	JUN		61B	B-58	43.3	1.62		232	4.76N	19 37 19
	JUN		101A	KC-135	2.6	-,05	2.35		-,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
			101B	B-58	31.7	1.50	_,,,,	233		19 51 15
	JUN			B-58	31.7	1.50		234	0.22N	20 05 50
	JUN		85B	KC-135	2.6	_,	2.35		-,,	
	JUN		28A	B-58	37.0	1.63	= • = •	234	0.18N	16 13 27
	JUN		28B	F-104	20.8	1.35		233	0.168	16 13 43
,	JUN		19A	B-58	37.2	1.64		233	0.24N	16 28 15
9	JUN		19B	F-104	29.5	1.42		233	0.205	16 30 05
1	JUN	1	6X	B-58	43.6	1.60		259	1.348	16 48 24
	JUN		30A	B-58	37.4	1.65		230	0.208	17 43 34
	JUN		30B	F-104	29.7	1.37		232	0.16S	17 44 38
,	JUN		34A	F-104	29.6	1.39		233	-,	17 56 06
	JUN		34B	B-58	43.4	1.61		230	4.00N	17 57 06
	JUN		24A	B-58	43.3	1.60		233	5.06N	18 10 37
	JUN		24B	F-104	20.9	1.36		231	0.238	18 11 26
	0 011	~~!		0-	20,0			201	0,200	10 11 20

TABLE D-1
MISSION LOG - EDWARDS PHASE I (Continued)

j BRELINGSBERGERMANNE INKOVER OKKUMINI

DY	DATE MO YR	WSW	A/C	alt KFT MSL	MACH	EPR	HDG	OFF- SET N/S	BOOM TIME HR MN SC ZULU
22 22 22 22 22 22 23 23 23 23 23 23	JUN 66 JUN 66 JUN 66 JUN 66 JUN 66 JUN 66 JUN 66 JUN 66	35B 25A 25B 23A 23B 17A 17B 22A 22B 31A	B-58 F-104 F-104 B-58 F-104 B-58 F-104 F-104 B-58 B-58 F-104	MSL 43.4 21.1 21.9 43.2 29.7 37.4 37.6 21.6 29.3 43.4 37.5 21.3	1.60 1.28 1.39 1.59 1.51 1.63 1.6. 1.40 1.40 1.67 1.64	·	225 235 233 233 237 232 231 227 232 229 231 232	N/S 0.92S 0.25N 0.21N 4.89N 0.34N 0.50N 0.39N 0.46S 4.25N 0.12N	2ULU  18 21 21  18 22 47  18 36 39  18 37 59  18 50 21  18 52 05  15 46 08  15 48 00  15 59 59  16 00 40  16 12 14  16 12 21
23 23 23 23 23 23 23	JUN 66 JUN 66 JUN 66 JUN 66 JUN 66	33B 20A 20B 36A 36B 7X	B-58 F-104 F-104 B-58 F-104 B-58 F-104 B-58	43.2 29.8 21.5 37.4 20.9 37.4 29.6 43.5	1.64 1.49 1.37 1.65 1.39 1.66 1.55		232 230 233 233 230 231 258 258	5.02N 0.10S 0.19N 0.10N 0.37S 0.25S 0.29S 9.86N	16 21 38 16 22 04 19 51 20 19 54 17 20 05 15 20 06 26 20 18 18 20 21 21

REMAKKS 1	REMARKS	J57 1		æ	6	-	-	5		-	<b>5</b>	0	7	-	2	8	-	N 25	Δ,	3		9	3 NO RDR	NODATA E1231	7	762 1	J57 NO RUK 1	,	* c	REMARKS 1	
TME	1 H	_	a	2	0	-	0	10	50	2	C	5	4	0		~	'm	12	<b>•</b> •	S.	4	3	<i>LU</i>	- 4	7	14	,		ń	1 1 S	
BOOM H MN	) } }	ŀ	- 1		l				1			- 1			- 1		l	- 65		- 1			1		0	8 38	)		¥	BOOM	
S	. v	-	' ¬	~	7	-	~	Н	7	~	-	7	~	2	٦	~	7	46.	٠,	7	4	H	• ~-		4	27 1	7	27	~	S	*1
8 2	_	6	3	ന	E	m	0	0	3 0	0	0	7	m	a)	n	% 8	ľ	n w	m (	3 34	m	ိုက	. <b>W</b>	, ,	3	3	n	3		8 2	
OFF.	֓֞֞֜֜֜֜֜֝֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֓֓֓֓֡֓֓֡֓֡֓֡֓֡	٦ 6	133	R 2	ĺ	R32.	ø		3	R71.		R38.		R67.	4	0	-	•		1. 2.				,	3	2	- 1	L10.	SET /Rak	OFF-	SE
<del>1</del> 00	<b>.</b> .	242	246	549	249	245	246	074	265	549	890	241	76	248	244	990	246	740 245	247	245	246	247	١ .	757	3	241	4	243	4	HD6	¥ H
EPR								1.67			1.76		1.76			1.76													DG1	EPR	D S P
A G		32	46	14	65	51	Š		65	٠.		•62		'n	65	:		7	ທ	6	ī	2	2	u	48	r m	4	946	7	E.	A R
Σ	4 Z 5	4	7 2		ì	4		1	5.	7	7	~	۲.	1	5		2	٠ ا	ر ا	8		4 1	٠ ٥	•	3	9		2 1	S a		E D W
ALT		32.			0,7	59	90	n	35	9	3	U U		9	35	١	20	9 4	32.	~	37.		0	(	~	18.	0	37.	MSL		ω,
AZC	0010	- 00	XB-70	0	·loo	X8-70	-	8-00	က	XB-70	DC-8	മ	WC135B	·	6	WC135B	) r-	X8-70	<b>6</b> 0	F-104	<b>~</b>	່ຕ	F-104	0	XB-70	$\circ$	F-104	~		A/C	
MSN	7_01	10-2	10-1	0-3	9-2	9-1	8-3	8-2	8-1	7-3	7-2	7-1	6-3	6-2	6-1	ית ו יינ	100	4-2	4-1	3-4	3-2	3-1	2-2	2-5	2-1	1-4	1-2	1-1		MSN	
3	o	99	9 C	9 9	9	. 9	67	67	67	29	67	67	99	99	99	2 4	2 4	99	99	99	99	2 6	9 4	99	99	99	99	99	۲ ۲	1	
DATE	> L	200	> > 2 C	2	NOV	200	A	AN	A A	SAN	SAN	SAN	DEC	DEC	DEC	י ע שני	יי טיר מיי		DEC	DEC		بار 2 الح 2 م		NON .	0 0 0	> > 2	NON	NOV V	Q	ATE	
2	ທ ້	m	ۍ د	<b>C</b>	ا د	٠ ح	٠,	<u>ا</u> ا	~	m	6	n	0	90	1 C	, c	'n	, o	9	10	1 ~	7 ~	00	0	· C	n n	23	(1)	۵		

\*Refer to Legend, Page D.I for explanation of notations and abbreviations.

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REMARKS 1 1	1 OFF COURSE 1	rd prd prd		e e
HDG OFF- OBS BOOM TME SET DY HR MN SC L/R,K ZULU	244 L 1.3 312 16 29 35	243 L 2.0 312 16 56 13 68 246 R 1.4 312 17 14 51	62 250 R 3.9 312 17 40 35 73 R .1	247 R 1.0 312 18 02 58 78 R .i
ALT MACH EPR KFT OR TKFF MSL SPD (LDG)	47.6 1.60 1.76	47.5 1.65 3 3.9 250 1.76 47.8 1.65	3 3-3 235 1-76 47-7 1-65 3 5-4 230 1-76	46.8 1.65 3 3.9 215 1.76
DATE MSN A/C DY MO YR	NOV 66 21~1 B-58 NOV 66 21-2 WC135	66 22-1 B-58 66 22-2 WC135 66 23-1 B-58	NOV 66 23-2 WC135 NOV 66 24-1 B-58 NOV 66 24-2 WC135	NOV 66 25-1 B-58 NOV 66 25-2 WC135
	! !	:		

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	REMARKS 1	1 1 1 1 M85 1	M85 1 1 M83 1	M63 1 1 M85 1 1 M85 1	M85 1 1 M83 1	REMAPKS 1 1 1 1 1 MR5	
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TABLE D-2	DEC-66119-	NOV 66121-	NOV 66121-	DFC 66122-	DEC 66122-	DEC 66123-	DEC 66123-	DEC 66124-	DEC 66124-	DEC 66125-	DEC 66125-	DEC 66126-	DEC 66126-	DEC 66127-	DEC 66127-	DEC 66128-	DFC 66128-	DEC 66129-	DEC 66129-	DEC 66130-	DEC 66130-	DEC 66131-	8 DEC 66131-	8 DEC 66132-	DEC 56132-	15 NOV 66149-	15 NOV 66150-1	5 NOV 65161-	1 DEC 66172-	1 DEC 66172-	5 NOV 66174-	DATE	? α >		- [6677]	-12000	8 DEC 66221-	5 NOV 66249-	5 NOV 66250-	15 NOV 66261-2	5 NOV 66274-

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Flight data for SR-71 missions not available for release at this time. Each condition (COND) represents a certain altitude, Mach number, etc. NOTE:

# APPENDIX E

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TYPICAL INSTRUMENT LOCATION LOG

(For 15 November 1966)

TYPICAL INSTRUMENT LOCATION LOG*	CHNL	l	104 1 MA4 ACQUSTIC SALE	102 1 MAS ACCISETE TOWN OF HIS MAJ AVIS VERT 103 2 MAJ ACCISETE AND OR SUSP 6 FT ANY FLR	108 2 MA2 ACOUSTIC CHT2 KIT SUSP 6 FT ABV FLP	110 2 MA4 ACOUSTIC CYTR FR 111 2 MA5 ACOUSTIC FR-XIT-DR XIT STOVE	- <u>1:122-WA&amp;ACOUSTICFR-KIT&gt;3-PR-4-15-3-6-FT-ABV-FER-WR-CHINA-CEOS1-</u> 113-1-MA7ACOUSTICOUTSINE SUBJECT-FROUP	114 IRIG R TI	CHRC 30 NI 201 1	102 1 A 1 - LF-ACEFL - 102 F WALL (N-S ACEFL)  - 2002 1 A 1 - LF-ACEFL - 102 F WALL (N-S ACEFL)	204 I MLS PRESSURE PRI F WALL NEXT TO All	205-1-444 PRESSHEET FORTH CON MILE STORES 206.2 STRAIN GARAGE WNOW 349 FROM CHTR	20. 1 SG3 STRAIN GARAGE UNDY 2ND FROM CNIR	200 2 MAP ACOUSTIC TPIGGER MIKE 210 2 SG43 STRAIN GARAGE WNOW 1ST FROM CNTR	212 2 SG44 STRAIN GARAGE WIDW CENTER 1	CHNL HOUSE INST TYPE LOCATION POICE INSTR	401 2 A1 LF ACCEL RO FLO 203 2 A3 LF ACCEL PO RFD 302 2 A2 LF ACCEL FR FLO	304 I AL LE ACCEL EN FEN CONC BLK AXIS VERT I AL LE ACCEL FR-KIT FLR CONC BLK AXIS VERT I I AL ACCEL FR FIR CONC BLK AXIS VERT
TYR	E CHML HOUSF	NOV 65 101 1 MA2	66 104 1 MA4 66 104 1 MA4	NOV 56 [07 1 FA3 - NOV - 56 - 105 - 1 - A3 - NOV - 55 107 2 MA3	NOV 66 108 2 1.42 NOV 66 109 2 MA3	NOV 66 110 2 MA4 NOV 66 111 2 MA5	-NOV-46-112-2-4AE-	NOV 66 114	MO YR INSTRUCTIONS	5-NOV-66-202-1-ATI-	NOV 66 204 1 ML3	5 NOV 66 206 2 SG41	MOV 66 201 1 SG3	MOV 55 200 2 34AP NOV 66 210 2 5643	2 \$644	HOV-66-214- DATE CHNL HOUSE WO YR INSTR	46 401 2 A1 66 203 2 A3 66 302 2 A2	NOV 66 305 1 A2

\*Refer to Legend, Appendix  ${\tt B}_{m{s}}$  for explanation of notation and abbreviations.

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6 319 2 A59 HF ACCEL ATR COMP DOOR 6 310 2 A59 HF ACCEL ATR COMP DOOR 6 310 2 A69 HF ACCEL ATR COMP DOOR 6 310 2 A69 HF ACCEL ATT CLOSET DOOR 6 311 2 A10 HF ACCEL ATT CLOSET DOOR 6 312 2 A10 HF ACCEL ATT CLOSE WOVABLE OR CWTR N WINDOW. 6 313 4 A10 HF ACCEL REALT-ONR WOVABLE DR CWTR N WINDOW. 6 314 CLOSET DOOR DLATE LINE N VALL NF CORNER (N-S ACCEL) 6 401 2 A5 LF ACCEL RAIL CMPP CLOSET TRUSS 6 402 2 A5 LF ACCEL RAIL CMPP CLOSET TRUSS 6 403 2 A5 LF ACCEL RAIL CMPP CLOSET TRUSS 6 404 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL CMPP CLOSET TRUSS 6 405 2 A7 LF ACCEL RAIL TRUST 6 406 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 407 2 A7 LF ACCEL RAIL TRUST 6 408 2 MLS PRESSURE RITH TRUST 6 409 2 MLS PRESSURE RITH TRUST 6 400 2 MLS PRESSURE RITH CODE AND VOICE 6 411 2 D1 D15PL ADJACKNY TO A5 WITH SAME AXIS 6 414 1 A CCEL TOP STFEL COL SOUTH SIDE AXIS 6 414 1 A CCEL TOP STFEL COL SOUTH SIDE AXIS 6 405 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 506 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 507 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 508 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 508 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 LF ACCEL TOP STFEL COL WEST SIDE AXIS 6 509 3 A34 L			۴	307	1		r	KIT WADE BETY KIT AND PR		
15 NOV 66 310 2 A59 HF ACCEL PR-KIT-DR MOVABLE KIT CABNT DOOR ABV SINK LFFTT 15 NOV 66 311 2 A10P HF ACCEL HPT LTXST TOOR 15 NOV 66 311 2 A10P HF ACCEL PR-KIT-DR MOVABLE DR CNTR N WINDOW 15 NOV 66 312 2 A12P HF ACCEL PR-KIT-DR MOVABLE DR CNTR N WINDOW 15 NOV 66 312 2 A12P HF ACCEL PR-KIT-DR MOVABLE DR CNTR N WINDOW 15 NOV 66 A01 2 A5 LE ACCEL PR-KIT-DR MOVABLE DR CNTR RULE N CORPER (N-S ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CORPER (N-S ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A01 2 A5 LE ACCEL SNOF DLATE LINE N WALL NE CRNER (F-W ACCEL) 1 NOV 66 A10 2 ALL PRESSURE BRI CNTR CLG SUSD 2 IN SELON COLD SNOW 64 A10 2 ACCEL SNOF DLATE LINE N WALL NE CRNER AND 65 A10 2 ALL PRESSURE BRI CNTR CLG SUSD 2 IN SELON COLD SNOW 64 A10 2 ACCEL SNOW 64 A10 2 A			C	308			Q.			
15 NOV 65 312 2 A13P HF ACCEL KIT CARNET   15 NOV 65 312 2 A13P HF ACCEL KIT CARNET   15 NOV 65 312 2 A13P HF ACCEL KIT CARNET   15 NOV 65 312 2 A13P HF ACCEL KIT CARNET   15 NOV 65 314 2 A12P HF ACCEL KIT CARNET   15 NOV 65 314 2 A12P HF ACCEL KIT CARNET   15 NOV 65 315 2 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 314 2 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 2 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 2 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A12P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FORD   15 NOV 65 315 A14P HF ACCEL RESTORMENT FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL RESTORMENT FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15 NOV 65 315 A14P HF ACCEL REPRESIONER FOR SOUTH SIDE     15			æ	309			Q	KIT-DR MOVABLE KIT CABNT DOOR ABV	- 1	
NOV 66 312 2 A110 HF ACCEL KIT CARNET  NOV 66 312 2 A110 HF ACCEL KIT CARNET  NOV 66 312 2 A110 HF ACCEL RETEAST WONDOW  NOV 66 312 2 A110 HF ACCEL RETEAST WONDOW  NOV 66 401 2 A5  NOV 6 402 2 A9  LF ACCEL ROOF PLATE LINE N WALL NF CORNER (N-S ACCEL) I  NOV 66 402 2 A9  LF ACCEL ROOF PLATE LINE N WALL NF CORNER (N-S ACCEL) I  NOV 66 402 2 A9  LF ACCEL ROOF PLATE LINE N WALL NF CORNER (N-S ACCEL) I  NOV 66 402 2 A7  LF ACCEL ROOF PLATE LINE WALL NF CORNER (N-S ACCEL) I  NOV 66 405 2 A7  LF ACCEL ROOF PLATE LINE WALL NF CORNER (N-S ACCEL) I  NOV 66 407 2 A9  LF ACCEL ROOF PLATE LINE WALL NF CORNER (N-S ACCEL) I  NOV 66 407 2 A0  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL INTERIOR OF ROOF GIPDER AT CENTERLINE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 A11 LF ACCEL TOP STEFL COL WEST SIDE  NOV 66 507 3 STEALN ROOT FLANGE ROOF PURLIN AT CENTERLINE  NOV 66 507 3 STEALN ROOT FLANGE ROOF PURLIN AT CE		5	6	310	1	ł	۲	CLUSET DOOR		
15 NOV 66 312 2 A11P HF ACCEL FR-LTI-PR MOVARIE DR CMTR N WINDOW.  15 NOV 66 314 A12 2 A11P HF ACCEL FR-LTI-PR MOVARIE DR CMTR N WINDOW.  15 NOV 66 401 2 A5 LF ACCEL FOOF PLATE LINE N FALL HF CORMER (N-S ACCEL) 15 NOV 66 402 2 A5 LF ACCEL FROM PLATE LINE N FALL HF CORMER FEW ACCEL) 15 NOV 66 402 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 402 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 402 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 A5 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 A12 LF ACCEL FROM PLATE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 HL2 PRESSURE FEW ACCEL TO FIND LINE LINE N WALL HF CORMER FEW ACCEL) 15 NOV 66 403 2 HL2 PRESSURE FEW ACCEL TO FIND LATE LINE AXIS OF TAX PLATE LINE AXIS OF TAX		Ś	\$	311			∢	TENION IN THE PROPERTY OF THE		
MANY 66 313 Z ALZP HF ACCEL   PRILEASI WANDY 66 314   MALL HOUSE   INST TYPE   IRIG A TIME CODE AND VOICE   MALL HOUSE   INST TYPE   IRIG A TIME CODE AND VOICE   MALL HOUSE   INST TYPE   MALL HOUSE   INST TYPE   INST CHORD ROOF TRUSS   MOV 66 402 2 49 LF ACCEL   RRI CMTP CLG ROTT CHORD ROOF TRUSS   MOV 66 402 2 AT LF ACCEL   RRI CMTP CLG ROTT CHORD ROOF TRUSS   MOV 66 405 2 AT LF ACCEL   RRI CMTP CLG ROTT CHORD ROOF TRUSS   MOV 66 405 2 AT LF ACCEL   RRI CMTP CLG ROTT CHORD ROOF TRUSS   MOV 66 405 2 AT LF ACCEL   RRI CMTP CLG ROTT CHORD ROOF TRUSS   MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E WALL ME CRNR (E-W ACCEL)   MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E MOV 66 407 2 AC LF ACCEL   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 410 2 MLS PRESSURE   RRI CMTP CLG SUSP E MOV 66 503 3 ATH LF ACCEL   TOP STFEL COL WEST SIDE   M-S RACKING I M-S RACKING I MOV 66 504 3 ATH LF ACCEL   TOP STFEL COL WEST SIDE   M-S RACKING I M-S ROOF GIRDER AT LATER PORT   M-S RACKING I M-S ROOF GIRDER AT LATER PORT   M-S RACKING I M-S ROOF GIRDER AT LATER PORT   M-S RACKING I M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GIRDER AT LATER PORT   M-S ROOF GI		'n		315			લ	KIT-DR MOVABLE DR CMTR N WINDOW	- 1	
TRIG R TIME CODE AND VOICE   PATE   LINE CODE AND VOICE     DATE   CHAL HOUSE   INST TYPE   LOCATION     NOV 66 401 2 A5   LF ACCEL   ROLF PLATE LINE N FALL NF CORMER (N-S ACCEL)     NOV 66 402 2 A5   LF ACCEL   ROLF PLATE LINE N FALL NF CORMER (F-W ACCEL)     NOV 66 402 2 A5   LF ACCEL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 405 2 A7   LF ACCEL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 405 2 A7   LF ACCEL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 405 2 A7   LF ACCEL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 406 2 A7   LF ACCEL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 401 2 DISPL   ROLF PLATE LINE N WALL NF CRNR (F-W ACCEL)     NOV 66 411 2 DI DISPL   AND ACCEN   TO AS WITH SAME AXIS     NOV 66 411 2 DI DISPL   AND ACCEN   TO AS WITH SAME AXIS     NOV 66 411 2 DI DISPL   AND ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 502 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 503 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 504 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   LF ACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE   N-S RACKING     NOV 66 505 3 A3H   RACCEL   TOP STFEL COL SOUTH SIDE		1	1	313	ZAI	1	۲	TAST WINDS		
DATE				314				B TIME CODE AND VOICE		
WOUND 66 407 2 A5 LF ACCEL RRICHTP CLG ROTT CHORD ROOF TRUSS  NOV 66 407 2 A5 LF ACCEL RRICHTP CLG ROTT CHORD ROOF TRUSS  NOV 66 407 2 A5 LF ACCEL RRICHTP CLG ROTT CHORD ROOF TRUSS  NOV 66 407 2 A1 LF ACCEL ROOF PLATF LINE WALL IN CORMER IF-W ACCEL) I  NOV 66 407 2 A1 LF ACCEL ROOF PLATF LINE WALL NE CRNR IM-S ACCEL) I  NOV 66 407 2 A1 LF ACCEL ROOF PLATF LINE WALL NE CRNR IF-W ACCEL) I  NOV 66 407 2 A1 LF ACCEL ROOF PLATF LINE WALL NE CRNR IF-W ACCEL) I  NOV 66 407 2 A1 LF ACCEL ROOF PLATF LINE WALL NE CRNR IF-W ACCEL) I  NOV 66 407 2 ML2 PRESSURE RT ATTIC  NOV 66 407 2 ML2 PRESSURE RT ATTIC  NOV 66 410 2 ML2 PRESSURE RT ATTIC  NOV 66 410 2 ML2 PRESSURE RT ATTIC  NOV 66 411 2 DI  NOV 66 413 2 ML2 PRESSURE RT ATTIC  NOV 66 413 RRIGH ROOF ROOF RIPE RAIS  NOV 66 413 RRIGH ROOF ROOF ROOF ROOF ROOF ROOF ROOF ROO		C		HNH	HOUSE	<b>⊷</b>	ST			
5 NOV 66 407 2 A5 LF ACCEL ROOF PLATE LINE N VALL NF CORNER (N-S ACCEL) 1 5 NOV 66 402 2 A5 LF ACCEL ROOF PLATE LINE N TALL NF CORNER TE-W ACCEL 1 5 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE N VALL NF CORNER TE-W ACCEL 1 5 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE WALL NF CRNR (N-S ACCEL) 1 5 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE WALL NE CRNR (N-S ACCEL) 1 5 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE WALL NE CRNR (N-S ACCEL) 1 5 NOV 66 407 2 Accel ROOF ROOF ROOF ROOF ROOF ROOF ROOF ROO		ļ.	1		TNST				n (	
NOV 66 402 2 A9		Ŗ,		t 0 1	2 A5		⋖	PLATE LINE N EALL NE CORNER (N-S	M: (	
5 NOV 66 507 3 A2 H LF ACCEL ROOF PLATE LINE " WALL NF CORNER TE W ACCEL) I 5 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE N WALL NE CRNR (H-S ACCEL) I 6 NOV 66 405 2 A7 LF ACCEL ROOF PLATE LINE N WALL NE CRNR (H-S ACCEL) I 7 NOV 66 407 2 A2 LF ACCEL ROOF PLATE LINE WALL NE CRNR (F-W ACCEL) I 7 NOV 66 407 2 A2 LF ACCEL ROOF PLATE LINE WALL NE CRNR (F-W ACCEL) I 7 NOV 66 407 2 ML2 PRESSURE BRI CNTR CLG SUSP 6 FT ASV FLR 7 NOV 66 410 2 ML4 PRESSURE BRI CNTR CLG SUSP 6 FT ASV FLR 7 NOV 66 411 2 D1 D15PL ADJACENT TO A6 WITH SAME AXIS 7 NOV 66 413 2 D2 D15PL ADJACENT TO A6 WITH SAME AXIS 7 NOV 66 413 2 D2 D15PL ADJACENT TO A6 WITH SAME AXIS 7 NOV 66 413 2 D2 D15PL ADJACENT TO A6 WITH SAME AXIS 7 NOV 66 413 2 D2 D15PL ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A1H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL SOUTH SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL NEST SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL NEST SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL NEST SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL NEST SIDE N-S RACKING I 7 NOV 66 501 3 A3H LF ACCEL TOP STEL COL NEST SIDE NOV 66 506 SOUTH SIDE NOV 66 506 SOUTH SIDE NOV 66 506 SOUTH SIDE NOV 66 507 SILL STRAIN ROOT FLANGE ROOF PURLIN AT CENTERLINE I 7 NOV 66 507 SILL STRAIN ROOT FLANGE ROOF PURLIN AT CENTERLINE I 7 NOV 66 510 SILL ROOT FLANGE ROOF DURLIN AT CENTERLINE I 7 NOV 66 510 SILL ROOT FLANGE ROOF DURLIN AT CENTERLINE I 7 NOV 66 510 SILL ROOT FLANGE ROOF DURLIN AT CENTERLINE I 7 NOV 66 51				402	2 A9			CMTP CLG BOTT CHORD RO	E. T	
5 NOV		1	1	404	2 45		i	PLATE LIME - WALL TO	· ·	
## NOV 66 405 2 A7 LF ACCEL				かしな	2 A1:	_		WALL MIN HT CNTR STUD	(°) (	
NOV				405	2 A7		F ACCFL	FLR PLATE LINE N WALL NE CRNR	1 3	
NOV 66 407 2 Ac		ļ	ł	400	ĺ	l	ACCE!	M WALL MID HT CNIR SIUN		
5 NOV 66 408 2 ML2 PRESSURE RITWN LR AND DR SUSP 6 FT ASV FLR  5 NOV 66 409 2 ML2 PRESSURE RAI ATTIC  5 NOV 66 410 2 ML4 PRESSURE RAI ATTIC  5 NOV 66 410 2 DL DISPL ADJACENT TO AS WITH SAME AXIS  5 NOV 66 410 2 DL DISPL ADJACENT TO AS WITH SAME AXIS  5 NOV 64 414  5 NOV 66 419  5 NOV 66 419  7 NOV 66 419  7 NOV 66 501 3 A1H LF ACCEL TOP STFEL COL INTERIOR OF RLDS E-W RACKING  5 NOV 65 501 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  5 NOV 65 503 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  5 NOV 65 503 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  5 NOV 65 503 A3H LF ACCEL CENTER OF ROOF GIRDER AT 174 POTHY  5 NOV 65 505 A3 A3H LF ACCEL CENTER OF ROOF GIRDER AT 174 POTHY  5 NOV 65 507 3 SIL STRAIN  6 NOV 65 507 3 SIL STRAIN  7 NOV 66 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT 60 507 3 SIL STRAIN  7 NOT				407			- ACCEL	FLR PLATE LINE E WALL NE CRNR (E-W ACCEL)		•
NOV 66 410 2   NL3   PRESSURE   REI CNTR CLG SUSP 2 IN RELOW CLG   1				807			RESSURE	AND DR SUSP 6 FT ASV FLR		
NOV 66 410 Z ML4 PRESSURE BRI CRIR CLS SISP Z INTEGRAL CLS INTEGRAL SAME AXIS  NOV 66 411 Z D1 DISPL ADJACENT TO A5 WITH SAME AXIS  NOV 66 414  NOV 66 501 3 A2H LF ACCEL TOP STFEL COL INTERIOR OF RLDS E-W RACKING I NOV 65 501 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 504 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 504 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 505 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 506 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 65 507 3 A3H LF ACCEL TOP STFEL SIDE  NOV 65 507 3 A3H LF ACCEL TOP STFEL SIDE  NOV 65 507 3 A3H LF ACCEL TOP STFEL SIDE  NOV 65 507 3 A3H LF ACCEL TOP STFEL SIDE  NOV 65 507 3 A3H LF ACCEL TOP STFE		ļ	ł	607	1	1	KESSURE.	ATTIC		
5 NOV 66 411 2 D1 DISPL ADJACENT TO AS WITH SAME AXIS  5 NOV 66 412 2 D2 DISPL ADJACENT TO AS WITH SAME AXIS  5 NOV 66 414  5 NOV 66 414  5 NOV 66 414  7 NOV 66 501 3 A3H LF ACCEL TOP STEEL COL INTERIOR OF RLDS E-W RACKING I STADOV 65 501 3 A3H LF ACCEL TOP STEEL COL SOUTH SIDE  5 NOV 66 503 3 A3H LF ACCEL TOP STEEL COL SOUTH SIDE  5 NOV 66 503 3 A3H LF ACCEL TOP STEEL COL SOUTH SIDE  5 NOV 66 503 3 A3H LF ACCEL TOP STEEL COL WEST SIDE  5 NOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 66 503 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL  5 NOV 65 505 3 A3H LF ACCEL CENTER OF SIDE AT LIVE POINT  5 NOV 65 507 3 SIL STRAIN ROTT FLANGE ROOF GIRDER AT LIVE POINT  5 NOV 65 510  7 NOV 65 510  8			•	0			SESSURE	CATA CLG SJOY A IN MILOW CLG		
NOV 65 412 2 D? DISPL SATSCENI TO BE WITH SAME AXIS  5 NOV 66 413  FRIG B TIME CODE AND VOICE  5 NOV 66 413  FRIG B TIME CODE AND VOICE  5 NOV 66 501 3 A1H LF ACCEL TOP STFEL COL INTERIOR OF RLDG E-W RACKING I  5 NOV 65 502 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  5 NOV 65 503 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  5 NOV 65 504 3 A4H LF ACCEL TOP STFEL COL WEST SIDE  5 NOV 65 505 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  6 NOV 65 505 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  7 NOV 65 507 3 SIL STRAIN  7 NOV 65 507 3 SIL STRAIN  7 NOV 65 507 3 SIL STRAIN  7 NOV 65 508 3 S2L STRAIN  7 NOV 65 509 3 S3L STRAIN  7 NOV 65 509 3 S3L STRAIN  7 NOV 65 510  7 NOV 65 511  7 NOV 65 511  7 NOV 65 512  7 NOV 65 513 3 MA PRESSURF FXTERIOR ABV NOOF  7 NOV 65 514  7 NOV 65 514  7 NOV 65 514  7 NOV 65 514  7 NOV 65 515  7 NOV 65 517  8 NOV 65 517  8 NOV 65 518  8 NOV 65 518  8 NOV 65 519  8 NOV 65 510  8 NOV 65 519  8			9	411			SPL	TO AS WITH SAME AXIS		
SPARE SNOV 66 414  SPARE SNOV 66 414  SPARE SNOV 66 414  SNOV 66 414  SNOV 66 501 3 A1H LF ACCEL TOP STFEL COL INTERIOR OF RLDS E-W RACKING 1  SNOV 65 502 3 A2H LF ACCEL TOP STFEL COL SOUTH SIDE  SNOV 65 503 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  SNOV 65 503 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  SNOV 65 504 3 A4H LF ACCEL TOP STFEL COL WEST SIDE  SNOV 65 504 3 A4H LF ACCEL CENTER OF ROOF GIPDER AT CENTERLINE  SNOV 65 505 3 A3H LF ACCEL CENTER OF ROOF GIPDER AT CENTERLINE  SNOV 65 506  SNOV 65 506  SNOV 65 507 3 S1L STRAIN  SNOT FLANGE ROOF GIPDER AT CENTERLINE  SNOV 65 509 3 S3L STRAIN  ROTT FLANGE ROOF GIPDER AT CENTERLINE  SNOV 65 509 3 S3L STRAIN  SNOV 65 509  SNOV 65 510  SNOV		1	ł	415		1	SPC	TO AS WITH SAME AXIS		
FOUND 66 414  DATE CHALL HOUSE 1WST TYPE  PATE CHALL HOUSE 1WST TYPE  NOV 66 501 3 A1H LF ACCEL TOP STFEL COL INTERIOR OF RLDS E-W RACKING 1  NOV 66 502 3 A2H LF ACCEL TOP STFEL COL SOUTH SIDE  NOV 66 503 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE  NOV 66 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 66 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE  NOV 66 503 3 A3H LF ACCEL CENTER OF ROOF GIPDER AT CENTERLINE  NOV 66 503 3 A3H LF ACCEL CENTER OF ROOF GIPDER AT CENTERLINE  NOV 66 505  NOV 66 505  NOV 66 507 3 S1L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE  NOV 66 510  ALANX  NOV 66 510  ALANX  NOV 66 513 3 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV ROOF  NOV 66 513 9 M4 PRESSURF FYTERIOR ABV R				Γ-,						
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Y MO YR INSTR 5 NOV 66 501 3 AlH LF ACCEL TOP STFEL COL INTERIOR OF RLDG E-W RACKING 1 5 NOV 66 502 3 A2H LF ACCEL TOP STFEL COL SOUTH SIDE 5 NOV 66 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE 5 NOV 66 503 3 A3H LF ACCEL TOP STFEL COL WEST SIDE 5 NOV 66 504 3 A4H LF ACCEL CEMTER OF ROOF GRDR HORZ ACCEL 5 NOV 66 506 5 NOV 66 506 5 NOV 66 506 5 NOV 66 507 3 SIL STRAIN BOTT FLANGE ROOF GIRDER AT LY4 POINT 5 NOV 66 509 3 S2L STRAIN ROTT FLANGE ROOF GIRDER AT LY4 POINT 5 NOV 66 509 3 S3L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE 5 NOV 66 510 5 NOV 66 510 6 NOV 66 513 3 MA PRESSURF INTERIOR ARV NOOF 5 NOV 66 513 3 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 66 513 3 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 66 513 3 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 65 513 3 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 65 514 GROSSURF FYTERIOR ARV NOOF 6 NOV 65 517 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 65 517 MA PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF 6 NOV 65 518 AND PRESSURF FYTERIOR ARV NOOF					1000 1000	T	J	LOCATION		
5 NOV 66 501 3 AIH LF ACCEL TOP STFEL COL INTERIOR OF RLDS E-W RACKING I SNOV 65 502 3 A2H LF ACCEL TOP STFEL COL SOUTH SIDE E-Y RACKING I NOV 66 503 3 A3H LF ACCEL TOP STFEL COL SOUTH SIDE N-S RACKING I NOV 66 504 3 A4H LF ACCEL TOP STFEL COL WEST SIDE N-S RACKING I NOV 66 505 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL ON SACKING I NOV 66 506 3 A3H LF ACCEL CENTER OF ROOF GRDR HORZ ACCEL ON SACKING I NOV 66 506 3 S2L STRAIN BOTT FLANGE ROOF GIRDER AT 1/4 POINT I NOV 66 509 3 S3L STRAIN ROTT FLANGE ROOF GIRDER AT 1/4 POINT I NOV 66 510 3 S3L STRAIN ROTT FLANGE ROOF GIRDER AT 1/4 POINT I NOV 66 510 3 S3L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE I NOV 66 510 3 S3L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE I NOV 66 510 3 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTERIOR ARV NOOF I NOV 66 513 MV PRESSURF FXTER		>	<b>&gt;</b>		INST					
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5 NOV 66 507 3 SIL STRAIN BOTT FLANGE POOF GIPDER AT CENTERLINE 1 5 NOV 66 508 3 S2L STRAIN ROTT FLANGE ROOF GIRDER AT 1/4 POINT 1 5 NOV 66 509 3 S3L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE 1 5 NOV 66 510 5 NOV 66 517 3 W2 PRESSURF INTERIOR 3 FT BFLOW ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV NOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV NOF 6 CHN HOUSF INST TYPE LOCATION PP		S		506						
5 NOV 66 508 3 S2L STRAIN ROTT FLANGE ROOF GIRDER AT 1/4 POINT 1 5 NOV 66 509 3 S3L STRAIN ROTT FLANGE ROOF PURLIN AT CENTERLINE 1 5 NOV 66 510 6 NOV 66 517 3 M2 PRESSURF INTERIOR 3 FT BFLOW ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV ROOF 6 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 514 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 517 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 517 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 7 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 8 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 8 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 8 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 8 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF 8 NOV 66 518 3 M4 PRESSURF FXTERIOR ARV ROOF		ı.		507			FRAIN	POOF GIPDER AT CENTERLINE		
5 NOV 66 509 3 S3L STRAIN ROTT FLANSE ROOF PURLIN AT CENTERLINE I A NOV 66 510 5 NOV 66 511 5 NOV 66 512 3 M2 PRESSURF INTERIOR ABV ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ABV ROOF 5 NOV 65 513 3 M4 PRESSURF FXTERIOR ABV ROOF 6 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF 7 NOV 65 514 3 M4 PRESSURF FXTERIOR ABV ROOF		h	ı	208			AA I N	ROOF GIRDER AT 1/4 POINT	1	
5 MOV 66 510 5 MOV 66 511 5 NOV 66 512 3 M2 PRESSURF INTERIOR 3 FT BFLOW ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ABV ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ABV ROOF 6 NOV 65 514 7 NOV 65 517 7 NOV		Ŋ		503			FRAIN	ROOF PURLIN AT CENTERLINE		
F NCV 56 511 5 NOV 66 512 3 M2 PRESSURF INTERIOR 3 FT BFLOW ROOF 5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV, ROOF 5 NOV 65 513 3 M4 PRESSURF FXTERIOR ARV, ROOF 5 NOV 65 514 6 NOV 65 514 7 NOV 65 514 7 NOT 65 514 7 NOT 65 514 7 NOT 65 514 7 NOT 65 514		ĸ		530						
5 NOV 66 512 3 M2 PRESSURE INTEXIOR 3 FT BELOW ROOF 5 NOV 66 513 3 M4 PRESSURE FXTERIOR ABV ROOF 5-NOV 55 514 DATE CHNL HOUSE INST TYPE LOCATION PATE CHNL HOUSE INST TYPE		1	1	715	:				_	
5 NOV 66 513 3 M4 PRESSURF FXTERIOR ARV ROOF 5-NOV 55 514 DATE CHNL HOUSF INST TYPE LOCATION P				517	2	Ď.	(1)	3 FT BELOW ROOF		
DATE CHNL HOUSE INST TYPE LOCATION P				513	Σ	۵.	u;		۲.	
TOTAL TOTAL			ł	# (F	21.01	٠		NOTEROOT LINE OF STATE		
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# APPENDIX F

# SUMMARY OF FREE FIELD OVERPRESSURE DATA

Contents		Page
Legend		F.1
Figure F-i	Sonic Boom Waveform Categ is	F.3
Table F-I	Summary of Data from Free Field Microphones located near E-2, Phase i.	F.4
Table F-2	Summary of Data from Free Field Microphones located near L-2, Phase 1.	F.21
Table F-3	Summary of Data from Free Field Microphones located near E-2, Phase II.	F.26

## LEGEND

The following is an explanation of notations and abbreviations used in this Appendix:

# <u>Summary of Data from Free Field Microphones located near</u> E-2, Phase I.

Date Month, Day, Year.
 Mission No. Mission identification number
 Altitude Altitude above mean sea level in feet.
 Microphone No. See Legend and Figure B-7 Appendix B.
 Δp Peak positive overpressure
 Δt Time from start of boom to negative peak

Table F-2 Summary of Data from Free Field Microphones located near L-2, Phase 1.

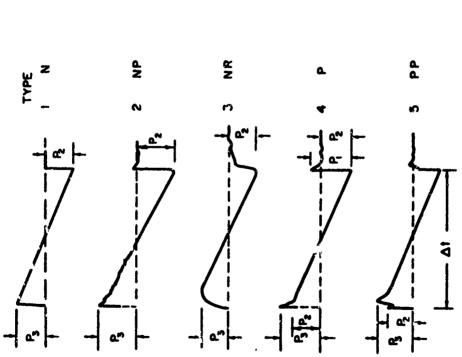
in seconds (Figure F-I)

Date Month, Day, Year.
 Mission No. Mission identification number
 Altitude Altitude above mean sea level in feet.
 Average Peak Average peak positive overpressure in psf. overpressure

# <u>Table F-3</u> <u>Summary of Data from Free Field Microphones located near E-2, Phase II.</u>

I. MSN Mission identification number 2. CHNL Tape recorder-channel number, Appendix B. 3. HOUSE, House number and instrument designation, INSTR. Figure B-7, Appendix B. 4. TYPE Waveform type code number, Figure F-1. Peak amplitudes, psf, Figure F-1. 5. PEAK **AMPLITUDES** Rise Time, sec. 6. RISE TIME 7. PERIOD Time from start of boom to negative peak, sec, (Δt, Figure F-I). 8. WAVE ANGLE Angle between overpressure wave front and ground, degrees. 9. GNO SPD Wave front speed at ground level, ft/sec.

FIG. F-1 SONIC BOOM WAVEFORM CATEGORIES



F.3

Table F-1
SUMMARY OF DATA FROM FREE FIELD MICROPHONES LOCATED NEAR E-2, PHASE I

SUMMAR	Y UF DA	IA FROM	FREE FIE	Ci) MI	CROPHONES L	OCATED I	VEAR E-	2, PHASE
Date	Mission No.	Aircraft	Altitude :	Mach No.	Microphone No. **	Δp 1b/ft <sup>2</sup>	Δt sec.	Rise Time sec.
6-4-66	14	XB-7'J	52,920	1.81	MLC-1	2.37	. 250	.0125
					MLC-5			i
					MLC-6	1.36		
		ľ	<u> </u>		MLC-2 MLC-3	2.59 2.72	.250 .250	.007
					MLC-4	2.42	.250	.0035
1				· '			1	
6-3-66	22	X3-70	72,000	2.83	MLC-1	1.65	.3175	.0055
			ļ.	ł	MLC-5	1.64	.3175	.007
			ł	Ì	MLC-6 MLC-2	1.53	.3175	.005
1		Ì	1		NLC-3	1.68	.3175	.005
	1	l	1	}	MLC-4	1.70	.3175	.007
	[	•		ĺ	ĺ	1	[	
6-8-66	1	XB-70	31,850	1.38	MLC-1	Noise		
		l	1	l	MLC-5	2,35	. 233	.03
	1	l	1	1	NE.C-6 NE.C-2	2.10	.234	.032
1		ł	ł	ł	MLC-3	2.08	.233	.03
1				•	NLC-4	2.38	.234	.028
	1	1	1	į		1		1
6-6-66	39		No Bo	on m	ļ	1		ĺ
į.	70	B-58	43,900	1.6	\ waa 1	1.97		.005
1	1 70	B-36	43,900	1.8	MLC-1 MLC-5	1.88	.185	.024
1	1	l	i	į	MLC-6	1.01		
į	l		1	[	MLC-2	2.23	. 185	.002
1		1	1	1	MLC-3	1.72	. 185	.007
1		}		1	NLC-4	1.98	. 1845	.023
1	40	B-56	31,400	1.48	MLC-1	3.55	.1573	.010
	1 **	1 5-35	32,100	1	MLC-5	3.36	.157	.0115
	1	1		Į	MLC-6	1.78		
	j	]	ł	1	MLC-2	3,21	.157	.007
	}	1	1	1	MLC-3	3.63	. 157	.0065
			1	ļ	X0.C-4	3.52	.157	.015
i	71	58-د	44,200	1.59	MLC-1	1.65	.179	.012
	! ''		1,	1	MLC-5	1.88	.179	.017
	1	1	1	ì	NLC-6	.930	[	
1		1	1	}	MLC-2	1.72	.179	.012
i	1	1		1	MCC-3	1.75	.178	.006
	1	}		1	MLC-4	1.78	.179	.016
1	41	B-58	31,340	1.45	NLC-1	2.49	.154	.016
1	1		,		MLC-5	3.56	.154	.017
1	1	1			MLC-6	1.24		
	1		1	1	MLC-2	2.33	. 154	.015
1	i	}		1	MLC-3	2.43	.154	.018
1		Ţ	1	1	NLC-4	2.64	.1535	.016
1	72	B-58	43,920	1.55	MLC-1	1.51	.172	.006
	-			1	MLC-5	2, 64	.172	.005
	1		1	1	MC-6	1.63		
-	1	Į	1	1.	MLC-2	2.09	.174	.001
1				1	NLC-3	2.02	.172	.003
<u> </u>	<u></u>		1		MC-4	1.78	.171	.005

<sup>1)</sup> Refer to Legend, Page F.I for explanation of motations and abbreviations. F.4

Table F-1 (Continued)

TO THE TO DESCRIPTION OF THE PROPERTY OF THE P

Date	Mission No.	Aircraft	Altitude ft	Mach No.	Microphone No. **	Δp lb/ft <sup>2</sup>	∆t sec.	Rise Time
6-6-66	43	B-58	Missed B	DOM A				
	74	B-58	32,440	1.3	MLC-1	3.16	. 195	.014
		2 00	02,110		MLC-5	3.20	.194	.010
					:a.c-6	1.67		
					MLC-2	3.12	. 194	.001
'					MLC-3	3.33	. 1945	.006
					MLC-4	3.09	.194	.009
	44	B-58	43,400	1.57	MLC-1	1.58	. 197	.007
				ĺ	MLC-5	1.96	. 196	.0005
					MLC-6	1.16		
					MLC-2	1.53	. 196	.006
	}	ļ			MLC-3	1.65	.195	.0005
					MLC-4	1.90	. 1955	.004
	75	B-58	31,840	1.46	MLC-1	2.67	. 157	.006
					MLC-5	3.00	. 1575	.004
				1	MLC-6	2.02		
				<u> </u>	MLC-2 MLC-3	3.02 4.94*/3.33	. 157 . 157	.001 .0005*/.001
					MLC-4	3.05	.1575	.0035
				1		3.00	. 10.0	.0035
	42	B-58	43,300	1.53	MLC-1	1.83	. 1835	.0065
			10 N. mi	East	MLC-5	1.80	. 183	.0065
			,	1	MCL-6	.930		••
				1	MCL-2	1.58	. 183	.007
			I	l	MLC-3	1.65	.1825	.011
				<u> </u>	MCC-4	1.98	. 1835	.0065
	73	B-58	31,860	1.43	MLC-1	2.95	.160	.506
				•	MLC-5	5.44*/3.72	.160	,0005*/.001
				ł	MLC-C	2.29		**
					MLC-3	3.12 3.03	.160 .160	.0005
					MLC-4	3.25	.160	.004
	~ .							
6-7-66	76-A	B-58	31,560	1.48	MLC-1	2.88	.164	.0065
					MLC-5	2.61	.1635	.006
				1	MLC-2	1.61 3.10	. 164	.008
				1	MCC-3	4.51	. 164	.0015
				l	MLC-4	3.47	.1635	.004
	45-B		40.400			l		
	40-R	<b>3-5</b> 8	43,660	1.70	MLC-1	1.75	.1715	.005
			l		MLC-5	2.01 1.05	.172	.0085
			ì .		MLC-2	2.29	.171	.001
			i i	l	MLC-3	2.27	.172	.0055
					MLC-4	1.96	.171	.009
	77-B	B-58	31.680	1.51	MLC-1	0.40	,,,	١
	11-5	B-30	31.000	1	MLC-1	2.48 2.75	,156	.011
			l	1	MLC-6	1.48	. 156	.010
			Ī	•	MCC-2	3.26	. 155	.005
				1	MLC-3	3.24	. 156	.005
		ŀ	i	l	HLC-4	2.71	. 1565	.027

Table F-1 (Continued)

Date	Mission No.	Aircraft	Altituda ft	Mach No.	Microphone No.**	Δp lb/ft <sup>2</sup>	∆t sec.	Rise Time
6-7-60	46-B	B-58	43,720	1.65	MLC-1	1.35	. 1715	.0005
			l		MLC-5	1.82	. 172	.011
				Ī	MLC-6	. 84		
				1	MLC-2	1.40	. 171	.003
		1		l	MLC-3	1.81	. 170	.006
			1		MLC-4	1.71	. 172	.00€
	48-A		No Bo	) (00) (	1			
	79-A	3-58	31,600	1.52	MLC-1	2.57	. 170	.028
		l	1		NLC-5	2.49	. 1695	.029
			l		MLC-6	1.16		
			l	1	MC-2	2.45	. 169	.027
		i	]		MLC-3	2.45	. 1695	.014
			İ		MLC-4	2.66	. 169	.017
	49-A	B-58	43,340	1.43	MLC-1	1.41	.211	.040
		l	l.	1	MLC-5	1.49	. 212	.032
		Į		1	MLC-6	1.42		
	l				MLC-2	1.33	. 2075	.024
				1	MLC-3	1.39	.212	.045
					MLC-4	1.59	. 2115	.035
	80-A	B-58	31,600	1.53	MLC-1	2.59	. 156	.0085
		l		ł	MLC-5	2.59	. 1555	.0115
				l	MLC-6	1,35		
		ŀ	ŀ	ł	MLC-2	3.10*/2.48	. 1555	.001/.003
		1	l	l	MCC-3	2.60	. 1565	.019
					NE.C-4	3,11	. 1555	.014
	50-A	3-58	43,340	1.43	MLC-1	, 930	. 197	.0105
	t I		i	į	MLC-5	. 938	. 192	.020
			i	l	MLC-6	. 483		
			İ	1	MLC-2	1.02	. 197	.045
			1	j	MLC-3	.908	. 1995	.023
					MLC-4	1.15	. 196	.049
	81-A	B-58	31,400	1.49	MC-1	1.75	. 151	.053
				1	MLC-5	2.07	. 1505	.042
			ĺ	ł	NE.C-6	.516		
	İ	ĺ		:	MLC-2	1.80	. 150	.050
			ļ		MLC-3	1.97	. 151	.034
			i		MIC-4	2.29	. 150	.047
6-8-66	43-A	B-58	42,380	1.62	MLC-1			
		l	•		MLC-5	1.70	. 177	.015
		l	ì		MLC-6	1.53		
					MLC-2	1.74	.174	.012
			Í	ĺ	MLC-3	1.73	. 176	.014
			1		MLC-4	1.63	. 175	.012
	75-A	B-58	31,200	1.44	MLC-1			
	ļ		!		MLC-5	3.52	. 156	.0055
			İ	l	2E.C-6	1.75		
			<b>i</b> .	l	MLC-2	3.18	. 156	.0115
					MCC-3	3.37	. 1565	.009
			1	1	MLC-4	€.15	. 157	.007

Table F-1 (Continued)

Dute	Mission No.	Aircraft	Altitude ft	Mach No.	Microphone No. **	Δp lb/ft <sup>2</sup>	Δt sec.	Rise Time
6-8-66	42-A	B-58	43,260	1.67	₩.C-1			
		1	]		MLC-5	2.09	.179	.009
					MLC-6	1.18		
		Ì	i .		MLC-2	2.73	.179	.006
			1		MLC-3	2.34	.179	.0035
					MLC-4	2.06	.179	.008
	73-A	B-58	31,300	1.5	MLC-1			
					MLC-5	2.35	. 147	,0155
				}	MLC-6	1.23		
					MLC-2	2.23	.147	.011
					MLC-3	2.16	.146	.014
					MLC-4	3.23	.147	.016
	41-A	B-58	43,200	1.6	MLC-1			
			<i>'</i>		MLC-5	1.74	.166	.006
					12.C-6	.963		
				l '	MLC-2	3.03	.166	.005
					MLC-3	1.82	.166	.006
					MLC-4	1.91	. 167	.006
	72-A	B-58	31,200	1.49	MLC-1			
					MLC-5	2.96	. 144	.006
ļ		1	Į į		MLC-6	1.58		
					MLC-2	2.88	.145	.004
		İ		i	MLC-3	3.24	.144	.002
					MLC-4	2.55	.145	.004
	57-RB	B-58	37, <b>6</b> 00	1.66	MLC-1			
	0	2 00	31,000		MLC-5	1.78	.161	.023
					MLC-6	.832		
				i	MLC-2	2.18	.162	.003
i					HLC-3	1.51	. 163	.930
					MLC-4	1.67	.162	.0085
	80-RB	B-58	31,300	1.46	MLC-1			
i	30	<b>3</b> -00	01,000	••••	MLC-5	2.52	.161	,005
					MLC-6	1.31		
1					MLC-2	2,58	.160	.014
					MLC-3	2.64	.160	.0075
					NLC-4	3.15	.161	.0025
	56-RB	B-58	43,040	1.64	MLC-1			
	20-10	5-36	45,040	1.0	MLC-5	2.61	.171	.004
					MLC-6	1.40		.004
					MLC-2	2.08	.171	.0135
					MCC-3	1.90	.169	.008
					MLC-4	2.06	.171	.0065
	87-RB	B-58	31,440	1.49	MLC-1			
	01-KB	5-36	31,770	****	MLC-5	3.09	.148	.0175
					MLU-6	1.66	. 140	
					MLC-2	4.27	.148	.001
				li	MLC-3	2.81	.148	.008
	I		i		M.C-4	3.19	.148	.017

Table F-| (Continued)

	Mission		Altitude	Mach	Microphone	Λb	Δŧ	Rise Time
Date	No.	Aircraft	ft	No.	No. **	Δp 1b/ft <sup>2</sup>	sec.	sec.
6-8-66	55-RB	D-58	43,200	1.64	MLC-1			
					MLC-5	2,18	.170	.003
			i l		MLC-6	1,71		
i					MLC-2	2.63	.169	.0125
					14LC-3	2.68	.166	.0015
1					MLC-4	2.06	. 169	.0055
	86-RB	B-58	31,360	1.49	MLC-1			
1	OU-ND	B-30	32,300	1	MLC-5	2.87	.144	.009
				i	MLC-6	1.62		
		]		Ì	MLC-2	2.63	.144	.011
•		ł	1	l .	MLC-3	3.03	.144	.0055
	[	[	1	•	MLC-4	2.48	.144	.006
l	İ			Ì	1	<u> </u>		
6-9-66	86-SRB	B-58	31,000	1.5	MLC-1	3.52	.153	.0055
•	[	(	[		MLC-5	3.72	. 153	.005
l	į		]	1	MLC-6	1.94		
1	<b>[</b>			1	MLC-2	4.09	. 153	.6045
į	}	]	1	I	MLC-3	5.32	. 152	.005
1	•		ļ	i	MLC-4	3.31	.1525	.6`4
ł				1		1	1205	
ì	55-SRB	B-58	35,720	1.69	MLC-1	1.42	.1395	.033
ł	1	1	1	i	MLC-5	1.46		.030
ł	1	}	1	ł	MLC-2	1.43	14.05	.030
ł	i	ł	ł	l	MLC-3	1.75	.1395	.0085
i	]	ļ		l	MLC-4	1.56	1405	.031
	i		1	Î		1	''''	
İ	87-SRB	B-58	31,000	1.53	NLC-1	3.02	. 147	.015
ł		1	1		MLC-5	2.93	.146	.006
Į.	i	l		I	MLC-6	1.58		
i	i	f	1	ĺ	MLC-2	3.12	.1455	.005
1	l	ì	Į		MLC-3	3.72	. 1465	.006
1	į	l		l	MEC-4	4.02	.146	.001
l			1			i		
ł	56-SRB	B-58	43,300	1.72	MLC-1	3.11	.1605	.002
l		1	1	İ	MLC-5	2.64	.161	.005
l	}	ł	l	1	MLC-6	1.34	1010	.0035
1		ŀ	1	1	MLC-3	2.98	.1615	.0075
		l	ļ	1	NLC-4	2.63	.161	.004
1	1	1	I	i		1	1	
1	80~SRB	B-58	31,000	1.53	MLC-1	2.79	.1405	.006
			1		MLC-5	3.12	.140	.007
	]		I	1	MLC-6	2.18	-	-
1	1	i	1		12.C-2	2.46	.140	.021
l	]	]	j	}	MLC-3	3.61	.140	.003
1	I	ł	ì	1	MLC-4	2.63	.1405	,024
l				1				
J	57-8RB	B-58	43,100	1.70	MLC-1	1.60	. 1505	.0085
I		1	1	}	MLC-5	1.56	.1495	.0055
l		1	1	1	MLC-6	. 838		
1	1	l	l	l	MLC-2	1.99	.150	.012
1		1	I		MLC-3	2.12	.150	,004
L	<u></u>	<u> </u>	<u> </u>	<u></u>	MLC-4	1.94	.150	.018

Table F-1 (Continued)

The state of the s

Date	Mission	Aircraft	Altitude	Mach	Microphone No.##	Δp 1b/ft <sup>2</sup>	۸t sec.	Rise Time
	No.		ft	No.	NO. "	16/10	8ec.	sec.
6-9-66	41-SA	B-58	42,920	1.52	MLC-1	1.75	. 180	.011
					NILC-5	2.93	.1805	.001
			l		MLC-6	1.74		
					MLC-2	1.79	.1805	.005
			l		MLC-3	2.23	.181	.0045
					MLC-4	2.19	.1805	.002
	73-SA	B-58	31,720	1.50	MLC-1	3,05	.156	.017
			)		MLC-5	2,83	.1555	.0045
				1	MLC-6	1.47		
					MLC-2	2.69	. 155	.0045
	!				MLC-3	3.61	.155	.014
					MLC-4	2,76	. 155	,018
į	42-SA	B-58	43,060	1.52	MLC-1	1.99	. 1755	.015
			, '	i	MLC-5	2.04	. 176	.018
1		ļ	1	}	MLC-6	1.21	] '	
į				l	MLC-2	2,23	.176	.005
	•		ŀ	1	MLC-3	2.49	.176	.0175
					MLC-4	2.08	.176	.0015
	75~SA	B-58	31,680	1.55	MLC-1	3.68	.149	.003
1	10-54	B-30	31,500	1	MLC-5	4.01 4/3.34	.1485	.001 /.005
.]				1	MLC-6	1,81		
}	1	ļ	į		MLC-2	2.99	.1488	.003
1	İ			1	MLC-3	4.24	.1485	.012
1	1				MLC-4	3.78	.149	.004
			Not	 :e 72	l -SA Abortec			
1		ŧ	Į	1	ĺ	l		ł
	43-SA	B-58	43,000	1.68	MLC-1	3.50	. 157	.003
1	1			I	MLC-5	2.35	. 1565	.001
	1	1		1	NLC-6	1.17		
			1	1	MLC-2	2.99	. 157	.004
<u> </u>	l			1	MLC-3 MLC-4	2.31 3.01	.157	.001
Į.			1	1	MLZ-4	3.01	1.137	
	42-SA	B-58	43,300	1.70	HLC-1	1.87	.1645	.007
1	ļ		į	1	MLC-5	2.07	. 166	.011
	l	Į	1	1	MLC-6	1.01		
1		j	ĺ	1	MLC-2	1.66	. 1645	.017
				i	MLC-3	2.05	. 1655	.011
}	1				IE.C-4	1.81	1.1665	.013
	46-SA	B-58	42,900	1.68	MC.C-1	1.69	. 156	.022
1	1	1		1	MLC-5	1.69	. 1555	.008
1	1	į .	i	ļ	MLC-6	.972		
I	i		į	1	ELC-2	2.26	. 1563	.007
I	Į.	1	!	1	MC-3	2.83	. 156	.006
	]			1	MLC-4	1.97	. 1565	.0205
	72-8A	B-58	£1,320	1.53	MLC-1	2.19	. 1455	,0145
i			1	1	MLC-5	2.29	.145	.016
		1		ł	MLC-6	1.17	- '	-
1			I	Ì	MLC-2	1.89	.145	.0095
ł		1	i	1	ME.C-3	2.57	. 145	,017
1	1	l	1	1	MLC-4	2.16	. 1455	.019

Table F-1 (Continued)

Date	Mission		Altitude	Mach	Microphone No.**	Δp lb/ft <sup>2</sup>	Δt	Rise Time
	Ko.	Aircraft	It	No.	No.""	16/11	sec.	sec.
6-13-66	18-A	B-58	37,740	1.64	MLC-1	2,59	.1605	.005
				l	MLC-5	3.36 /2.77	.1605	.0004/.0008
				[	MLC-6	1,85		
				j	MLC-2	2,71	.160	.0035
	Į.		•	1	MLC-3	2.83	.160	.0003
	•			1	MLC-4	2,78	.160	.004
	18-B	B-58	49,600	1.66	MC-1	2.16	. 1935	.0005
	ĺ	۰	İ	Ĭ	NLC-5	1.96	. 1955	.003
	1	i	1	1	MLC-6	1,04		
	l		l .	1	MC-3	1.88	. 195	.0055
	ĺ		[	[	MLC-3	2.00	. 1955	.007
					NLC-4	2.31	. 1955	.0035
	21-A	B~58	37,840	1.69	MLC-1	3,00	.1455	.0005
		]	. ,	1	MLC-5	2.55	.146	.CO65
	ĺ	1	•	1	MLC-6	1.34		
į	(	Í	I	1	MLC-2	2.76	.146	.0035
•	1		Ĭ		MLC-3	2.98	.146	.004
	1	<b>[</b>			ME.C-4	2.94	. 146	,005
	21-B	B-58	49,160	1.72	MLC-1	1.83	. 195	.0045
İ		1	1,	1	MILC-5	1.84	.195	.004
	1		Ì	1	MLC-6	.936		
	1	[	1	ĺ	NLC-2	1.83	. 1945	.0045
l	ł	Î	ĺ	i	MLC-3	1.98	. 195	.004
	]				NLC-4	2.03	. 195	.0045
	29-A	B-58	49,300	1.67	NG_C-1	1.83	. 195	.0055
•	1	, , , , , ,	,	1	MLC-5	2.01	. 195	.0035
1	ļ	1	i	1	NEC-6	1.04		
i	l	ţ	i	1	MLC-2	1.73	. 1955	.004
1	i		1	l	MLC-3	2.03	. 195	.0055
	1			•	MLC-4	1.84	. 1955	.013
	29-B	B-58	38,140	1.67	MLC-1	3.56 7/2.93	.156	.0002*/.001
l	1		1 ,		MLC-5	3.07	. 156	.0045
<u> </u>	l	l	ł	1	MEC-6	1.52		
	1	l	1	1	MLC-2	2.58	.1555	.0035
1	l	•	•	ĺ	MLC-3	2.66	.156	.009
	İ			}	MLC-4	3.33*/3.22	. 156	.0002*/.001
	32-A	B-58	49.820	1.64	MLC-1	1.85*/1.80	. 1825	.0002*/.005
1	] "	1	1		NLC-5	1.91	. 1825	.005
1	1	1	1	1	MLC-6	1.10		
	[	· 1	(	(	ME.C-2	1.91	. 1825	.004
l		1	j	]	MLC-3	1.91	.182	.004
	1		<u> </u>		MLC-4	1.93	. 1825	.004
	32-B	B~58	38,000	1.67	MEC-1	2.35	.149	.015
		1	1,	1	MLC-5	2.40/2.50	.149	.0002/.004
	i	l	l	Ī	M.C-4	1.31		
	l	ĺ	ļ	1	M.C-2	2.06	.149	.004
	1	[	[	ĺ	M.C-3	2.39	.149	.005
	Į	l	i	l	NLC-4	2.56	.149	.0035

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Table Fel (Continued)

Date	Mission No.	Aircraft	Altitude ft	Mach No.	Microphone No.**	Δp lb/ft <sup>2</sup>	Δt sec.	Rise Time sec.
6-20-66	48-A	B-58	41,300	1.55	MLC-1	2.71	. 179	.005
			·		MLC-5	2.61	. 479	.004
	1	i			MLC-6	1.40		
	ł	<b>}</b>		İ	MLC-2	2.52	.1785	.005
					MLC-3	2.66	. 179	.005
	1	<b>]</b>			NLC-4	2.93	.1775	.005
	79-A	B-58	32,100	1.45	MLC~1	2.57	. 1535	.002
	1	i - 1	Í	l	MLC-5	2.52	. 1535	.004
	Į.	!		1	MLC-6	1.37	~	{
	l	i I			MLC-2	2.27	. 1535	.006
	İ	i			MLC-3	2.54	.1535	.005
	}	Ì			MLC-4	2.50	. 1535	.005
	53-A	B-58	42,700	1.59	MLC-1	1.49	.1755	.020
	1		<b>l</b> '		MLC-5	1,49	.1755	.020
	1	1		1	NLC-6	, 588		
	Í	İ		ĺ	MLC-2	1.39	.1755	.021
	i	[ '	í		MLC-3	1.54	.175	.023
	j		[		NLC-4	1.43	.1755	.021
	84-A	B-58	31,220	1.43	MLC-1	2.68	. 145	.0015
	1	}	}	}	NLC-5	2,58	.1445	.017
	l	ļ	1	ł	MLC-6	1.37		
	1		l	l	MLC-2	2.36	. 1445	.004
	i		ſ	ĺ	MLC-3	2.66	.144	.0155
		)		)	MLC-4	2.59	.1445	.019
	54-A	B-58	43,000	1.59	MLC-1	1.28	. 164	.0.65
	l .	1	, '	İ	MLC-5	1.31	. 1635	.0075
!	ì	į.	Ì	(	MLC-6	.718		
	{	1	l	İ	MLC-2	1.36	. 164	.005
	ł	l	ł .	ĺ	MLC-3	1.42	.1645	.0055
		1	1	İ	MLC-4	1.49	. 1645	.,0065
ĺ	59-B	B-58	43,360	1.41	MLC-1	2.31	,2175	.007
	Ì	İ	]	l	MLC-5	2.31	.2176	.010
	I	1	Í	ĺ	MLC-6	1.01		
		[		1	ME.C-2	2.21	.218	.005
	ì	1	1	l	MLC-3	2.24	.218	.0075
	1			•	MLC-4	2.47	.2175	.0045
	98-B	B-58	31,340	1.56	NLC-1	3,27	. 1545	.0025
	ł	l	ĺ	ĺ	MLC-5	3.04	. 1535	.005
	]	j	1	1	MLC-6	1.50		
		i	<b>!</b>	•	NEC-3	2.74	.1545	.004
	1	1	1	l	MLC-3	3.25	. 1545	.006
	1	İ			MLC-4	2.96	. 1545	.064
	60-B		No Bo	on.				
	90-B	B-58	31,800	1.55	MLC-1	2.74	.145	.016
	1	1	1		MLC-5	2.76	.145	.0135
	}	1	1	ł	ME.C-6	1.31	.145	.0135
	1			l	M.C-2	2.66	.1455	.004
	i	1	l	l	ME.C-3	3.16	.145	.002
1	,	1	1	1	MC-4	2.62	.1455	.011

Table F-1 (Continued)

· Contest decision — established and considerable with

	Mission		63.646.45		24	A	T	T
Date	No.	Aircraft	Altitude ft	Mach No.	Microphone No.**	Δp lb/ft <sup>2</sup>	Δt sec.	Rise Time sec.
6-20-66	85-A	B-58	32,320	1.45	MLC-1	3.22	.143	.016
<u>,</u> 1		}	}	j	MLC-5	2.37	.142	.0115
1		}	ì	l	MLC-6	1.27		
	ĺ		•	ļ	MLD-2	2.33	. 1435	.0145
		}	1	]	MLC-3	2.68	.142	.011
	'				MLC-4	2,38	.1435	.016
	93-B	D-58	32,140	1.55	NE.C-1	2,48	.1415	.005
[	l .	•	l	1	NLC-5	2,86	.1410	.008
	1	•	]		MLC-6	1.47		
<u> </u>	1	]	}	}	MCC-3	2.84	.1415	.013
1		ì	1	•	MLC-3	2.92	.141	.006
		•			MLC-4	3.32	,1405	.0045
6-21-66	89-B	B-58	31,760	1.46	MLC-1	2,84	. 151	.018
1	ļ	ļ	]	ļ	NLC-5	2.65	. 1515	.907
]	j	}	ļ	i	NE.C-6	1.46		
]	ļ	}	}	ļ	MC-2	3.00	. 152	.014
]	1	j	ļ		MLC-3	2.67	. 151	.013
	}	1	ł		M.C-4	2.98	. 1515	.012
İ	58-B	9-58	43,600	1.67	MLC-1	1.93	. 175	.006
1	}	ļ	<u> </u>	]	HLC-5	2.20	. 1745	.002
1	}	]	İ	j	MLC-6	1.26		
1	ì			Ĭ	MLC-2	1.55	.175	.012
1	1				MC-3	1.79	.1745	.002
	ļ	}		j	MLC-4	1.91	.175	.0075
	99-B	B-50	31,700	1,47	MLC-1	2,66	.1485	.025
1	ł	ł	1	1	NLC-5	3.544/3.16	.149	/.007
•	1	}	]	ł	MCC-6	1.78		
l	ł	l	1	ł	MCC-2	2.71	.1485	.002
		]		•	MLC-3	3.19	.1485	.0015
		İ	1		MLC-4	3,89	.148	.004
	64-B	B-58	39,860	1.59	MC-1	1.13	.167	.∩25
]	j	ļ	1	}	M.C-5	1,16	.1675	.006
1	]	l	]	•	MLC-6	.575		
		l	1	1	MC-3	1.08	.1675	.0125
1	l	ł	l	ł	MLC-3 MLC-4	1.14	.167	.025
		<b>j</b>				1.19	. 1665	.030
j	100-8	B-58	31,760	1.46	M.C-1	3.55	.147	.0025
	1	}	!	1	MLC>-5	2.96	.1465	.004
]	}	1	1	}	MC-6	1.39		
]	]	}	!	1	182-2	2.46	.1465	.005
	j	1	I	)	MC-3	2.48	.146	.010
	ł				MLC-4	3.54	.1465	.005
	69-13	<b>3</b> −58	44,080	1.62	MC-1	1.32	.1675	.005
	1	l			IR.C-5	1.44	.167	.007
	l	l	ł	ŀ	MLC-6	.732		
	1	i	l	}	MLC-2	1,28		.012
ł		1	l	ł	MLC-3	1.55	.167	.006
	<b>l</b> _	1	1	<u> </u>	MEC-4	1.44	.1665	.001

Table F-1 (Continued)

Date	Mission No.	Aircraxt	Altitude ft	Mach No.	Microphone No. **	Δp lb/ft <sup>2</sup>	∆t sec.	Rise Time
	<del></del>		ļ					<del> </del>
6-21-66	69-B	B-58	39,440	1.39	MLC-1	1.59 1.59	.1855	.023
	1	1	<u> </u>		MLC-5		. 186	.000
		1		•	MLC-6	.837	1855	.018
					MCC-2	1.58	.1855	.016
	1				MLC-3	1.60	.1855 .18*3	.013
	ļ	l			MLC-4	1.66	, 10	.013
	48-A	B-58	43,140	1.60	MLC-1	1.45	. 178	.003
					MLC-5	1.57	.1775	.026
	1		į		MLC-6	.785		
Ì	l		ĺ		MLC-2	1.16	.1775	.011
1	Ì		]	)	MLC-3	1.81	. 177	.002
	ļ				MLC-4	1,44	.1775	.022
Ī	40-A	B-58	43,840	1.65	MLC-1	1.55	.171	.012
}	] - "		],	]	MLC-5	1.77	.171	.006
l	1				MLC-6	1.05		
1	1	1	]	}	MLC-2	1.87	.171	.005
1	1		1	l	MLC-3	1.88	.1705	.009
	1		1		MLC-4	1.96	.171	.0055
	60-B	B-58	43,940	1.64	MLC-1	1.55	. 165	.007
ł	<b>2</b> -3	B-30	43,340	1.04	MLC-5	1.46	.165	.013
1	į	1	i	l	MLC-6	.759		
	1	1	i	l	MLC-2	2.24	.1655	.004
}	1	1	1	i	MLC-3	1.43	.1655	.017
1					MLC-4	1.82	.165	.0095
			43,260	1.62	<b></b> .	2,46	. 1825	.008
1	61-B	B-58	43,200	1.62	MLC-1	2.05	.1815	.011
ı	1	l	ł		MLC-6	1.10		
1	}	}	i	1	MLC-2	3.32	.1815	.0025
ł	ł	l	Į.	1	MLC-3	1.93	.1805	.020
1	İ	1	1	1	MLC-4	2.38	.181	.007
			21 200	١	100		1400	010
	101-B	B-58	31,700	1.5	MLC-1 MLC-5	2.68 2.68	.1485	.019
1	ļ	<b>!</b>	ļ	ţ	MLC-8	1.39	.1453	.015
ł	Į.	1	ł	ļ	MC-2	2.49	.148	.019
ł	1	1	1	1	MLC-3	2.72	149	.001
ł		l		l	MC-4	2.76	. 185	.020
	1.		1	1	}	}		
Į.	85-A	B-58	31,700	1.5	MLC-1	2.23	.146	.023
1	1	ł		Ī	MLC-5	3.74	.145	.020
1	1	i	1	Į	MLC-6	1.57		
1	1	!	1	1	MLC-2	2.64	.1455	.00
[	1	1	1	ł	M.C-3	2.55	.146	.005
					MLC-4	3.12	.1455	.007
6-22-66	28-A	B-58	37,000	1.63	M.C-1	2.26	.162	.013
1	1	1	1		ME.C-5	2.73	.163	.0115
	1	1	l .	ļ	MLC-5	1.45		
1	1	]	i	1	MLC-2	2,36	.163	.0245
I	1	1		1	MC-3	3 .	.1625	.008
L	<u> </u>	<u></u>	<u></u>	L	MC-4	2.62	. 163	.017

Table F-1 (Continued)

Date	Mission No.	Aircraft	Altitude ft	Mach No.	Microphone No.##	Δp lb/ft <sup>2</sup>	` Δt sec.	Rise Time
6-22-66	19-A	B-58	37,200	1.64	NLC-1	2,30	. 1555	.0155
	!	į			MLC-5	2.02	. 156	.015
	Į.	ŧ			MLC-6	1.08		
]		l	<b>j</b>	i i	MLC-2	2.20	.156	.026
	İ	1			MLC-3	1.78	. 1565	.0085
					MLC-4	2.04	.156	.0135
l	6-X	B-59	43,560	1,60	MLC-1	2,48	. 167	.006
ļ .	Ì		·		MLC-5	3.36	.167	.0115
1	ĺ	[			MLC-6	2.48	~-	
1	j	}	]		MLC-2	1.79	.1665	.0245
}	}	1			MLC-3	5.06	.167	.0055
	]				MLC-4	4.12	.167	.016
	30-A	B-58	37,400	1.65	MR.C-1	2.21	. 163	.008
}	j	}	}	<b>i</b> .	MLC-5	1.92	. 1635	.032
	1	1	1		MLC-6	1.01		
}	1	}	}	1	MLC-2	1.98	. 163	.0185
	}	}	<b>j</b> ,		MLC-3	2.10	. 163	.0295
					MLC-4	1.93	.1625	.0045
	34-B	B-58	43,400	1.61	MLC-1	1.44	.169	.018
	]	1	j	}	MLC-5	1.36	.170	.024
1	i	Ì	1	1	MLC-6	.800		
[	1	f .	1	l	MLC-2	1.74		.0105
J		1	j	}	MLC-3	1.59	.170	.003
					MLC-4	1,44	.170	.0165
Ì	24-A	B~58	43,300	1.6	MC-1	1.58	No	.021
1	1	1	[	!	MLC-5	1.59	time.	.031
ŀ	ł	l .	l	1	MC-6	1.34	Could	
	Į.	[	f		MLC-2	1.28	not	.022
	J	1			MLC-3	1.47	read.	.016
]	1				MLC-4	1.55		.0225
	35-A	B-58	43,400	1.6	MLC-1	1.15	. 165	.0225
[	1	1	1		MLC-5	1.19	. 165	.0175
l		ļ			MLC-6	1.01		
1		I	ſ	· '	MLC-2	.989	. 165	.0365
}	1	ļ	J.		MLC-3	1.57	. 1645	.0155
					MLC-4	1.35	.165	.028
	25-B	B-58	43,220	1.59	MLC-1	1.69	. 179	.0135
	1	1			NE.C-5	1.67	. 1795	.0165
]	j	1	i		MLC-6	.852		
1		1		İ	MLC-2	1.23	. 180	.009
		1	[		MLC-3	1.66	. 1785	.0175
	1	1			MLC-4	1.44	. 1795	.010
	23-B	B-58	37,440	1.63	MLC-1	2.73	. 157	.0055
	1				MLC-5	2,45	. 158	.009
	Î	1	[		MLC-6	1.21		
	]	1	l		MLC-2	2.05	. 157	.0075
	1	l			MLC-3	2.36	. 158	.0145
	1	j	j		MLC-4	2.60	. 157	.0125

Table F-1 (Continued)

Date	Mission No.	Aircraft	Altitude ft	Mach No	Microphone No. ##	Δp 15/ft <sup>2</sup>	åt sec.	Rise Time
6-23-66	17-A	B~58	37,600	1.64	MLC-1	2,38	, 1625	.0035
		Į			MLC-5	2.24/2.37	. 162	.005/.0065
1	İ			<b>i</b> :	MLC-6	1.17		
1	1		Ì		MLC-2	2.17/2.22	. 162	.010/.014
					MLC-3	2.35	. 162	.0045
					MLC-4	2.92	.162	.001
l	22-B	B-58	43,360	1.67	MLC-1	1.13/1.43	.1685	.0025/.016
					MLC-5	1.46	.168	.0065
					MLC-6	.859		
	i i				MLC-2	1.53/1.87	.168	.0025/.0055
					MLC-3	. 877/1.60	.168	.002/.010
					MLC-4	1.76	.168	.0055
	31-A	B-38	37,480	1.64	MLC-1	1.11/1.92	.155	.0025/.016
					MLC-5	1.80/1.95	.155	.007/.011
					MLC-6	.990		
	1				MLC-2	2,12	.155	.006
					MIC-3	2.03	.154	.008
					MLC-4	1.79/1.90	. 155	.0015/.015
	33-A	B-50	43,200	1.64	MLC-1	1.2C	.163	.005
					MLC-5	1.20/1.28	.164	.004/.007
	l i				MLC-6	.755		
					MLC-2	1.03/1.26	. 162	.0055/.013
					MLC-3	.701/1.25	.163	.002/.013
					MLC-4	1.30	.164	.006
	20-B	B-58	37,400	1.85	MLC-1	1.67/1.93	. 159	.006/.019
					MLC-5	1.88	. 159	.005
					MLC-6	1.07		
	1			ı	MLC-3	1.97/2.27	.159	.003/.013
	j	- 1	1	i	MLC-3	2.26	.1595	.007
					MLC-4	2.17	.159	.0095
	36-B	B-58	37,400	1.66	MLC-1	4,37	.160	.015
					MLC-5	5.11	.1805	.006
[	1			1	MLC-6	2.69		
i	l		l	- 1	MLC-2	4,24	.160	.0025
Į.		- 1		İ	MLC-3	7.65	. 1595	.005
1		İ			MLC-4	6.12	.160	.005
	6X-2	B~58	43,520	1.67	MLC-1	1.61	.168	.019
i		i	1	1	MLC-5	1.52	.168	.019
	i	İ		ŀ	TTC-6	~~		
1	l				MLC-2	2.27	.168	.006
į	j		1	- 1	MLC-3	1.51	.1675	.0135
	1	į			MLC-4	2,04	.168	.0125
6-4-66	2	F-104	No Track	cing	MLC-1	1.19	.087	
į	ł	ı	1	_	MLC-5	1.16	.087	
	1	Ì	ŀ	i	MLC-6	.622		
	ł	İ	1	- 1	MLC-2	1.30	.087	
		i	l	ł	MLC-3	1.26	.087	
<del>-</del>				1	MLC-4	1.04	.087	

Table F-I (Continued)

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would agree to the day

	24 4				I			
Date	Mission No.	Aircraft	Altitude ft	Mach No.	Microphone No. **	Δp 1b/ft <sup>2</sup>	Δċ sec.	Rise Time
ļ			ļ	10.		10/10	<b>80</b> C.	340.
6-13-66	26-A	F-104	21,200	1.4	MLC-1	1.75	.0735	.0055
					MLC-5	1.74	.073	.0055
<b>!</b>		1		1	NLC-6	.883		
	f .	}		1	MCC-3	1.88	.0735	.0035
ļ :	i	l		1	MLC-3	1.88	.0735	.0035
				Ì	MLC-4	1.93	.074	.0035
	26-B	F-104	29,660	1.6		<b>N</b> i	sed Boo	! #
6-14-66	26-A	F-104	No Track	l :in∉	MLC-1	2.10	.072	
				l	MLC-5	2.28	.072	
]	ļ		Ì	ŀ	MLC-6	1.03		
		l		}	MLC-2	1,72	.0715	l
]			ł	1	MLC-3	2.15	.072	
			ł		MJ.C-4	2.15	.0735	1
	26-B	¥-104	29,920	1.54	MLC-1	1.61	.080	. 2065
		**.53	,		MLC-5	1.43	.0795	.0055
1	l	í	į		NILC-6	.814		
	[	l		1	MLC-2	1.48	.079	.013
	Ì	1			MLC-3	1.45	.0795	.007
					MLC-4	1.43	.079	.00€
	38-A	P-104	No Track	ing	MLC-1	2.07	.074	.004
	l	l		1	MLC-5	2.10	.074	.0055
			l	•	MLC-6	1.08		
	l	ļ	,		MLC-2	1.94	.0735	.006
i			j		MLC-3	1.94	.074	.004
					MLC-4	2.35	.074	.0045
	38-B	F-104	29,700	1.52	MLC-1	1.49	.0795	.019
]		į į	!	1	MLC-5	1.36	.0785	.0135
					MLC-6	.788		
<u> </u>	ł	ŀ			MLC-2	1.63	.079	.0085
					MLC-3	1.33	.0795	.0095
					MLC-4	1.62	.0795	.0115
	37-A	F-104	29,700	1.49	MLC-1	1.30	.079	.009
					MLC-5	1.19	.0795	.004
1					MLC-6	. 788		
1					MLC-2	1.41	.079	.004
1					MLC-3	1.28	.079	.008
					MLC-4	1.56	.0795	.007
	37-B	F-104	21,080	1.39	MLC-1	3.214/2.93	.0755	.0005*/.002
			i i		MLC-5	2,60	.075	.004
					MLC-6	1.34		
					MLC-2	2.67	.075	.0015
					MLC-3			
					MLC-4	2,99	.075	.004
6-15-66	1X-A	F-104	14,080	1.21	MLC-1	4.24	.080	.0005
					MLC-5	3.75	.0795	.0045
			•		MLC-6	1.99		
					MLC-2	3.17	.080	.0035
					MLC-3	4.40	.080	.0005
					MLC-4	3.48	.0795	.004

Table F-1 (Continued)

	Mission	Γ	4344444		Tw/	1 4-	4.4	D4 == 04=0
Date	No.	Aircraft	Altitude ft.	Mach No.	Microphone No.**	Δp lb/ft <sup>2</sup>	Δt sec.	Rise Time
6-15-66	1X-B	F-104	28,140	1.5	MLC-1	1.32	.079	.009
	l	ļ	,	ĺ	MLC-5	1.50	.079	.005
	Į.	İ		ļ.	MLC-6	.831		
	1			1	MLC-2	1.62	.0785	.0005
	l	i			MLC-3	1.36	.079	.0055
	-				MLC-4	1.52	.0785	.0055
	2X-A	F-104	29,700	1.32	MLC-1	1.62	.090	.014
	1	]		l	MLC-5	1.63	.090	.0115
ļ	i	ł		[	MCLC-6	l		
	1		i		MLC-2	1.55	,0905	.C07
	l	Į.		i	MILC-3	1.69	.090	.009
		i			MLC-4	1.76	.0905	.0125
	2X-B	F-104	14,080	1.20	MLC-1	4.27	.079	.0035
		1		1	MLC-5	4.44	.079	.004
·		1		i	MLC-6	2.13		
					MLC-2	4.30	.079	.004
	}	ì		Ì	MLC-3	4.40	.0795	.034
	ļ	į			MLC-4	4.30	.079	.0035
	3X-A	F-104	29,100	1.58	MLC-1	1.15	.075	.0135
	J	1	35,100	}	MLC-5	1.19	.0755	.0105
	ł	ļ		l	MCC-6	.631		
	Ì	1	1	1	MLC-2	1.39	.0745	.0105
		j		l	MLC-3	1.20	.0755	.008
		1			NLC-4	1.23	.075	.0095
	3х-в	F-104	14,200	1.15	MLC-1	2.35	077	006
İ	34-7	1 104	14,200	*	MLC-5	2.33	.077 .077	.006 .006
	i	}	}	ł	MLC-6	1.20		.000
	l	i			MLC-2	2.10	.077	.0115
}	1		<b>!</b>	•	MLC-3	2.29	.077	.010
					MLC-4	2,17	.0775	.006
	١	1 n 104						4045
	4X-A	F~104	14,060	1.28	MLC-1	3.38	.0675	.0215
1	ļ		I	}	MLC-5 MLC-6	3.28 1.69	.0685	.0055
<b>[</b>	İ	[		}	MLC-2	3.20	.0675	.0035
1	1		ļ		MLC-3	3.19	.0675	.0035
			1	}	MLC-4	2.49	.0675	.0035
				١				l i
	4X-B	F-104	29,880	1.62	MLC-1	3.29/2.56	.078	.0005/.004
1	1	İ	<b>l</b>		MLC-5	2.41	.0765	.0045
]	1	1	ļ	l	MLC-6	1.20		
1	1	j	<b>[</b>	<b>,</b>	MLC-2	2.26	.077	.0045
Ī		I		l	MLC-3	2.44	.077	.005
l	1		! !		MI,C-4	2.46	.0775	.0035
6-16-66	27-A	F-104	29,300	1.65	MLC-1	1.28	.075	.0055
]	Į	I	1		MG.C-5	1.48	.075	.004
l	1	I	1		MLC-6	.797		
1	1	I	l		MLC-2	1.54	.075	.001
	Į.	I	l		MLC-3	1.45	.075	.0055
l	Ì	l	[	1	MLC-4	1.52	.075	.004

Table F-! (Continued)

	Mission		Altitude	Mach	Microphone	Δр	Δt	Time Rise
Date	No.	Aircruft	ft.	No.	No.**	1b/ft <sup>2</sup>	sec.	sec.
	<b></b>							ļ
6-16-65	27-B	F-104	20,540	1.4	MLC-1	1.63	.074	.003
				ļ l	MLC-5	1,61	.0735	.004
	}	ł	i		MLC-6	.897	•	
	i '				MLC-2	1.95	.0735	.0035
	1	•	}	ļ	MLC-3	1.56	.0735	.005
	1		ļ		MLC-4	1.58	.0735	.0035
	5-X	F-104	29,700	1.65	MLC-1	1.93	.072	.005
		1	i i		MLC-5	1.79	.072	.0045
	ł	<b>1</b>		[	MLC-6	.964		
	1	1		1	MLC-2	1.64	.071	.003
	1	1	ł		MLC-3	1.71	.0715	.0045
	İ				MLC-4	1.71	.072	.0045
6-22-66	28-8	P-104	20,820	1.35	MLC-1	2.05	.0775	.0135
0-32-00	40-5	1-104	20,020	1.33	MLC-5	2.20	.078	.0085
	ł	ì	ł	}	MLC-6	1.34		.0003
	ì	1	<u> </u>	1	MLC-2	2.15		.0105
	1	1	l	ł		•	.077	.0065
	į	l	}	į	MLC-3	3.46	.078	
	ļ		Ì		MLC-4	2.98	.0775	.0085
	19-B	F-104	29,500	1.42	MLC-1	1.51	.0885	.0175
	Į.	İ		l	MLC-5	2,05	.089	.0025
	1	1	1	1	MLC-6	1.03		ļ
	l	l		1	MLC-2	1.50	.0885	.008
	l	i		1	MLC-3	1.94	.0885	.0095
		1		1	MLC-4	1.99	.089	.0085
	30-B	P-104	29.720	1.37	MLC-1	1.01	.093	.0215
	]	}	}	ĺ	MLC-5	j ,985	.094	.0265
	i	ļ		i	MLC-6	.439		
	(	{		1	MLC-2	.724	.092	.0385
	1	Į	i	1	MLC-3	.958	.0935	.0265
					MLC-4	1,02	.093	.0290
	34-A	F-104	29,600	1.39	MLC-1	1,31	.09E	.018
	1		,	1	MLC-5	1.29	.0965	.0225
	ł	ł	1	ł	MLC-6	.981		
	l	l .	ł	ļ .	MLC-2	1.45	.0945	.0215
	1	Ì		[	NLC-3	1.07	.0985	.011
		ł	ļ	1	MLC-4	1.30	.0945	.621
	24-B	F-104	20,860	1.36	MLC-1	1,76	.0785	.012
	43-5	1-10-3	20,000	1.30	MLC-5	2.37*/1.69		<b>f</b> <u>⊾</u>
	i	1	l	Į.	•		.0775	.0005*/.0135
	l	}	l	ŀ	MLC-6	1.06	.077	002
	į	Į.	1	l	MLC-3	1.76 1.99		.007
					MLC-4	2.90	.078	.007
	]				]	j		
	35-B	F-104	21,060	1,28	MLC-1	3.02	.0815	.005
	ł	l	I	1	MLC-5	2.85	.082	.0035
	1	1	1	l	MLC-6	1.43		
			i	l	MCC-3	2.24	.0825	.007
	ł	1	l	1	NLC-3	2.50	.0815	,007
i	l	L	l	L	MLC-4	1.82	.0805	,0045

Table F-1 (Continued)

Date	Mission No.	Alieraft	Altitude ft.	Mach No.	Microphone No.##	Δp 1b/ft <sup>2</sup>	Δt sec.	Rise Time
6-22-66	25-A	F-104	21,900	1.39	MLC-1	1.24	.075	.007
	ł				MLC-5	1.36	.075	,0065
	]		Ì		15LC-6	.749		
	i				MIC-2	1.42	.078	.0095
	}	}	]	1	MLC-3	1.75	.075	.0045
	ĺ				MLC-4	1.46	.075	.012
	23-A	F-104	29,720	1.51	MLC-1	.993	.083	.036
	<b>,</b>			į į	MLC-5	.985	.084	.0195
	· ·			1	MLC-6	.904		
	l .				MCTC~ 3	2.17	.084	.0045
	Ī				MT_C-3	1.01	.083	.0225
					MLC- 4	1.24	.0845	.0135
6-23-66	17-B	F-104	21,600	1.4	MLC-1	2.31	.076	.0015
	ł	i			MLC-5	1.33/2.03	.0755	.002/.007
	Į.				MLC 3	.938	•	
	1				MLC-2	1.43/1.48	.076	.002/.005
	!			]	MLC-3	1.93	.076	.0055
	1				MLC-4	1,82	.076	.002
	22-A	F-104	29,260	1.4	MLC=1	1.39/1.80	.083	.001/.0085
					MLC-5	1.22/1.51	.083	.0045
	ļ	į į			MLC-6	.781		
	i				MLC-2	1.55	.0825	.010
	Į.				MLC-3	1.28/1.43	.083	.0015/.006
					MLC-4	1.74*/1.52	.082	.001/.0045
	31-B	F-104	21,260	1.39	MLC-1	2.17	.076	.006
	ł	į			MLC-5	1.02/2.08	.076	.0015/.013
	ĺ		}		MLC-6	.547		
	ĺ	ł			MLC-2	1.72/1.97	.076	.003/.0095
	ŀ			ł	MLC-3	1.93	.076	.013
	}				MLC-4	1.63/2.49	.076	.001/.006
	33-B	F-104	29,840	1.49	MLC-1	1.43	.084	.012
	Į.			<b>!</b>	MLC-5	1.61	.084	.011
	1	•		Ì	MLC-6	.885		
	(	l i			MLC-2	2.41	.084	.004
_	!	Î			MTC-3	1.85	.084	.010
	ļ				MLC-4	1.82/1.92	.084	.0085/.011
	20-A	F-104	21,520	1.37	MLC-1	1.86	.078	.011
	1				MLC-5	1.61/1.97	.080	.007/.012
	]		- 1		MLC-6	1.07		
				[	MTC-3	.985/1.74	.079	.0025/.020
		l l			MCC-3	2.14	.080	.003/,0095
					MLC-4	1.83	.079	.0,12
	36-A	P-104	20,860	1.39	NLC-1	1.93	.077	.002
	1				MC-5	2.24	.077	.095
	ļ				MLC-6	1.25		
	l				MLC-2	1.97/2.12	.077	.001/.0055
	Ī				MLC-3	1.85/2.14	.0765	.0045/.007
	l	1		j	MLC-4	1.70/2.04	.077	.003/.005

Table F-i (Concluded)

Date	Nission No.	Aireraft	Altitude ft.	Mach No.	Microphone No.	Δp lb/ft <sup>2</sup>	Δt sec.	Rise Time sec.
6-23-66	7~X	F-104	29,640	1.55	NLC-1 NLC-5 NLC-6 NLC-2 NLC-3 NLC-1	1.99 1.70 .806 3.33 1.27/1.56	.081 .081  .082 .0815	.008 .016  .0075 .009/.0205

## NOTES:

<sup>\*</sup> Slash (/) denotes two peaks.

 $<sup>^{**}\</sup>mbox{MLC-2}$  moved to southeast corner of yard of concrete blockhouse after flights of June 6, 1966.

TABLE F-2
SUMMARY OF DATA FROM FREE FIELD
MICROPHONES LOCATED NEAR L-2, PHASE II)

					Average
					<u>Peak</u>
<u>Date</u>	<u>Mission</u>	<u>Aircraft</u>	Altitude	<u>Mach</u>	Pressure
	No.		Ft.	No.	psf
6-6-66	70	B-58	43,400	1.60	0.94
	40	B-58	31,400	1.48	0.57
6-7-66	76A	B~58	31,500	1.48	0.94
	46B	B~58	43,720	1.65	0.64
	79A	B-58	31,600	1.52	1.47
	80A	B~58	31,600	1.53	1.21
	818	8-58	31,400	1.49	1.03
6-8-66	43A	B-58	42,380	1.62	0.32
	75A	B-58	31,200	1.44	0.85
	42A	B-58	43,260	1.67	0.28
	73A	B-58	31,200	1.50	0.65
	41A	B-38	43,200	1.60	0.26
	72A	B-58	31,200	1.49	1.13
	<b>57</b> B	B-58	37,600	1.66	0.57
	56RB	B-58	43,040	1.64	0.11
	87	B-58	31,440	1.49	0.50
	55RB	B-58	43,200	1.64	0.14
	86RB	B-58	31,360	1.49	0.40

<sup>1)</sup> Refer to Legend, Page F.I, for explanation of notations and abbreviations.

TABLE F-2 (Continued)

SUMMARY OF DATA FROM FREE FIELD

MICROPHONES LOCATED NEAR L-2, PHASE I

					Average
					<u>Peak</u>
Date	Mission	Aircraft	<u>Altitude</u>	Mach	Pressure
	No.		Ft.	No.	psf
6-9-66	55SRB	B-58	35,720	1.69	0.68
	87SRB	9-58	31,000	1.53	1.06
	80\$RB	B-58	31,000	1.53	1.21
	57SRB	B-58	43,100	1.70	0.58
	41SA	B-58	42,920	1.52	1.13
	73SA	B-58	31,720	1.50	1.12
	42SA	B-58	13,060	1.52	1.21
	75SA	B-58	31,680	1.55	0.62
	43SA	B-58	43,000	1.68	1.66
	42SA	B-58	43,300	1.70	0.39
	46SA	B-58	42,900	1.68	0.53
	72\$A	B-58	31,320	1.53	1.15
6-13-66	18A	B <b>-</b> 58	37,740	1.64	1.50
0-15-00	18B	B-58	49,600	1.66	1.15
	21A	B-58	37,840	1.69	1.50
	21A 21B	B-58	49,160	1.72	1.31
	21B 25A		•	1.72	0.97
		B-58	21,200		
	29A	B-58	49,300	1.67	1.01
	29B	B-58	38,140	1.67	1.67
	32A	B~58	49,820	1.64	1.15
	32B	B <b>~</b> 58	38,000	1.67	1.50

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TABLE F-2 (Continued

SUMMARY OF DATA FROM FREE FIELD

MICROPHONES LOCATED NEAR L-2, PHASE I

					Average Peak
<u>Date</u>	Mission	Aircraft	Altitude	Mach	Pressure
	No.		Ft.	No.	psf
6-14-66	26A	F-104	20,000	1.4	1.21
	26B	F-104	29,920	1.54	0.53
	38A	F-104	20,000	1.4	0.42
	38B	F-104	29,700	1.52	0.68
	37A	F-104	29,700	1.49	0.83
	37B	F-104	21,080	1.39	0.55
6-15-66	4XB	F-104	29,880	1.62	0.45
6-20-66	48A	B-58	41,300		0.93
	53A	B-58	42,700	. 3	0.86
	84A	B-58	31,220	1.43	0.53
	54A	B-58	43,000	1.59	0.53
	59B	B-58	43,360	1.41	0.53
	988	B-58	31,340	1.50	د1.5ء
	93B	8-58	32,140	1.55	1.56
6-21-66	89B	B <b>-</b> 58	31,760	1.46	1.34
	58B	B-58	43,600	1.67	0.69
	99B	B-58	31,700	1.47	1.34
	66B	B-58	39,860	1.59	1.04
	100B	B-58	31,760	1.46	1.14
	68B	B-58	44,080	1.62	0.71

## TABLE F-2 (Continued) SUMMARY OF DATA FROM FREE FIELD MICROPHONES LOCATED NEAR L-2, PHASE I

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The state of the s

				Average
				<u>Peak</u>
Mission	Aircraft	<u>Altitude</u>	Mach	Pressure
No.		Ft.	No.	psf
		-		1.88
		-	1.60	0.96
40A	B-58	43,840	1.65	0.87
60B	B-58	43,940	1.64	0.48
101B	B-58	31,700	1.50	1.54
28A	B-58	37,000	1.63	1.30
28B	F-104	20,800	1.35	0.34
19A	B-58	37,200	1.64	1.31
19B	F-104	29,500	1.42	0.31
6X	B-58	43,560	1.6	0.46
30A	8-58	37,400	1.65	2.00
34B	B-58	43,400	1.61	0.79
35B	F-104	21,060	1.28	0.14
23∧	F-104	29,720	1.51	0.43
238	B-58	37,440	1.63	1.19
17A	8-58	37,600	1.64	2.13
17B	F-104	21,600	1.40	0.70
22A	F04	29,200	1.40	0.47
22B	B-58	43,300	1.67	0.47
31 A	B-58	37,400	1.64	0.97
31B	F-104	21,200	1.39	0.59
33A	B-58	43,200	1.64	0.74
	No.  85A 48A 40A 60B 101B  28A 28B 19A 19B 6X 30A 34B 35B 23A 23B  17A 17B 22A 22B 31A 31B	No.         85A       B-58         48A       B-58         40A       B-58         60B       B-58         101B       B-58         28A       B-58         28B       F-104         19A       B-58         19B       F-104         6X       B-58         30A       B-58         34B       B-58         35B       F-104         23A       F-104         23B       B-58         17A       B-58         17B       F-104         22A       F04         22B       B-58         31A       B-58         31B       F-104	No.         Ft.           85A         B-58         31,700           48A         B-58         43,140           40A         B-58         43,840           60B         B-58         43,940           101B         B-58         31,700           28A         B-58         37,000           28B         F-104         20,800           19A         B-58         37,200           19B         F-104         29,500           6X         B-58         43,560           30A         B-58         37,400           34B         B-58         43,400           35B         F-104         21,060           23A         F-104         29,720           23B         B-58         37,440           17A         B-58         37,600           17B         F-104         21,600           22A         F04         29,200           22B         B-58         37,400           31A         B-58         37,400           31B         F-104         21,200	No.         Ft.         No.           85A         B-58         31,700         1.50           48A         B-58         43,140         1.60           40A         B-58         43,840         1.65           60B         B-58         43,940         1.64           101B         B-58         31,700         1.50           28A         B-58         37,000         1.63           28B         F-104         20,800         1.35           19A         B-58         37,200         1.64           19B         F-104         29,500         1.42           6X         B-58         43,560         1.6           30A         B-58         37,400         1.65           34B         B-58         43,400         1.61           35B         F-104         21,060         1.28           23A         F-104         21,060         1.51           23B         B-58         37,400         1.63           17A         B-58         37,600         1.64           17B         F-104         21,600         1.40           22A         F04         29,200         1.40

## TABLE F-2 (Continued) SUMMARY OF DATA FROM FREE FIELD MICROPHONES LOCATED NEAR L-2, PHASE I

					Average
					<u>Peak</u>
<u>Date</u>	Mission	<u> Aircraft</u>	Altitude	Mach	Pressure
	<u>No.</u>		<u>Ft.</u>	No.	<u>ps f</u>
6-23-66	33B	F-104	29,800	1.49	0.70
	20B	B-58	37,400	1.65	1.34
	36A	F-104	20,800	1.39	0.59
	7X	F-104	29.600	1.55	0.30
	6X2	B-58	43,500	1.67	0.32

SUMMARY OF DATA FROM FREE FIELD MICROPHONES LOCATED NEAR E-2, PHASE . 1.

•	<b>Q</b> Q	9	9	۰ و	۰ ب	9	9	9	9	9	9	9	۰ م	٥	•	Ø	9	S	9	9	9	9	9	9	۰ ب	9	9	٥	9	9	9	•	9	9	Φ	9	9	9
	SPD	FT/SEC	1345						1242						1351						1250					•	1626						1439					
	WAVE		58.9					1	66.5						58.7					,	19.4						20.9					,	54.3					
	PER-		•229	•229	•228	• 230	.231		9	•165	9	9	S		•085	.081	•081	•081	•081		3	.237	3	S.	m		153	S	S	S	Ś		•079	•078	•019	• 078	•079	
	TIME T2	0	.0015	• 1065	• 002	• 005	•003		•002	•002	•003	• 007	• 002		•0015	<b>•</b> 004	• 0000	•003	•0032		•002	008	•004	•008	900•		014	.017	.0075	•008	•012		•001	• 0002	<b>•</b> 00 <b>•</b>	• 005	• 005	
!	RISE T1	SEC								•0002						•0002								•						.001								
;	'SF) TIVE	P2	3	2.67	ú	9	8		•	5.44	2	•	4		•	8	•	2,86	₩.		•	4	<b>ب</b>	•	•		2.30	7	2.14	6	4.				Ŷ	2.80	٥,	
i	PEAK AMPLITUDES (PSF) SITIVE	P1 P2	2	2.34	•2	7	3			2.51	•	6	~5		0	•	4	3.58	8		٥.	1.87	7		e,		1.62	•	ထူ		. <del>†</del>		•2		-	3.95	6•	
	AMPLIT E	P3	3.06	2.91	2.78	2.86	2.94	1.44	2.99	3.37	2.86	2.93	3.24	1.59	2.75	3.07	3.85	3.21	3.34	1.78	2.37	2.23	2.82	2.25	3.09	1.52	2.15	2.21	2.32	2.40	2.52	0.99	3.96	3.71	3.02	3.47	3.41	1.69
	POSITIV	P2								3.57						3.81														2.10								
		p1																																				
	TYPE	_	8	7	7	7	7		7	9	7	2				_	-		7			ĸ			-		•		w.	-	•		4	4	~	4	7	
	HOUSE 1		MLC1	MLC2						MLC2		Ξ			MC1					MLC6												MLC6	Į	Ξ	ĭ	Σ	ĭ	MLC
		j	7										2																									
	CHNL		601	603	605	607	609	611	601	603	605	607	609	611	601	603	605	607	609	611	601	603	605	607	609	611	601	603	605	607	609	611	601	603	609	607	609	611
	NSM		1-1	1-1	1-1	1-1	1-1	ן-ן	1-3	1-3	1-3	1-3	1-3	1-3	1-4	1-4	1-4	1-4	1-4	1-4	2-1	2-1	2-1	2-1	2-1	2-1	2-3	2-3	2-3	2-3	2-3	2-3	2-4	2-4	5-4	2-4	2-4	5-4

1) Refer to legend, page F.2 for explanation of notations and abbreviations.

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GND	าสร		1476					•	1384					•	1413						14/6						1399			†			10-1					
WAVE	AMBLE		53.6					•	26.7					,	26.1					,	20.9						22.0			:			742					
PER-	'		S	5	.153	S	S	1	'n.	<b>60</b>	3	•234	S.	(	•077	•077	•077	•077	•077		4	• 149	4	4	*		9	S.	.232	2	S)	•	•	-1	7	.173	-	
TIME	12		S	•0055	• 002	•0035	5		•002	• 002	S	• 0045	<b>*</b> 00 <b>*</b>		*00*	• 002	•002	<b>•</b> 00 <b>•</b>	•00•		~	•015	-	7	-		• 002	•002	• 002	900•	•002	•	Ĥ,	.015	15	0	16	
RISE	<b>=</b>													,	•001															,			,	• 0002	• 001			
	GATIVE	P2	7	ထ	1.67	٥.	6.		5	6	~	2.51	•		4	2.36	£.	€,			Ç	2.01	8	7	0	(	7.	• 5	2.02	2	•2		ø	•67				
UDES	NEGA	ΡΊ	0	6	1.96	2			2	-	7	2.13	•2		8	2.76	7	9			8	1.83	8	6	8	1	7	ů.	2.23	7	7			04.				
<b>AMPLITUDES</b>	ш	P3	9	4	3	9	2.56	3	3	4.	6	4	Š	e.	4.	e.	•	3	4.	•2	Ü	٠,	5	•2		6	ň	Ş	4	6	*	~	• 10	• 10	•67	.67	•67	•32
PEAK	ITI	P2												•																				• 60	• 60			
		Pl													2.79																							
TYPE			~	~	2	~	~		~	~	7	7	7		<b>A6</b>	7	~	7	~		m	m	m	m	m		~	~	7	~	7		m	∞	ಹ	~	m	
USE	NSTR	,	Ę.	MI C2	<b>M</b> L03	M C4	MLC5	MLC6	MLC1	MLC2	MLC3	ALC 4	MLC5	MLC6	MLC]	MLC2	MLC3	MLC4	MLC5	ALC6	MC01	FLC2	M.C3	MLC4	MLC5	MLC6	EC1	MLC2	MLC3	MLC4	MLC5	MLC6	M C	MLC2	<b>EC3</b>	MLC4	MLCS	MLC6
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GND		2235						153						212						163						227						180					
WAVE		5200						67.1						53.3						56.7						41.3						45.0					
PER-			•284	8	8	8		5	S	.157	S	8		•	80	.284	3	8			.147	4		•147			•264	26		• 264		16	9	9	•165	9	
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JDES NEGA	4	•62	•60					•75	.83	•61	• 70	•76		.95	õ		1.03	.97			•91			.91		1	.51	• 56		•65		2.14	2		2.00	0	
AMPLITUDES E NE	P3	•2	1.16	~	?	5	•	1.29	6	1.30	2		S	•		1.91	æ	9	<b>.</b> 874		1.79			1.67	.887		1.33	6			•	4.	ň	5	2.12	6	• 2
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RISE T1		•001	.0015																						•0002							•0009			• 0000	
ATIVE P2	1.82		1.67	1.93	1,93		1.59		5	1.73	•		6	0	8		6.		9	1.68	•	-	1.78		1.82	1.86	1.67	2.01	1.87		0	7	7	2,19	.2	
ν iii ⊶	1.59		4.	1.48	•		1.21		1.25	•	~		7	4.	2	2.37	.2		•2	1.25	4	ė	•2		•	Š	1,53	-	'n		4	6	4	2.16	9	
AMPLITUDE E P3 P	•		•	•	•	•	•	•	•	•	1.97	•	•	•	•	•	•	•	•	•	•	•		•	•	٠	•	•	•	1.26	•	•	•		•	•963
PEAK POSITIV P2		6	2.04																						3.36							2.47			2.17	
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PER- 100		296	• 296	962.	• 582	• 295	•	•	9	•	.163	Ø	ĺ		-	-	•074	_		• 169	•170	•170	•170	•170	(	5	9	• 298	Ò	9	- (	<u> </u>	<u> </u>	~	•076	~	
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GND	1667	1527		1535	1702	1639	1455
WAVE	45.0	48•7		43.6	7 • 7 7	49.3	52.6
PER- IOD.	.076 .076 .076	.075	• 188 • 189 • 189	.172 .173 .173 .173	.072 .073 .073 .073	.074 .075 .075 .075	.155 .154 .154 .154
TIME T2	0065	000	.015 .008 .0075	.015 .0065 .014 .0085	.001 .005 .004 .004	.0045 .005 .0045 .005	.007 .006 .005 .005
RISE T1	•0025	•0015	•001		.005 .0015		• 001
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g	ao				1.12 1.46 1.37	1.30	1.9
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IPL 1 TUG P3	0.00	1.00 1.09 .60 1.59		• • • • •		1	2.12 2.24 1 1.55 1 1.02 1
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PEAK POSITIV P2	1.75	0.84 0.62 0.76	1.01	
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PER-		.186	•185	.186	.186	.185	,	•196	.196	•196	196	•196		.174	•174	.174	•175	•175		•175	.175	•175	.174	.174		•195	.195	.195	.195	.195		• 186	• 186	.186	.187	.187	
TIME 12	<u> </u>	.012	.0135	.014	.0135	•001		• 00 2	•0025	•004	.0015	•0045		•003	•002	•003	•008	900•		•004	•0075	•015	÷00•	• 005		• 0025	.015	•005	<b>6008</b>	•005		.0035	•005	• 00 5	.0015	.012	
RICE	<u>:</u>		•0005		•0000			•0002		.0015								•0025			•005	•005	•0005				•0045						• 0002				
T 1 V F	P2	•	•	•	1.44	1.36		•	•	1.53	•	1.70		e	•	6	1.76	4.		7	4.	2	1.38	7		•	•	1.25	•			•	1.51	•	•	•	
UDES NEGAT	P1	0	6	1.14	9				4.	1.18	9			•	•	•	1.28				0.81	-		0.76		6	1.41	1.35	1.59	•		8	2.34	2	S	ŝ	
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GATIVE P2	1.86 1.86 1.68 1.95	1.84 1.98 1.61 1.98	2.05 2.12 2.00 2.38 2.15	1.92 1.90 1.96 1.97	1.93 1.94 2.02 2.01 1.90	1.77 1.88 1.84 1.94
UDES NEGA P1	1.39 1.58 1.52 1.52	2.02 2.02 1.42 1.77	1.82 1.49 1.73 1.91	1.49 1.51 1.57	2.21 1.25 1.48 1.40 1.66	10.55 10.55 10.32 10.45 10.45
PEAK AMPLITUDES SITIVE NEG P2 P3 P1	2222	12.00 22.00 20.00 12.00 10.00 10.00				
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## APPENDIX G

TYPICAL ANALOG TAPE LOG

(For Tape Recorder 4, 13 January 1967)

See Appendix B for explanation

of notations and abbreviations.

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APPENDIX H

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### APPENDIX H

#### DERIVATION 3

# A. RELATION OF INSIDE PRESSURE TO OUTSIDE PRESSURE a)

The response of the large glass window in Test House E-I was analysed to determine the relation of the inside pressure to the outside pressure. This Appendix presents the results of this analysis.

med that the change in the inside pressure was proportional to the 'volume inside the garage produced by deflection of the window. also assumed that the deflected surface of a simply supported plate mounted in a rigid airtight enclosure when subjected to a sonic boom loading was closely approximated by its first mode static deflected shape multiplied by an appropriate DAF, i.e.:

$$w = a_{11} \sin \frac{\pi x}{L} \sin \frac{\pi y}{b} g(t)$$
 (H-1a)

$$w = a_{II} \sin \frac{\pi x}{L} \sin \frac{\pi y}{b} (DAF)_{n}$$
 (H-1b)

$$w = w_s (DAF)_n$$
 (H-ic)

where

$$a_{||} = \frac{16 L^4}{D\pi^6 [1 + (L/b)^2]^2}$$
 (H-2)

Here it was assumed that the DAF due to net pressure is a known quantity. A more rigorous approach to this problem without making this assumption leads to the following differential equation:

$$\ddot{f}_{11} + (A + Ct) \dot{f}_{11} + 2Cf_{11} = F\dot{f}_{0}$$
 (H-3)

where

$$A = \frac{D\pi^4}{\rho L^4} \left[ 1 + (L/b)^2 \right]^2$$

a) A Glossary of terms is given at the end of this Appendix.

$$C = \frac{16 P_a L^2 b^2}{V_{\pi}^4}$$

$$F = \frac{4 L b}{\pi^2}$$

then

$$w = f_{11}(+)\sin\frac{\pi x}{L}\sin\frac{\pi y}{b}$$
 (H-4)

In general  $t_{|||}$  (t) cannot be expressed in closed form and it is necessary to obtain the solution to (H-3) by numerical integration. In order to obtain w, it was assumed in the remainder of this discussion that (DAF)<sub>n</sub> was a known quantity.

The total volume displaced in the garage is

$$V^{I} = DAF \int_{0}^{L} \int_{0}^{b} a_{II} \sin \frac{\pi x}{L} \sin \frac{\pi y}{b} dydx \qquad (H-5a)$$

$$= a_{11} \frac{4 L b}{\pi^2} DAF$$
 (H-5b)

Assuming pressure times volume inside the garage is constant,

$$P_a V = (P_a + P_i) (V - V^{\dagger})$$
 (H-6)

Defining

$$P_{n} = P_{o} - P_{i} \tag{H-7}$$

Equation (H-6) becomes

$$P_{n} = P_{c} - P_{a} \left[ \frac{v^{1}}{v - v^{1}} \right] \approx P_{o} - P_{a} \frac{v^{1}}{v}$$
(H-8)

The maximum displacement occurs at the center of the plate and is

$$w_n = a_{11} DAF$$
 (H-9a)

$$w_{n} = \frac{16 L^{4} P_{n} (DAF)}{D\pi^{6} [1 + (L/b)^{2}]^{2}}$$
 (H-9b)

Substituting Equations (H-5b) and (H-8) into Equation (H-9b) yields

$$w_n = B \left[ P_0 - \frac{P_a + L + b + w_n}{v_m^2} \right]$$
 (H-10a)

where

$$B = \frac{16 L^4 (DAF)}{D\pi^6 [1 + (L/b)^2]^2}$$

Solving for  $\mathbf{w}_{\mathbf{n}}$  gives

$$w_{n} = \frac{P_{o} B}{1 + \frac{4 P_{a} B L b}{\pi^{2} V}}$$
 (H-10b)

Noting that

$$W_0 = P_0 B \tag{H-II}$$

Equation (H-7b) can be written

$$\frac{\frac{w_n}{w_0} = \frac{1}{1 + \frac{4 P_a B L b}{\pi^2 v}}$$
 (H-12)

Since

$$\frac{P_n}{P_0} = \frac{w_n}{w_0} \tag{H-13}$$

The inside pressure is

$$P_{i} = P_{0} = \frac{\frac{4 P_{a} B L b}{\pi^{2} V}}{1 + \frac{4 P_{a} B L b}{\pi^{2} V}}$$
(H-14)

which can be rewritten as

$$P_{i} = P_{o} = \frac{P_{a}B}{P_{a}B + \frac{\pi^{2}V}{4Lb}}$$
 (H-15)

## GLOSSARY OF TERMS

E = modulus of elasticity

L,b = plate dimensions

t = plate thickness

v = Poisson's ratio

P = pressure

DAF = dynamic amplification factor

V = volume of enclosure

V = change in volume

ρ = mass per unit area

P\_ = atmospheric pressure

t = time

w = lateral deflection of plate

K = effective stiffness

 $= \frac{d}{d}()$ 

D = window stiffness =  $\frac{Et^3}{12(1-v^2)}$ 

# Subscripts

i = inside

o = outside

n = net

s = static

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Response of Structures to Sonic Booms B-58 and F-104 Aircraft	s Produced by X	B <b>-</b> 70,	
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The response of test structures and structure elements to sonic booms produced by XB-70, B-58 and F-104 aircraft was studied. These aircraft produced sonic booms of different signature durations. They were flown at several flight track offsets, altitudes and Mach numbers so as to generate different overpressure levels and signature characteristics. Free field signature data and the effects of free field signature parameters on structural response were analysed. Studies were made of the plate response (lateral deformation) and racking response (in-plane deformation) of the test structures. Damage complaints resulting from the test missions were investigated and the results analysed. The implications of the magnitudes of the responses of the test structures and the investigation of the damage claims resulting from the test missions on possible damage caused by supersonic flights were discussed.

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	Overpressure						
	Structure response	İ					
	Racking response					İ	
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